

GOVERNO DO ESTADO



CEARÁ

AVANÇANDO NAS MUDANÇAS

**GOVERNO DO ESTADO DO CEARÁ
SECRETARIA DOS RECURSOS HÍDRICOS**

**COMPANHIA DE GESTÃO DOS RECURSOS HÍDRICOS COGERH
PROJETO DE DESENVOLVIMENTO URBANO E GESTÃO DOS RECURSOS HÍDRICOS
DO ESTADO DO CEARÁ PROURB/CE**

**PROJETO EXECUTIVO DA BARRAGEM SOUZA
NO MUNICÍPIO DE CANINDÉ**

**TOMO II
ESTUDOS BÁSICOS**

**VOLUME III
ESTUDOS HIDROLÓGICOS**

CONCREMAT

**FORTALEZA
DEZEMBRO DE 1996**

GOVERNO DO ESTADO



CEARÁ

AVANÇANDO NAS MUDANÇAS

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SECRETARIA DOS RECURSOS HÍDRICOS
COMPANHIA DE GESTÃO DOS RECURSOS HÍDRICOS - COGERH
PROJETO DE DESENVOLVIMENTO URBANO E GESTÃO DOS RECURSOS HÍDRICOS
PROURB/CE

PROJETO EXECUTIVO DA BARRAGEM SOUZA

TOMO II
RELATÓRIO GERAL

ESTUDOS BÁSICOS

VOLUME III
ESTUDOS HIDROLÓGICOS



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FORTALEZA
DEZEMBRO/96



CONCREMAT
ENGENHARIA E TECNOLOGIA S A

PROJETO EXECUTIVO DA BARRAGEM SOUZA

CANINDÉ - CEARÁ

ESTUDOS HIDROLÓGICOS

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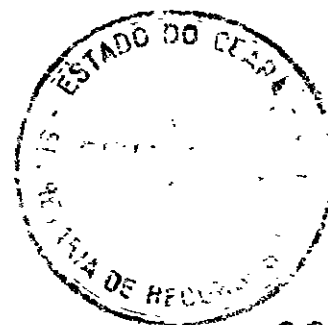
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3 - ESTUDOS HIDROLÓGICOS

3.1 - Introdução

O presente documento se constitui no relatório dos estudos hidrológicos na bacia do rio Juriti, visando as definições do projeto da barragem do Açude Público Souza, no município de Canindé, Estado do Ceará

A bacia hidrográfica do rio Juriti, no local do barramento, possui uma área de 220 Km². A extensão do rio Juriti, da sua nascente, até o local do barramento, é de aproximadamente 21 Km, com desnível geométrico em torno de 238 metros.

Para delimitação e planimetragem da bacia hidrográfica, realizada neste estudo, foi utilizada as cartas planialtimétricas, escala 1 100 000, elaboradas pela SUDENE (CANINDÉ - FOLHA - SB 24 V B - 111 E QUIXADÁ - FOLHA - SB 24 - V - B - VI)

Na determinação das curvas "COTA x ÁREA x VOLUME", utilizou-se a planta topográfica planialtimétrica na escala 1 10 000, com curvas de nível equidistantes de 1 metro

Ao longo dos estudos foram utilizados dados, produtos e critérios já desenvolvidos pelo Plano Estadual de Recursos Hídricos do Estado do Ceará (PERH-CE). O referido estudo, de abrangência regional, permite dispor de informações já tratadas do local de barramento e de uma normalização de dados e metodologias que facilitam amplamente este trabalho

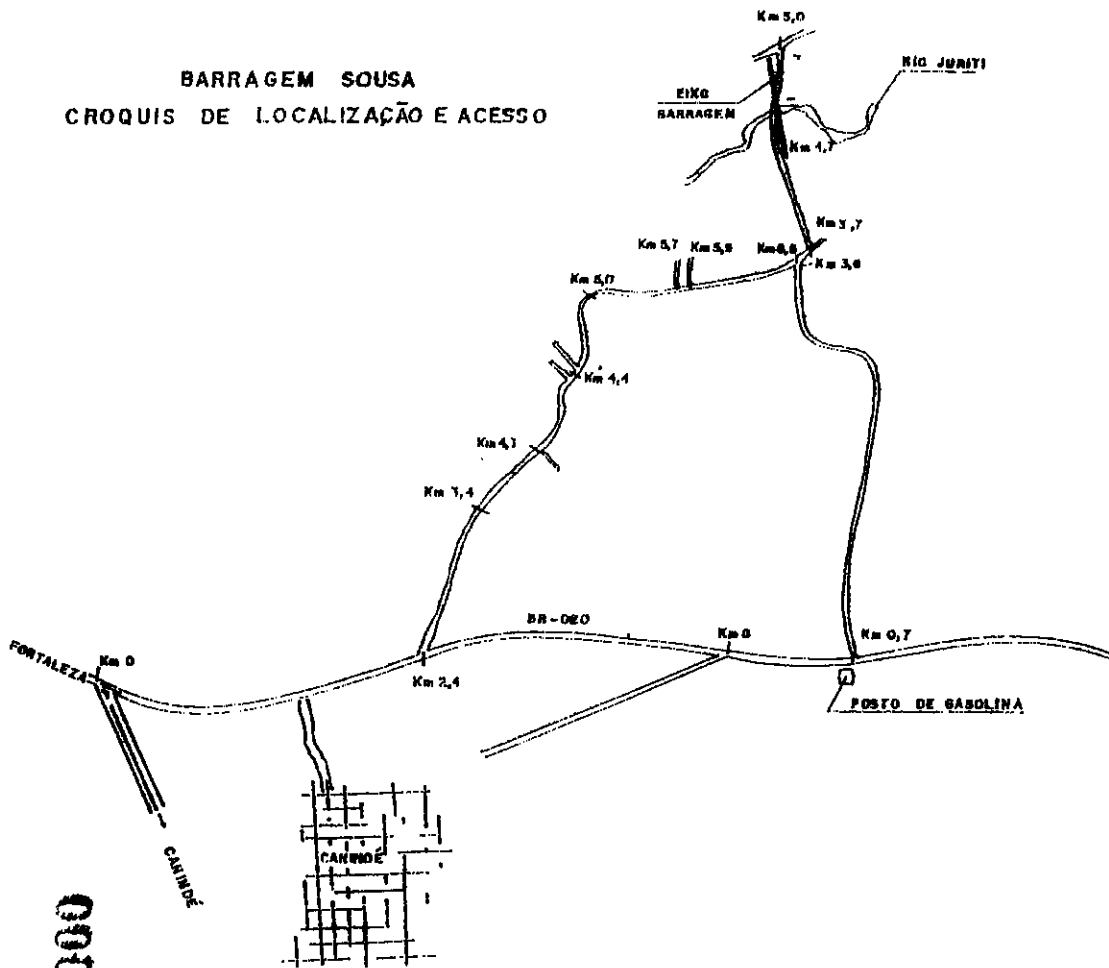
3.2 - Caracterização Física e Morfológica da Bacia Hidrográfica do Reservatório da Barragem Souza.

O boqueirão do "Souza" se localiza no rio Juriti, tributário do rio Canindé, pertencente à bacia do rio Curu, ao sudeste do município de Canindé, região denominada Sertão de Canindé no Estado do Ceará. O mapa da **Figura 1** apresenta esta localização

A bacia hidrológica correspondente ao ponto de controle do boqueirão denominado Souza pertence a alta bacia do rio Canindé, suas vertentes se localizam nas escarpas ocidentais da serra da Baturité



BARRAGEM SOUSA
CROQUIS DE LOCALIZAÇÃO E ACESSO



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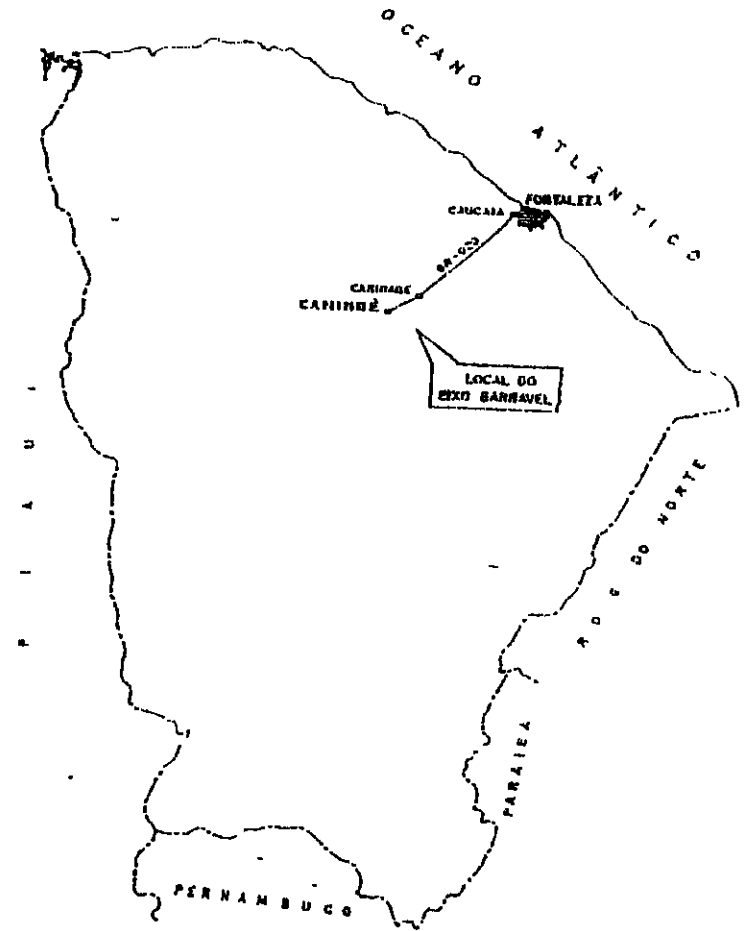


FIGURA 1
MAPA DE LOCALIZAÇÃO E ACESSO

No local o barramento drena uma área de 219,3 Km². A extensão do rio Juriti até o local do barramento é de aproximadamente 21 Km, com um desnível geométrico da ordem de 238 metros. a bacia é formada de terrenos cristalinos, com declividade média dominante e pronunciada nas nascentes (8 a 10%), conseqüentemente com uma razoável aptidão para a formação de picos de cheia de considerável vazão instantânea e pequena duração

Neste local, a bacia apresenta os seguintes parâmetros

Área (A)	219,3 Km ²
Comprimento do Thalveg	21 Km
Altitude Média	250 m
Perímetro (P)	36 Km
Índice de Compacidade (Ic)	0,68

$$Ic = (0,28 \times P) / \sqrt{A} = (0,28 \times 36) / \sqrt{220} = 0,68$$

Os riachos que conformam este sistema nascem nas escarpas da serra de Baturité, acompanhando um extenso sistema de canyons paralelos, para logo após ter vencido o primeiro trecho de bacia, convergir de forma radial no rio Juriti. Assim, esta rede de drenagem configura as características morfológicas mais importantes da bacia, tendo riachos com várias confluências, convergindo em forma de leque alguns quilômetros a montante do local identificado para o barramento

Outra característica morfológica decorre do comportamento das declividades destes cursos de água, que se apresentam uma região superior com declividades da ordem de 8-10% para passar na média e baixa bacia a declividades da ordem de 1-3 %



3.2.1 - Vegetação

A vegetação nativa predominante é constituída de mata xerófila de alta densidade, característica da caatinga, muitas das quais perdem sua folhagem durante a estação seca e a recobram exuberante as primeiras precipitações. Verifica-se a grande ocorrência da aroeira, unha de gato, sabiá, jurema branca, jurema preta, marmeleiro, mofumbo e mandacaru.

As faixas, que formam o fundo dos vales, notadamente na porção inferior da bacia do rio Juriti, a jusante do boqueirão se abrem em extensas áreas planas (baixios), onde ocorrem as principais culturas da região.

A vegetação mais rica aparece no topo da serra de Baturité, com ocorrência de espécies vegetais mais desenvolvidas, tais como, o Pau Branco, Ipê, Cedro, Sabiá e Aroeira, em grande parte componente da Área de Preservação Ambiental (APA - Baturité).

Já nas partes mais baixas a vegetação é mais rala e muito depredada, aparecendo predominantemente a vegetação arbustiva secundária e espécimes tais como o sabiá, Catingueira, Jurema, Aroeira, Marmeleiro, Mofumbo e Mandacaru.

3.2.2 - Fatores Pedológicos e Uso da Terra

Os solos da região são em geral de pouca profundidade, recobertos por vegetação arbustiva, onde se desenvolve uma pequena agricultura de subsistência e a pecuária extensiva.

3.2.3 - Fatores Geomorfológicos

Os solos na região da barragem são predominantemente oriundos do intemperismo do complexo cristalino (gnaisse migmatíticas), posicionados no pré-cambriano, caracterizados por relevo mais movimentado, chegando a ondulado forte em áreas com abundância de afloramento rochosos. São encontradas expressivas áreas com recobrimentos de seixos rolados, predominando os solos basicamente arenosos e/ou areno argilosos.



Nas encostas encontramos um relevo ondulado a forte ondulado, com áreas restritas de topografia mais suave, ocorrendo comumente afloramento de rochas

Os solos no alto vale do rio Canindé, no qual deságua o Juriti, são rasos na sua quase totalidade, com a vegetação arbórea - arbustiva apresentando característica de "Caatinga"

Ao longo do rio encontramos a presença de solos aluviais, constituídos de uma faixa estreita de sedimentos oriundos do transporte fluvial, com relevo plano e de boa qualidade

O rio Juriti e seus afluentes apresentam um padrão de drenagem dentritico, com ramificações irregulares do curso em toda sua extensão, aliás uma característica do sistema drenante das cabeceiras do vale do rio Curu e do rio Canindé, drenos principais do sistema hidrográfico focalizado

Os sedimentos quaternários que ocorrem com pouca representatividade na área são representados pelos aluviões do rio Juriti, capotando rochas do cristalino

A área apresenta pouca variedade petrográfica, abrangendo rochas sedimentares e metamórficas, com predominância das metamórficas (migmatitos e gnaisses)

Situada na parte sul do vale do Curu a região engloba a feição morfológica denominada Planalto Sertanejo

3.3 - Caracterização Climática,

3.3.1 - Climatologia

Na região de localização e de influência da barragem, é dominante praticamente um único tipo climático, ou seja, quente e semi-árido, com estação chuvosa ocorrendo no outono e temperatura superior a 18°C no mês mais frio, correspondente, ao tipo BSH de Koppen

A temperatura média anual é da ordem de 27°C nas partes mais elevadas da bacia, com amplitude térmica média anual pequena, de 2 a 3°C, situando-se o período mais quente na primavera (Outubro a Novembro), que é a época mais seca, e a mais fresca no outono (Março e Abril) que corresponde à época chuvosa

A umidade relativa média anual situa-se na faixa de 65%, aumentando na fase chuvosa para 70%

3.3.2 - Regime Pluviométrico

A precipitação média calculada na bacia é de 660 mm. A **Figura 3** mostra um quadro comparativo dos 10 postos pluviométricos apresentando a distribuição mensal, os valores médios anuais, os coeficientes de variação da precipitação e o número de anos de dados disponíveis nos respectivos postos pluviométricos. O regime de ocorrência de dias chuvosos para o município de Canindé pode ser observado na **Tabela 1**.

Nas proximidades de Canindé o regime pluviométrico é moderado e típico da região semi-árida, ocorrendo chuvas mais intensas na Serra de Baturité. **Figura 2**

O Trimestre mais úmido corresponde aos meses de Fevereiro, Março e Abril, com média de 443mm.

Os postos pluviométricos existentes na bacia fazem parte da rede pluviométrica pertencente a SUDENE. Estes postos dispõem de dados coletados mediante pluviômetros convencionais no período 1913 - 1988.

Estes dados foram sujeitos à análise de consistência durante o PERH, que consistiu na aplicação da metodologia do "Vetor Regional" para identificação de erros e preenchimento de falhas.

O posto com maior influência na bacia é o do Açude Salão com coordenadas geográficas 4° 25' e 39° 20', com precipitação média anual em torno de 644 mm.

Foram utilizados, os dados de chuva máxima de um dia, do posto Salão, para obtenção das chuvas correspondentes às frequências consideradas na determinação das descargas máximas a serem esperadas no local do açude.

As séries integrais do posto mencionado, contendo os valores das médias mensais e anuais e máximas diárias anuais, encontram-se nas **Tabelas 1 e 2**.

3.3.3 - Caracterização da Chuva Sobre a Bacia

O estudo das variações da precipitação anual próxima ao local da barragem foi feito utilizando os dados da estação pluviométrica Salão. Os dados são relativos ao período de 1917 a 1985 e encontram-se apresentados na **Tabela 2**.



TABELA 1

FREQUÊNCIA DE PELO MENOS n DIAS COM OCORRÊNCIA DE CHUVA

POSTO	MÊS	NÚMERO DE DIAS									
		1	2	4	8	12	15	18	20	22	25
2881734	JAN	29,2	27,7	21,5	10,8	1,5	1,5	1,5	0,0	0,0	0,0
	FEV	29,2	29,2	26,1	20,0	15,4	10,8	7,7	4,6	0,0	0,0
	MAR	30,8	30,8	29,2	26,1	23,1	13,9	10,8	6,2	4,6	1,5
	ABR	29,2	29,2	27,7	26,1	20,0	12,3	9,2	6,2	1,5	0,0
	MAI	30,8	29,2	23,1	12,3	7,7	4,6	1,5	0,0	0,0	0,0
	JUN	26,1	20,0	15,4	4,6	1,5	0,0	0,0	0,0	0,0	0,0
	JUL	15,4	12,3	9,2	0,0	0,0	0,0	0,0	0,0	0,0	0,0
	AGO	10,8	6,2	1,5	0,0	0,0	0,0	0,0	0,0	0,0	0,0
	SET	3,1	0,0	0,0	0,0	0,0	0,0	0,0	0,0	0,0	0,0
	OUT	4,6	0,0	0,0	0,0	0,0	0,0	0,0	0,0	0,0	0,0
	NOV	7,7	3,1	0,0	0,0	0,0	0,0	0,0	0,0	0,0	0,0
	DEZ	16,9	13,9	4,6	1,5	0,0	0,0	0,0	0,0	0,0	0,0

Fonte: PERH - Plano Estadual de Recursos Hídricos

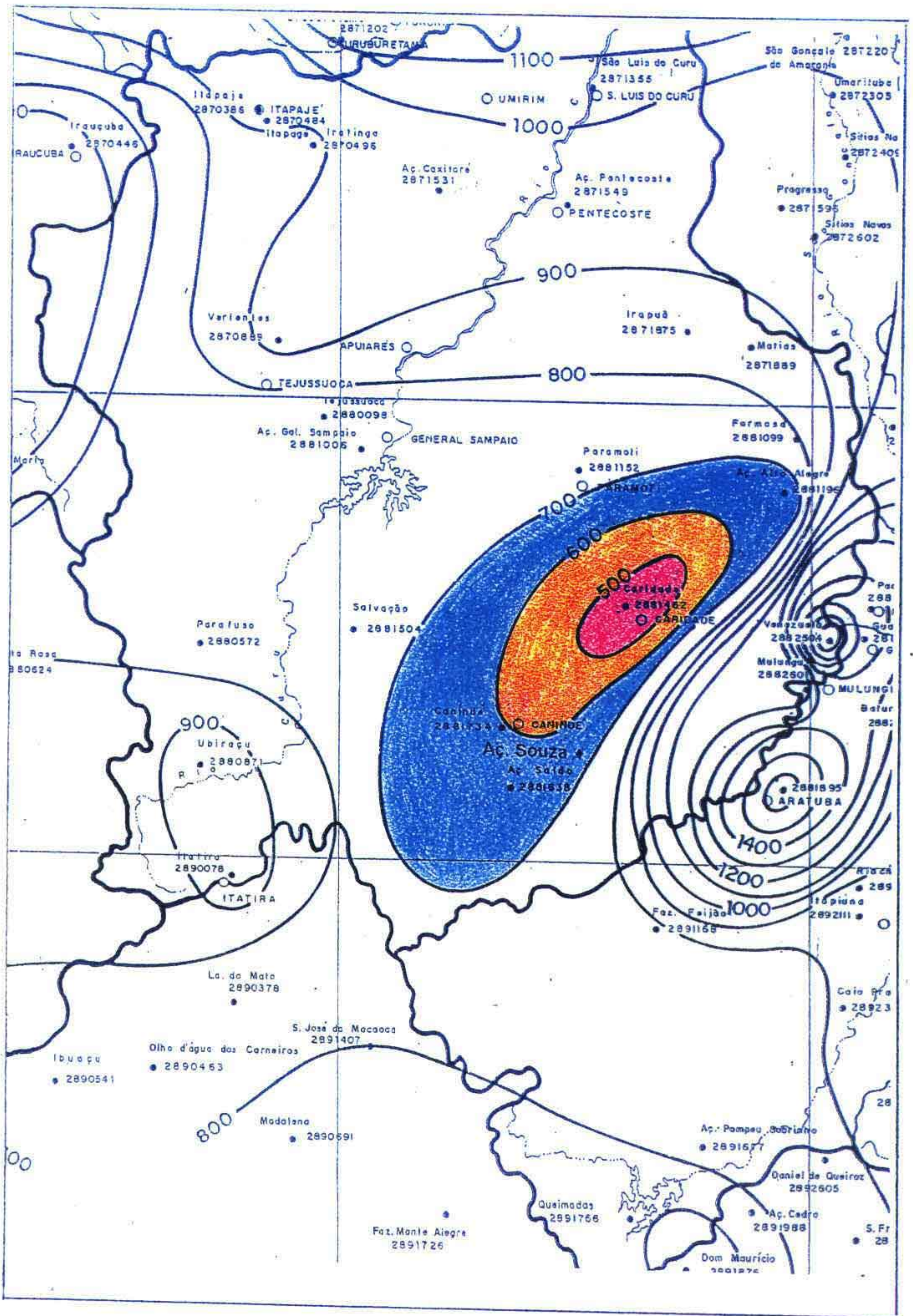


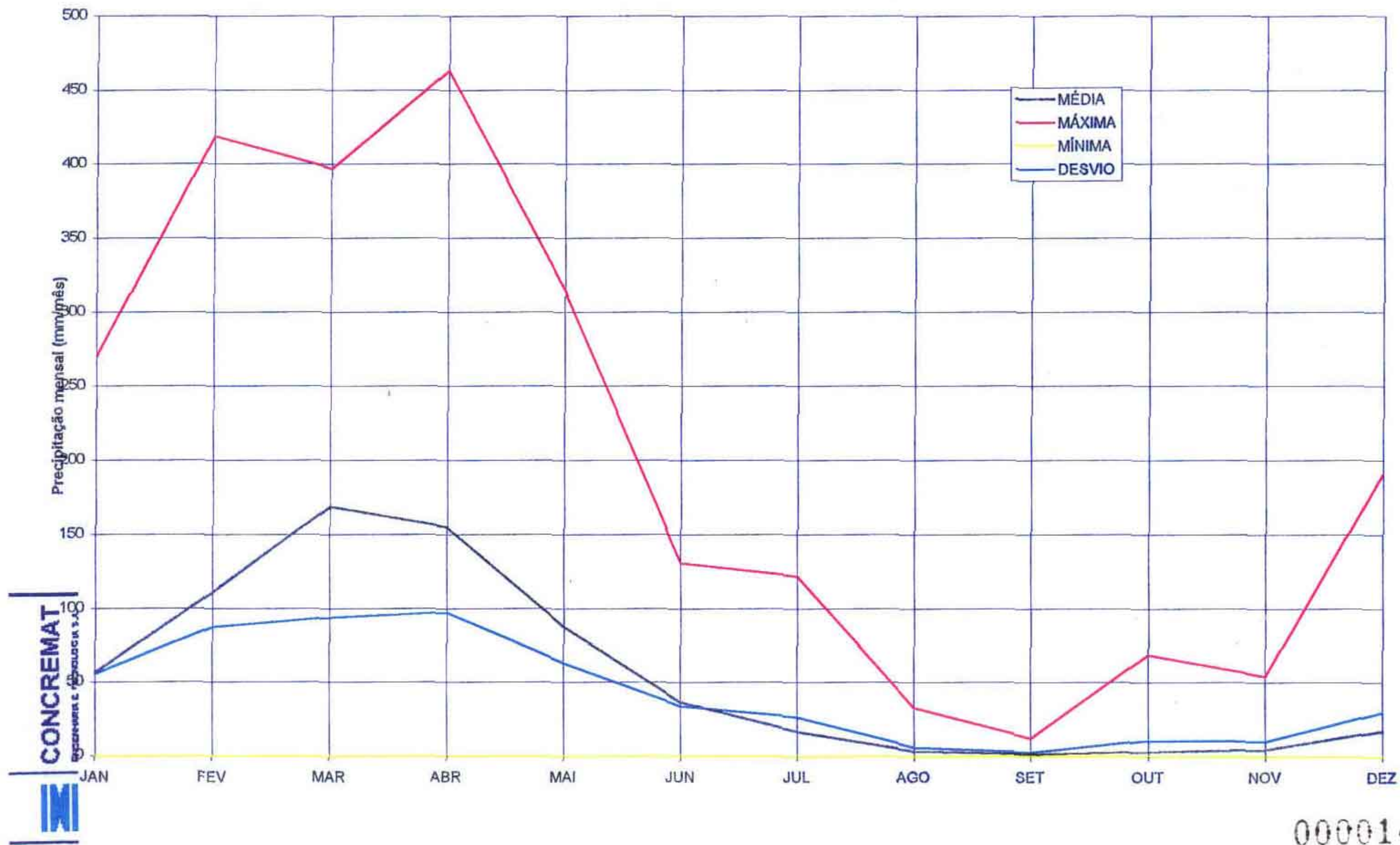
FIGURA - 2

ISOIETAS

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Figura 3 - Distribuição Mensal da Precipitação Média, Máxima, Mínima e Desvio Padrão



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TABELA 02													
SUDENE/DPG/PRN/HME *** BANCO DE DADOS HIDROCLIMATOLÓGICOS DO NORDESTE ***													
* SISTEMA DE PLUVIOMETRIA *													
PLUVIOMETRIA MENSAL													
EDIÇÃO EM 15/0290 POSTO -													
SALÃO AC ESTADO - CEARÁ LATITUDE 04-25 NÚMERO - 21881838													
MUNICÍPIO - CANINDÉ LONGITUDE 39-19													
COD.NAC. - 00439015 INSTALADO EM 1912 P/DNOCS ALTITUDE 200M													
ANO	JAN	FEV	MAR	ABR	MAI	JUN	JUL	AGO	SET	OUT	NOV	DEZ	TOTAL
1916	-	-	-	-	-	90,4	0,0	0,0	0,0	0,0	0,0	33,4	-
1917	165,7	214,8	378,3	86,8	171,6	68,3	0,3	0,0	0,0	0,0	22,3	8,4	1116,5
1918	71,2	23,8	89,8	22,7	28,6	11,8	0,0	13,5	0,0	0,0	0,0	44,3	305,7
1919	13,3	29,5	9,0	0,2	19,3	0,0	0,0	6,0	0,0	0,0	0,0	0,0	77,3
1920	5,5	30,7	306,1	300,2	94,6	18,1	15,7	9,0	0,0	0,0	0,0	78,4	858,3
1921	59,3	266,1	274,2	265,0	151,1	35,9	61,3	0,0	0,0	0,0	2,0	0,0	1114,9
1922	5,0	49,5	114,0	506,1	188,3	131,8	43,9	15,0	0,0	0,0	30,0	9,0	1092,6
1923	59,8	224,9	60,2	81,0	121,5	40,8	34,9	0,0	0,0	0,0	0,0	0,0	622,9
1924	214,9	523,9	443,4	515,2	173,5	58,3	18,9	0,0	0,0	10,3	0,0	34,7	1993,1
1925	59,3	94,1	175,5	247,1	67,7	0,0	0,0	0,0	0,0	0,0	0,0	3,0	646,7
1926	81,6	206,3	458,4	306,8	214,4	0,0	0,0	0,0	0,0	0,0	0,0	0,0	1267,5
1927	73,7	156,6	112,6	170,5	77,9	0,0	0,0	0,0	0,0	0,0	0,0	0,0	591,3
1928	94,2D	4,8D	152,8D	226,4D	3,0	16,3	8,7	0,0	0,0	2,5	0,0	7,9	516,6
1929	55,4	237,8	191,8	129,3	76,7	5,8	67,7	0,0	18,4	0,0	0,0	23,7	806,7
1930	122,2	65,3	187,6	102,5	6,5	41,6	0,0	8,0	0,0	0,0	0,0	14,0	547,7
1931	149,1	92,0	124,0	149,6	55,3	0,0	0,0	0,0	0,0	0,0	0,0	2,4	572,4
1932	14,3	127,9	58,7	23,6	7,4	4,7	0,0	0,0	18,0	0,0	0,0	3,5	258,1
1933	91,0	84,2	101,0	237,4	32,7	0,0	0,0	0,0	0,0	0,0	0,0	7,0	553,3
1934	24,0	349,0	218,0	195,4	117,0	23,0	0,0	0,0	0,0	0,0	12,0	11,7	860,1
1935	29,7	241,3	159,5	222,6	130,8	72,6	5,5	5,5	0,0	0,0	0,0	2,2	869,5
1936	25,6	121,3	56,2	51,7	61,0	66,0	0,0	0,0	0,0	0,0	0,2	3,0	385,0
1937	0,0	156,0	62,0	195,0	59,5	74,0	8,0	0,0	0,0	0,0	0,0	2,5	557,0
1938	22,5	14,1	277,5	112,5	96,0	24,5	12,0	0,0	3,0	0,0	0,0	0,0	562,1
1939	5,0	150,0	224,1	187,1	37,3	14,8	14,6	0,0	10,0	68,5	4,0	4,0	659,4
1940	68,1	113,3	144,9	183,2	222,4	35,0	41,9	9,0	0,0	0,0	0,0	0,0	827,8
1941	4,0	84,0	208,4	137,0	26,7	8,0	8,0	0,0	0,0	0,0	0,0	16,0	484,1
1942	16,0	70,0	57,0	108,0	18,0	16,0	0,0	0,0	0,0	4,0	12,0	0,0	301,0
1943	87,5	50,0	42,5	105,7	34,4	4,0	24,7	0,0	0,0	0,0	0,0	14,5	363,3
1944	66,0	10,0	240,0	35,1	97,0	0,0	0,0	0,0	0,0D	0,0	1,0	16,8	465,9
1945	123,0	172,7D	116,0D	282,5D	113,0D	22,0D	6,0D	0,0D	0,0	2,5D	0,0D	100,0D	937,7
1946	238,0	84,5	101,0	81,5	29,5	9,5	0,0	0,0	0,0D	0,0	14,2	33,2	591,4
1947	36,3D	188,3D	389,2D	81,9D	87,6D	70,9D	90,3D	4,0D	0,0	0,0D	43,1D	17,9D	1009,5D
1948	12,0	42,6	150,3	84,0	107,9	22,7	18,7	0,0	0,0	0,0	0,0	2,4	440,6
1949	16,4	87,3	196,8	147,0	36,6	0,0	0,0	0,0	0,0	0,0	54,2	0,0	538,3
1950	12,3	56,0	234,3	424,0	134,2	45,0	0,0	0,0	0,0	0,0	0,0	21,0	926,8
1951	20,0	10,0	113,0	87,0	6,0	97,0	14,2	0,0	0,0	0,0	2,0	44,7	393,9
1952	30,7	35,6	106,0	112,6	58,4	6,2	0,0	0,0	0,0	3,5	0,0	35,0	388,0
1953	5,5	41,8	93,1	154,6	50,9	17,9	19,0	0,0	0,0	0,0	30,8	3,2	416,8
1954	4,8	175,5	59,3	56,1	95,1	1,0	0,0	0,0	0,0	0,0	1,2	0,0	393,0
	JAN	FEV	MAR	ABR	MAI	JUN	JUL	AGO	SET	OUT	NOV	DEZ	TOTAL

D - TOTAL MENSAL SOMENTE

E,F,G - VALOR ESTIMADO

H - VALOR HOMOGENEIZADO

* VALOR DUVIDOSO

0:0015



CONTINUAÇÃO

SUDENE/DPG/PRN/HME

*** BANCO DE DADOS HIDROCLIMATOLÓGICOS DO NORDESTE ***

* SISTEMA DE PLUVIOMETRIA *

PLUVIOMETRIA MENSAL

POSTO - SALÃO
NÚMERO - 21881838
COD.NAC - 00439015

AC

ESTADO - CEARÁ
MUNICÍPIO - CANINDÉ
INSTALADO EM 1912 P/DNOCS

EDIÇÃO EM 15/0290
LATITUDE 04-25
LONGITUDE 39-19
ALTITUDE 200M

ANO	JAN	FEV	MAR	ABR	MAI	JUN	JUL	AGO	SET	OUT	NOV	DEZ	TOTAL
1955	38,0	50,0	84,0	17,0	0,0	16,0	0,0	0,0	0,0	0,0	0,0	0,0	205,0
1956	0,0	91,0	205,0	130,0	0,0	0,0D	0,0	0,0	0,0D	0,0	0,0	0,0D	426,0
1957	44,0	17,0	201,0	274,0	14,0	0,0	0,0	0,0	0,0	0,0	0,0	6,3	554,3
1958	0,0	0,0	0,0	20,0	65,0	10,0	0,0	0,0	0,0	0,0	0,0	0,0	95,0
1959	23,0	180,0	59,0	70,0	39,0	96,0	0,0	0,0	0,0	0,0	0,0	0,0	467,8
1960	0,0	23,0	300,0	96,0	58,0	10,0	0,0	0,0	0,0	0,0	0,0	0,0	487,0
1961	73,0	272,0	284,0	145,0	98,0	14,0	0,0	0,0	0,0	0,0	0,0	7,4	893,4
1962	67,2	83,4	199,0	116,7	68,6	0,0	0,0	0,0	0,0	0,0	0,8	8,5	544,2
1963	134,4	80,1	222,8	101,3	26,6	0,0	0,0	0,0	0,0	0,0	39,9	141,8	746,9
1964	259,5	222,9	141,6	324,5	81,6	32,5	70,4	0,0	0,0	0,0	0,0	0,0	1133,0
1965	78,6	3,0	138,0	79,0	104,0	21,3	0,0	0,0	0,0	0,0	0,0	0,0	423,9
1966	0,0	70,9	35,4	134,1	83,0	3,0	28,5	0,0	0,0	0,0	0,0	6,5	358,4
1967	15,0	197,8	207,0	200,0	82,6	0,0	0,0	0,0	0,0	0,0	0,0	0,0	702,4
1968	27,0	72,0	359,2	276,2	260,0	0,0	0,0	0,0	0,0	0,0	0,0	0,0	994,4
1969	61,5	49,4	106,2	205,0	30,9	43,3	102,1	44,4	0,0	0,0	0,0	0,0	642,8
1970	83,0	46,1	230,7	74,3	62,7	39,4	0,0	0,0	0,0	5,4	0,0	0,0	541,6
1971	103,1	110,5	164,0	148,7	104,3	46,0	80,0	11,2	0,0	-	0,4	4,2	-
1972	21,5	33,5	5,6	38,2	7,0	32,0	2,5	5,0	0,0	0,0	0,0	20,0	165,3
1973	53,2	128,2	193,2	96,7	92,9	57,9	24,2	2,2	0,0	0,0	0,0	18,6	667,7
1974	203,6	129,5	291,6	356,8	243,6	36,3	13,8	0,0	1,8	21,5	2,9	23,0	1324,4
1975	47,5	112,6	169,0	180,3	163,9	40,2	58,4	0,3	11,6	0,0	1,6	45,0	831,4
1976	35,3	89,2	155,2	72,0	22,6	7,4	6,0	4,2	0,0	0,0G	0,0	3,8	395,7
1977	125,4	103,2	71,4	184,4	67,8	48,8	49,2	0,0	0,0	0,0	0,0	0,0	650,0
1978	6,0	128,6	212,6	185,7	64,9	27,0	9,2	0,0	0,0	0,0	33,0	9,8	676,8
1979	29,3	55,4	91,2	87,2	75,5	0,0	0,0	0,0	0,0	0,0	0,0	17,2	365,8
1980	67,4	242,8	76,5	42,5	34,0	24,2	0,0	0,0	0,0	0,0	0,0	0,0	487,4
1981	25,4	24,0	315,0	44,0	41,0	0,0	0,0	0,0	0,0	6,0	0,0	44,2	494,6
1982	10,2	61,8	249,1	136,0	73,3	7,2	0,0	5,0	0,0	0,0	4,0	0,0	552,6
1983	0,0	78,4	124,0	83,4	18,4	0,0	0,0	0,0	0,0	0,0	0,0	19,2	323,4
1984	15,0	97,2	197,4	169,0	189,0	31,8	24,4	3,0	0,0	0,0	0,0	0,0	726,8
1985	195,6	327,6	352,1	415,8	122,1	96,6	36,6	0,0	0,0	0,0	1,0	41,6	1589,0
	JAN	FEV	MAR	ABR	MAI	JUN	JUL	AGO	SET	OUT	NOV	DE	TOTAL
Nº DE ANOS C/ DADOS	69	69	69	69	69	70	70	70	70	70	70	70	68
MÉDIA	68,3	114,0	172,7	166,3	79,7	27,0	14,5	2,0	0,8	1,8	4,4	14,4	643,8
MÁXIMA	259,5	523,9	468,4	515,2	260,0	131,8	102,1	44,4	18,4	68,5	54,2	141,8	1993,1
MÍNIMA	0,0	0,0	0,0	0,2	0,0	0,0	0,0	0,0	0,0	0,0	0,0	0,0	77,3
D - TOTAL MENSAL SOMENTE		E,F,G - VALOR ESTIMADO				H - VALOR HOMOGENEIZADO				* - VALOR DUVIDOSO			
ORIGEM DOS DADOS - ARQUIVO DE MICROFICHAS DA SUDENE													



A partir da análise das séries pluviométricas verifica-se que o regime das chuvas caracteriza-se por uma irregularidade interanual, ou seja, em determinado ano ocorre alta pluviosidade, enquanto que em outros as precipitações são bastante deficientes. Os coeficientes de variação são bastante elevados, normalmente com valores de 0,4.

A média anual de chuvas da estação Salão está em torno de 644 mm.

As precipitações se concentram no quadrimestre de fevereiro a maio, quando se verifica, em média, 81% da precipitação anual. O período de menor precipitação se situa, normalmente, nos meses de agosto, setembro e outubro, nos quais se verifica em média, o máximo de 1% da média anual.

Praticamente a metade da chuva anual ocorre em somente 2 meses, março e abril. O trimestre fevereiro/abril ou março/maio é responsável por quase dois terços da pluviosidade do ano.

A visualização gráfica dessa repartição temporal é representada na **Figura 04**, que contém o histograma médio mensal para o posto considerado neste trabalho, o qual está mais diretamente associado e representável ao local do barramento.

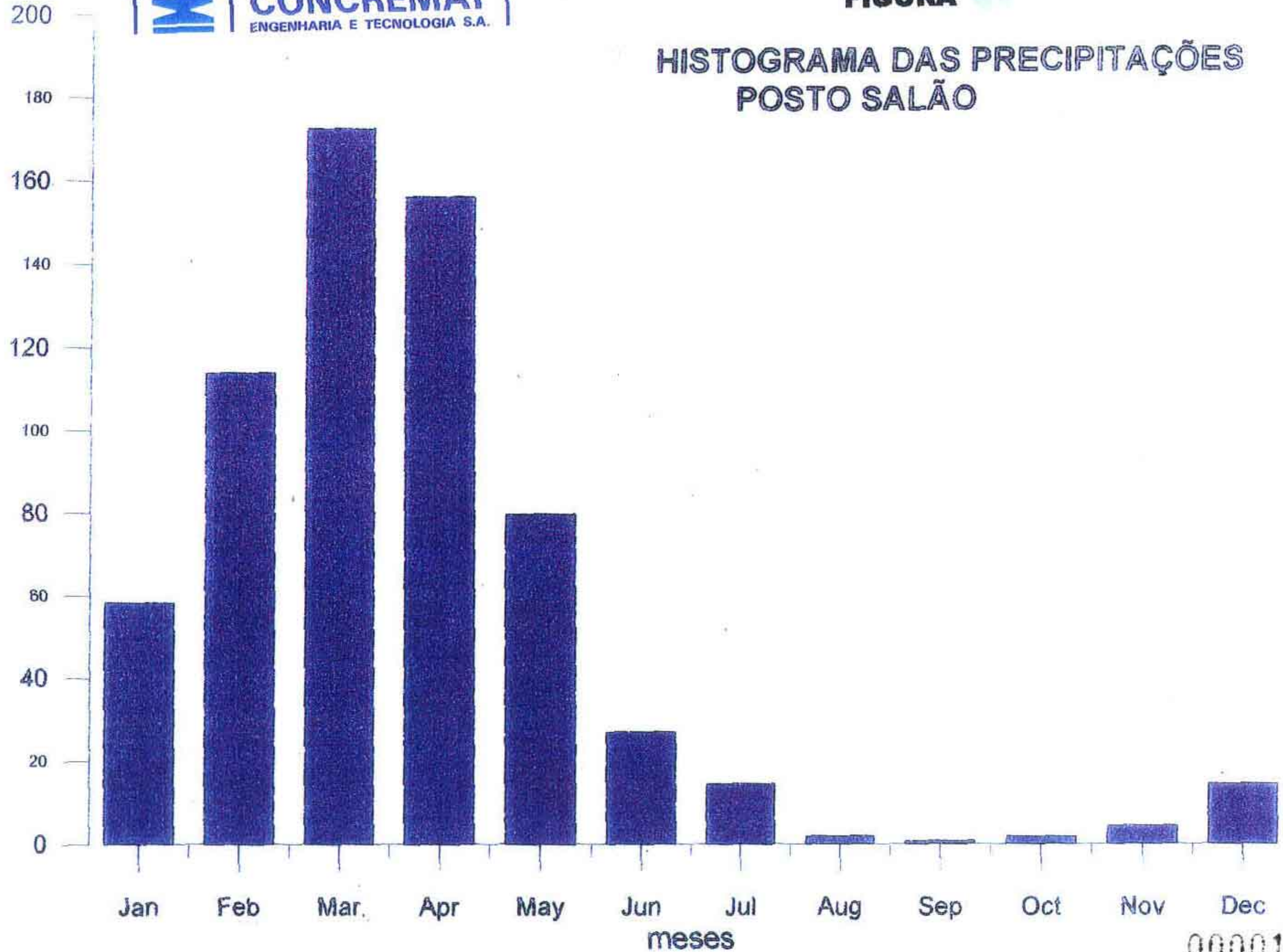


CONCREMAT
ENGENHARIA E TECNOLOGIA S.A.

FIGURA

HISTOGRAMA DAS PRECIPITAÇÕES POSTO SALÃO

precipitações (mm)



000018



3.3.4 - Série de Precipitações Médias Diárias na Bacia Hidrográfica.

A obtenção da precipitação média na bacia hidrográfica, correspondente ao boqueirão barrável foi obtida pelo método de Thissen, utilizando técnica de Monte Carlo

A metodologia é totalmente digital e consiste no cálculo dos coeficientes de Thissen de cada posto pluviométrico mediante a contagem do número de pontos aleatórios que se situam dentro da bacia hidrográfica e na proximidade de cada posto

A geração dos pontos foi realizada mediante uma rotina de tipo RANDOM, com inicializador (SEED) no relógio do computador e distribuição estatística de valores de tipo retangular, com média (1/2) e variância (1/12).

Estes números aleatórios foram compostos em pares e multiplicados pelos respectivos fatores de escala (latitude e longitude) que permitissem gerar pontos localizados num retângulo que circunscrevia a bacia hidrográfica em estudo

Uma rotina computacional avaliou a localização de cada ponto (dentro ou fora da poligonal que define a bacia hidrográfica) e outra rotina se encarregou de definir qual posto pluviométrico se encontra vizinho ao ponto pesquisado. Sendo os pontos gerados mediante distribuição retangular, a cobertura dos mesmos é homogênea e, conseqüentemente, o número de pontos associados a cada posto pluviométrico representa um excelente indicador do coeficiente de Thissen de referido posto

A vantagem desta técnica com relação às tradicionalmente utilizadas (método de construção geométrica dos polígonos de Thissen) apoia-se de no fato que, esta pode ser aplicada para cada intervalo de tempo com os postos que efetivamente dispõem de dados naquele período, facilitando assim enormemente o cálculo e estendendo a série de precipitações médias o máximo possível com os dados disponíveis

No caso da barragem de Souza foram utilizados no cálculo da precipitação média 10 postos pluviométricos (a Tabela 4 e apresentam os nomes, códigos, características e coordenadas dos postos pluviométricos utilizados) e foram pesquisadas 40 configurações diferentes de polígonos de Thissen. Os resultados



Tabela - 4

Nomes, Códigos e Coordenadas dos Postos Pluviométricos da Região.

Nome do Posto	Código do Posto	Latitude	Longitude
Paramoti	2881152	4° 04'	39° 15'
Salvação	2881504	4° 15'	39° 29'
Parafuso	2880572	4° 16'	39° 39'
Ubiraçu	2880871	4° 24'	39° 39'
Itatira	2890078	4° 31'	39° 37'
Lagoa do Mato	2890378	4° 40'	39° 37'
Caridade	2881462	4° 13'	39° 12'
Saião	2881838	4° 25'	39° 19'
Fazenda Feijão	2891168	4° 34'	39° 10'
General Sampaio	2881006	4° 02'	39° 29'



Tabela - 5 CARACTERIZAÇÃO DO REGIME PLUVIOMETRICO
ANÁLISE MENSAL E ANUAL

PERÍODO 1932-1988

POS.º	CODIGO	PARAM	LAT	LONG	JAN	FEV	MAR	ABR	MAI	JUN	JUL	AGO	SET	OUT	NOV	DEZ	ANUAL
PARAMOTI	2881162	PR	4° 04'	39° 16'	58 316	100 089	193 544	184 175	90 889	45 825	16 112	1 135	2 065	6 126	2 523	18 621	718221
		S			58 827	82 577	90 519	100 849	70 341	40 944	23 304	3 123	4 393	15 210	4 900	28 217	300 398
		CVR			1 009	0 825	0 468	0 546	0 776	0 913	1 446	2 751	2 129	2 463	1 942	1 515	0 418
		G			1 413	1 313	0 467	0 549	1 034	1 303	2 036	4 490	2 340	2 947	2 307	1 904	0 941
		R1			0 053	0 003	0 023	0 000	0 137	0 296	0 128	0 000	0 253	0 471	0 093	0 000	0 000
R	0 251	0 120	0 571	0 600	0 234	0 253	0 070	0 205	0 706	0 239	0 104	0 285	0 540				
SALVAÇAO	2881604	PR	4 16	39° 29'	53 807	107 046	194 509	182 212	90 958	33 244	11 347	2 007	1 254	1 819	5 111	23 726	707 140
		S			58 793	76 226	98 511	108 785	73 512	36 334	8 494	3 900	4 477	6 923	4 767	36 443	292 160
		CVR			1 093	0 712	0 566	0 597	0 808	1 093	1 630	1 043	3 395	3 995	2 890	1 536	0 412
		G			1 352	0 812	0 305	0 310	1 414	1 969	2 240	2 296	4 943	5 186	4 544	2 137	1 097
		R1			0 249	0 000	0 081	0 000	0 000	0 252	0 000	0 000	0 000	0 000	0 000	0 000	0 155
R	0 255	0 197	0 362	0 454	0 243	0 145	0 493	0 000	0 427	0 000	0 010	0 109	0 669				
PARAFUSC	2880672	PR	4° 16'	39° 39'	73 044	118 527	207 314	177 253	91 105	35 321	11 389	2 540	3 739	1 977	4 054	18 412	744 670
		S			84 238	86 779	96 565	103 022	76 964	40 457	19 479	7 035	23 532	7 251	20 607	27 073	305 777
		CVR			1 153	0 732	0 465	0 581	0 845	1 145	1 710	2 769	6 292	3 667	5 083	1 470	0 411
		G			1 382	0 739	0 334	0 664	1 162	1 210	2 445	3 606	7 019	4 571	6 766	1 445	0 929
		R1			0 160	0 000	0 136	0 029	0 111	0 192	0 000	0 331	0 000	0 033	0 000	0 059	0 000
R	0 333	0 151	0 330	0 285	0 334	0 397	0 468	0 000	0 069	0 000	0 000	0 254	0 526				
JBIRACU	2880871	PR	4° 24'	38° 39'	74 446	147 363	245 332	204 874	99 702	48 954	23 295	6 814	2 349	4 493	1 839	27 723	893 882
		S			69 200	105 649	130 170	117 328	72 305	12 456	25 888	11 075	3 504	9 664	20 364	28 125	356 594
		CVR			0 930	0 715	0 531	0 573	0 725	0 867	1 111	1 625	1 492	2 151	2 598	1 015	0 399
		G			1 536	0 808	0 218	0 591	0 925	0 993	1 722	2 593	2 376	4 766	4 511	1 651	1 362
		R1			0 266	0 000	0 112	0 006	0 126	0 192	0 000	0 327	0 000	0 000	0 000	0 000	0 015
R	0 305	0 034	0 329	0 478	0 254	0 207	0 189	0 003	0 545	0 011	0 104	0 436	0 648				
ITATIPA	2890078	PR	4° 31'	39° 37'	64 275	105 395	208 000	209 937	142 837	89 284	44 842	15 453	4 339	2 670	4 502	25 172	904 405
		S			47 157	30 747	115 787	111 030	95 394	89 050	42 348	29 885	8 532	5 233	5 625	21 602	303 691
		CVR			0 269	0 766	0 562	0 329	0 672	0 998	0 949	1 889	1 967	2 036	1 919	1 287	0 374
		G			0 918	1 128	1 139	0 366	0 721	0 549	0 941	4 844	2 979	2 971	3 013	1 751	0 025
		R1			0 000	0 000	0 192	0 000	0 000	0 000	0 122	0 061	0 000	0 060	0 016	0 000	0 000
R	0 268	0 293	0 436	0 556	0 196	0 165	0 336	0 000	0 224	0 415	0 069	0 028	0 493				
LAGOA DO MATO	2890378	PR	4° 40'	39° 37'	46 277	85 530	205 260	174 628	100 386	53 751	17 058	4 854	1 174	1 121	1 651	14 004	705 693
		S			87 959	108 226	289 395	238 657	76 521	72 853	26 433	11 548	3 940	3 854	5 165	20 621	784 477
		CVR			2 115	1 765	1 410	1 367	0 762	1 355	1 550	2 379	3 357	3 437	3 129	1 471	1 112
		G			5 437	3 145	5 552	4 912	0 972	4 013	2 847	3 611	4 211	4 610	4 232	1 712	5 380
		R1			0 022	0 000	0 000	0 000	0 000	0 000	0 000	0 000	0 000	0 000	0 000	0 000	0 000
R	0 653	0 766	0 913	0 82	0 164	0 244	0 032	0 192	0 000	0 000	0 000	0 067	0 308				
CARIDADE	2881462	PR	4° 13'	39° 12'	35 787	70 509	128 558	118 802	53 858	29 302	8 696	1 788	1 205	0 866	2 874	10 433	460 753
		S			29 489	53 389	56 324	69 171	35 490	28 982	11 282	3 921	2 501	1 445	1 701	12 396	193 343
		CVR			0 524	0 587	0 438	0 598	0 659	0 989	1 297	2 195	2 076	1 671	1 561	1 188	0 420
		G			1 072	1 543	0 432	1 598	0 522	2 359	2 228	3 474	3 332	3 143	2 214	2 624	0 947
		R1			0 084	0 006	0 041	0 009	0 000	0 294	0 038	0 084	0 175	0 125	0 000	0 000	0 000
R	1 473	0 272	0 358	0 574	0 256	0 237	0 607	0 000	0 000	0 000	0 067	0 230	0 281				
SALAO	2881838	PR	4 28	39° 19'	51 260	108 102	189 672	154 495	78 170	31 125	13 404	2 040	1 188	4 921	5 289	15 504	635 166
		S			60 475	78 330	83 002	92 398	56 489	32 527	21 666	6 545	3 570	19 230	14 266	26 293	268 858
		CVR			1 186	0 725	0 489	0 596	0 722	1 045	1 631	3 208	3 006	3 903	2 701	1 696	0 408
		G			1 783	1 134	0 128	0 901	1 009	1 072	2 164	5 051	2 873	4 977	3 347	2 835	0 950
		R1			0 193	0 000	0 047	0 000	0 000	0 183	0 000	0 000	0 000	0 000	0 000	0 000	0 000
R	0 275	0 053	0 320	0 504	0 067	0 186	0 592	0 049	0 110	0 000	0 172	0 185	0 801				
FAZENDA FEJAO	2881168	PR	4 31	39° 10'	55 272	100 093	177 865	183 121	115 312	59 104	26 391	5 607	1 332	2 312	3 468	22 228	735 705
		S			52 401	70 689	89 584	65 618	61 139	47 957	37 530	12 262	3 742	5 530	6 568	27 663	314 355
		CVR			0 440	0 625	0 506	0 525	0 721	0 803	1 422	2 187	2 810	2 391	1 894	1 245	0 427
		G			1 189	0 667	0 411	0 615	1 170	0 626	3 064	4 974	5 284	4 688	1 940	1 850	0 976
		R1			0 000	0 001	0 146	0 000	0 000	0 151	0 000	0 000	0 000	0 000	0 000	0 000	0 000
R	0 261	0 352	0 453	0 481	0 359	0 536	0 752	0 029	0 163	0 253	0 000	0 219	0 666				
GENERAL SAMPAIO	2881006	PR	4 02	39° 29'	62 610	112 695	213 042	186 204	99 091	44 823	17 074	2 702	2 475	1 178	5 104	18 896	765 717
		S			60 314	92 584	95 186	113 468	75 201	46 810	28 159	5 243	5 378	3 367	13 332	27 057	345 586
		CVR			0 915	0 822	0 147	0 609	0 780	1 044	1 649	1 940	2 173	2 873	2 612	1 432	0 451
		G			1 287	1 301	0 229	0 652	0 830	1 296	2 513	2 237	2 717	1 195	3 652	1 987	0 600
		R1			0 000	0 000	0 000	0 000	0 032	0 281	0 049	0 000	0 000	0 000	0 000	0 000	0 000
R	0 240	0 272	0 450	0 538	0 374	0 181	0 253	0 202	0 170	0 000	0 000	0 000	0 485				

PR Média, S Desvio Padrão, CVR Coeficiente de Variação, G Assimetria, R1 Autocorrelação (mes anterior), R Autocorrelação (mesmo mes do ano anterior)



obtidos são apresentados na **Figura 3**, onde são apresentadas os principais parâmetros

Pelo exame dos resultados do Thiessen, constata-se que o posto Salão, com coordenadas geográficas 4° 25' e 39° 20', tem ampla predominância sobre a bacia, tendo sido utilizados seus dados de chuva máxima de um dia para obtenção das chuvas correspondentes as frequências consideradas na determinação das descargas máximas a serem utilizadas e espedadas no local da barragem:

As series integrais do posto mencionado contendo os valores das médias mensais e anuais e máximas diárias anuais encontram-se nas **Tabela 2 e 3**.





TABELA 03 - VALORES DAS PRECIPITAÇÕES MÁXIMAS DIÁRIA ANUAIS

ANO	PRECIPITAÇÃO (mm)	ANO	PRECIPITAÇÃO (mm)	ANO	PRECIPITAÇÃO (mm)
1916	41,5	1941	72,2	1968	90,3
1917	74,5	1942	36,0	1969	54,0
1918	44,3	1942	32,0	1970	40,3
1919	90,3	1944	83,0	1971	71,4
1920	66,5	1946	119,0	1972	48,7
1921	60,4	1948	32,2	1973	52,6
1922	66,0	1949	47,7	1974	69,6
1923	46,0	1950	75,0	1975	36,6
1924	76,0	1951	52,0	1976	26,6
1925	66,0	1952	35,0	1977	90,5
1926	69,1	1953	42,3	1978	113,2
1927	50,5	1954	32,5	1979	38,8
1929	68,7	1955	55,4	1980	52,0
1930	37,5	1956	66,0	1981	59,0
1931	70,7	1957	79,0	1982	81,8
1932	62,3	1959	73,0	1983	35,4
1933	45,0	1960	52,0	1984	70,1
1934	56,0	1961	58,0	1985	89,6
1935	41,0	1962	55,8	1986	58,6
1936	46,0	1963	79,3	1987	39,2
1937	129,5	1964	118,8	1988	137,4
1938	107,2	1965	64,5		
1939	76,2	1966	68,0		
1940	51,3	1967	79,0		



3.3.5 - Chuvas Intensas

Para uma utilização prática dos dados de chuva nos trabalhos de hidrologia, faz-se necessário conhecer a relação entre as três características fundamentais da chuva: intensidade, duração e frequência.

Nos trabalhos hidrológicos, em geral, interessa não somente o conhecimento das máximas precipitações observadas nas séries históricas, mas, principalmente, prever com base nos dados observados, e valendo-se dos princípios das probabilidades, quais as máximas precipitações que possam vir a ocorrer em uma certa localidade, com determinada frequência.

A metodologia empregada na determinação das precipitações intensas foi a das ISOZONAS, desenvolvida pelo Prof. Taboaga Torrico. O método consiste, basicamente, em utilizar estudos estatísticos de uma série de chuvas diárias, para, através de um processo de desagregação e regionalização, estimar as precipitações de menores durações (2 horas, 1 hora, 6 minutos).

Os cálculos foram feitos conforme a sequência descrita a seguir:

- **Seleção do posto pluviométrico** - O posto pluviométrico selecionado foi o Salão, devido à sua representatividade na bacia. Este posto apresenta as seguintes coordenadas geográficas: 4° 25' de Latitude Sul e 39° 20' de Longitude de Longitude Oeste. A **Tabela 2** e a **Figura 2**, apresentam a série histórica das precipitações mensais e as principais características das mesmas para o Açude Souza.

- **Compilação dos dados** - Foram compiladas as chuvas máximas anuais, isto é, para uma dada duração escolheu-se a máxima intensidade pluviométrica observada em cada ano hidrológico que compõe a série observada na **Tabela 3**.

- **Estudo probabilístico das precipitações extremas** - A série observada foi ajustada à distribuição de Gumbel. Em geral, as distribuições de valores extremos de grandezas hidrológicas, tais como as chuvas, por exemplo, ajustam-se satisfatoriamente à distribuição tipo I de Fisher-Tippett, conhecida também como a distribuição de Gumbel. Os parâmetros estatísticos obtidos bem como as



precipitações de períodos de retorno de 50, 100, 500, 1 000 e 10 000 anos, constam na **Tabela 6**

TABELA 06			
ANÁLISE ESTATÍSTICA DA SÉRIE DE PRECIPITAÇÕES MÁXIMA DIÁRIAS ANUAIS (POSTO SALÃO)			
PERÍODO DE RETORNO (ANOS)	PROBABILIDADE	VARIÁVEL REDUZIDA	PRECIPITAÇÃO (mm)
50	0,9800	3,902	130,6
100	0,9900	4,600	144,6
500	0,9980	6,214	176,9
1000	0,9990	6,907	190,7
10000	0,9999	9,210	236,8

- Média da Amostra (\bar{x}) 63,8 mm
- Desvio Padrão (S_x) 24,2 mm
- Média da Variável Reduzida (Y_n) 0,55
- Desvio Padrão da Variável Reduzida (S_n) 1,19
- Moda dos Valores Extremos (X_f) $52,6X_f = \bar{x} - S_x (Y_n/S_n)$, donde $X_f = 52,6$
- Variável Reduzida $Y = 0,05 (x - 52,6)$
- Precipitação (x) $x = (Y + 2,63)/0,05$



- **Cálculo da chuva virtual de 24 horas de duração (p24h)** - Esses valores foram obtidos a partir da multiplicação das chuvas de duração de um dia, pelo fator 1,10 (recomendado por Taborga para esta transformação) Os resultados constam na **Tabela 7**
- **Determinação da ISOZONA a qual pertence a bacia** - O posto Salão esta situado na ISOZONA "D" conforme pode ser observado na **Figura 5**
- **Determinação das precipitações com duração inferiores a 24 horas de duração para períodos de retorno de 500 e 1.000 e 10.000 anos** - Essas precipitações foram obtidas multiplicando-se a chuva de 24 horas pela relação R, entre a chuva de 24 horas e 1 hora, obtida da **Figura 5** Para o período de retorno de 500 anos, que não consta na figura, o valor da chuva foi obtido por interpolação Os resultados constam na **Tabela 8** Logo estes resultados foram plotados em papel probabilístico e obtidos os valores de durações de precipitação requeridos (5 min, 15 min, 30 min, 60 min, 2 hs, 6 hs, 12 hs). (**A Figura 6** apresenta o gráfico das alturas de chuva para os diversos períodos de duração)



TABELA 07

CHUVAS VIRTUAIS DE 24 HORAS DE DURAÇÃO, PARA PERÍODOS DE RETORNO DE 50, 100, 500, 1.000 E 10.000 ANOS.

PERÍODO DE RETORNO (ANOS)	PRECIPITAÇÃO DIÁRIA (mm)	CHUVA VIRTUAL 24 HORAS (mm)
50	130,6	143,7
100	144,6	159,1
500	176,9	194,6
1000	190,7	209,8
10000	236,8	260,5

TABELA 08

PRECIPITAÇÕES INTENSAS PONTUAIS DE 1 HORA E 24 HORAS DE DURAÇÃO PARA PERÍODOS DE RETORNO DE 50, 100, 500, 1.000 E 10.000 ANOS

PERÍODO DE RETORNO (ANOS)	P (24 HORAS) (mm)	R (%)	P (1 HORA) (mm)
50	143,7	0,407	58,5
100	159,1	0,403	64,1
500	194,6	0,397	77,3
1.000	209,8	0,390	81,8
10.000	260,5	0,379	98,7

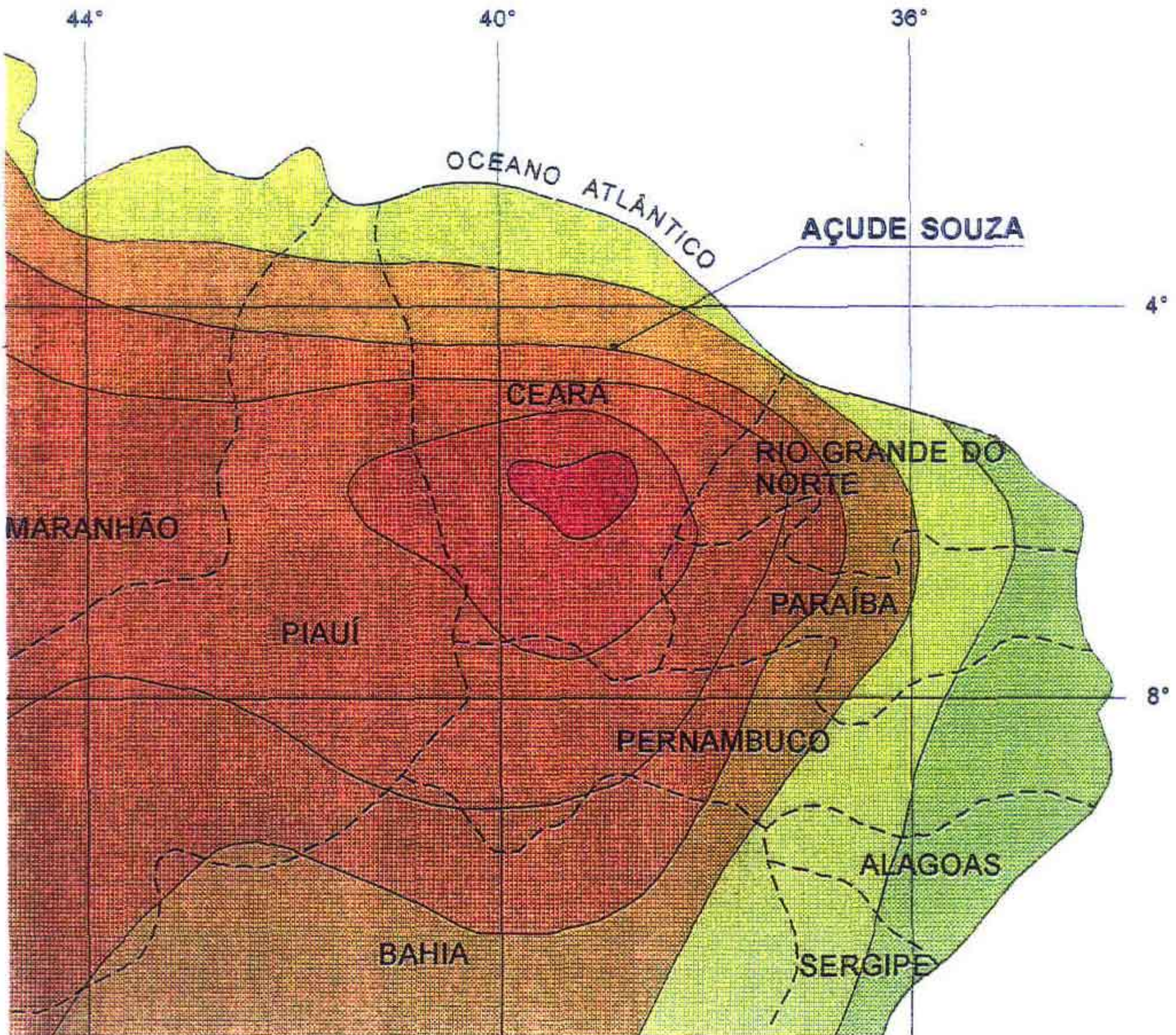
TABELA 09

PRECIPITAÇÕES INTENSAS ESPACIAIS DE 1 HORA E 24 HORAS DE DURAÇÃO PARA PERÍODOS DE RETORNO DE 50, 100, 500, 1.000 E 10.000 ANOS, DISTRIBUÍDAS NA BACIA DO AÇUDE SOUZA.

DURAÇÃO DAS CHUVAS	T = 50 ANOS	T = 100 ANOS	T = 500 ANOS	T = 1.000 ANOS	T = 10.000 ANOS
PRECIPITAÇÕES (MM)					
1	46,2	50,6	61,1	64,6	78,0
24	113,5	125,7	153,7	165,7	205,8

FIGURA 05

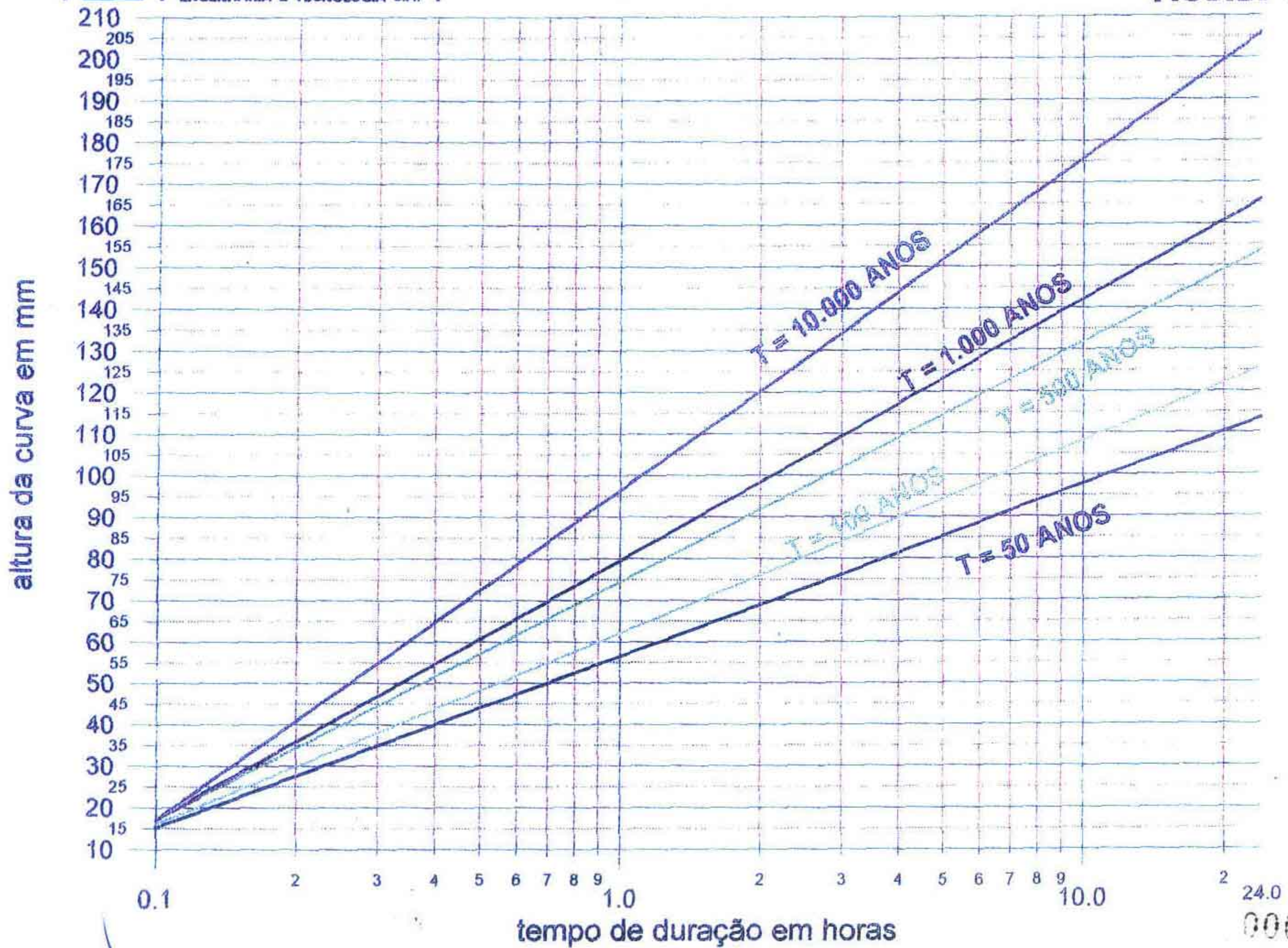
ISOZONAS DO NORDESTE



ISOZONAS DE IGUAL RELAÇÃO

ISOZONA	TEMPO DE RECORRÊNCIA EM ANOS											
	1 HORA / 24 HORAS CHUVA										5 min. 24h CHUVA	
	5	10	15	20	25	30	50	100	1000	10000	5-50	100
B	38.1	37.8	37.5	37.4	37.3	37.2	36.9	36.6	35.4	34.3	8.4	7.5
C	40.1	39.7	39.5	39.3	39.2	39.1	38.0	38.4	37.2	36.2	9.6	9.0
D	42.0	41.6	41.4	41.2	41.1	41.0	40.7	40.3	39.0	37.9	11.2	10.0
E	44.0	43.6	43.3	43.2	43.0	42.9	42.6	42.2	40.9	39.6	12.6	11.2
F	46.0	45.5	45.5	45.1	44.9	44.9	44.6	44.1	42.7	41.3	13.9	12.4
G	47.9	47.4	47.2	47.0	46.0	45.7	46.4	45.9	44.5	43.1	16.4	13.7
H	49.9	49.4	49.1	49.9	49.9	46.5	46.3	47.0	46.3	44.9	16.7	14.9

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3.3.6 - Chuva de Projeto

- Cálculo do Tempo de Concentração da Bacia (T_c)

$$T_c = 57(L^3 / H)^{0.385}, \text{ onde}$$

- T_c = Tempo de concentração em (minutos)
- L = extensão do talvegue em (Km) = 21 Km
- H = máximo desnível no talvegue em (m) = 238m

Logo

$$T_c = 57 (21^3 / 38)^{0.385} = 233.38 \text{ minutos ou}$$

$$T_c = 4 \text{ horas}$$

- Chuvas de Projeto

Com o valor do Tempo de Concentração da bacia de 4 horas, entramos na curva "ALTURA x DURAÇÃO x FREQUÊNCIA" e encontramos as chuvas de projeto para os períodos de retorno 50, 100, 500 e 1 000 e 10 000 anos, conforme Tabela 10.

3.4 - Estudo das Cheias

A determinação das cheias na região do Juriti foi feita através do método do Hidrograma Unitário Triangular (HUT) desenvolvido pelo Soil Conservation Service (SCS). O hidrograma foi determinado para uma precipitação unitária, de 1 milímetro, distribuída sobre toda a bacia, com uma duração correspondente a um quinto do tempo de concentração ($1/5 T_c$)

O excesso de precipitação para o cálculo do hidrograma total foi determinado através do método "Curve number", também desenvolvido pelo SCS

3.4.1 - Cálculo do Hidrograma Unitário Triangular

O cálculo do HUT foi feito para uma duração igual a um quinto do tempo de concentração e uma precipitação de 1 mm, como mencionado anteriormente. O HUT tem a forma mostrada na Figura 5. O seu cálculo foi procedido da seguinte forma



- $t_r = 1/5 T_c = 1/5 \times 4h = 0,80h$

t_r = duração do excesso de chuva

- $t_p = 0.6T_c + 0.5t_r = 0.6 \times 4 + 0.5 \times 0.8 = 2,80h$

t_p = tempo até a vazão de pico

- $t_b = 2.76t_p = 2,67 \times 2,80 = 7,48h$

t_b = tempo de base do hidrograma

- $q_p = (2 \times P_e \times A_b) / t_b = (2 \times 0,001m \times 220 \times 10^6 m^2) / 7,48h \times 3600s = 16,34m^3 / s$

q_p = vazão de pico do hidrograma

P_e = excesso de precipitação = 1 mm

A_b = área da bacia hidrográfica = 220 km²

3.4.2 - Cálculo da Precipitação Excedente (P_e) para os Diversos Períodos de Retorno.

O cálculo da precipitação excedente foi feita através do método do "Curve Number" do SCS, utilizando-se a seguinte fórmula:

$$P_e = (P - 5080/N + 50,8)^2 / (P + 20320/N - 203,2), \text{ onde}$$

- P_e = Excesso de chuva ou Precipitação excedente em mm

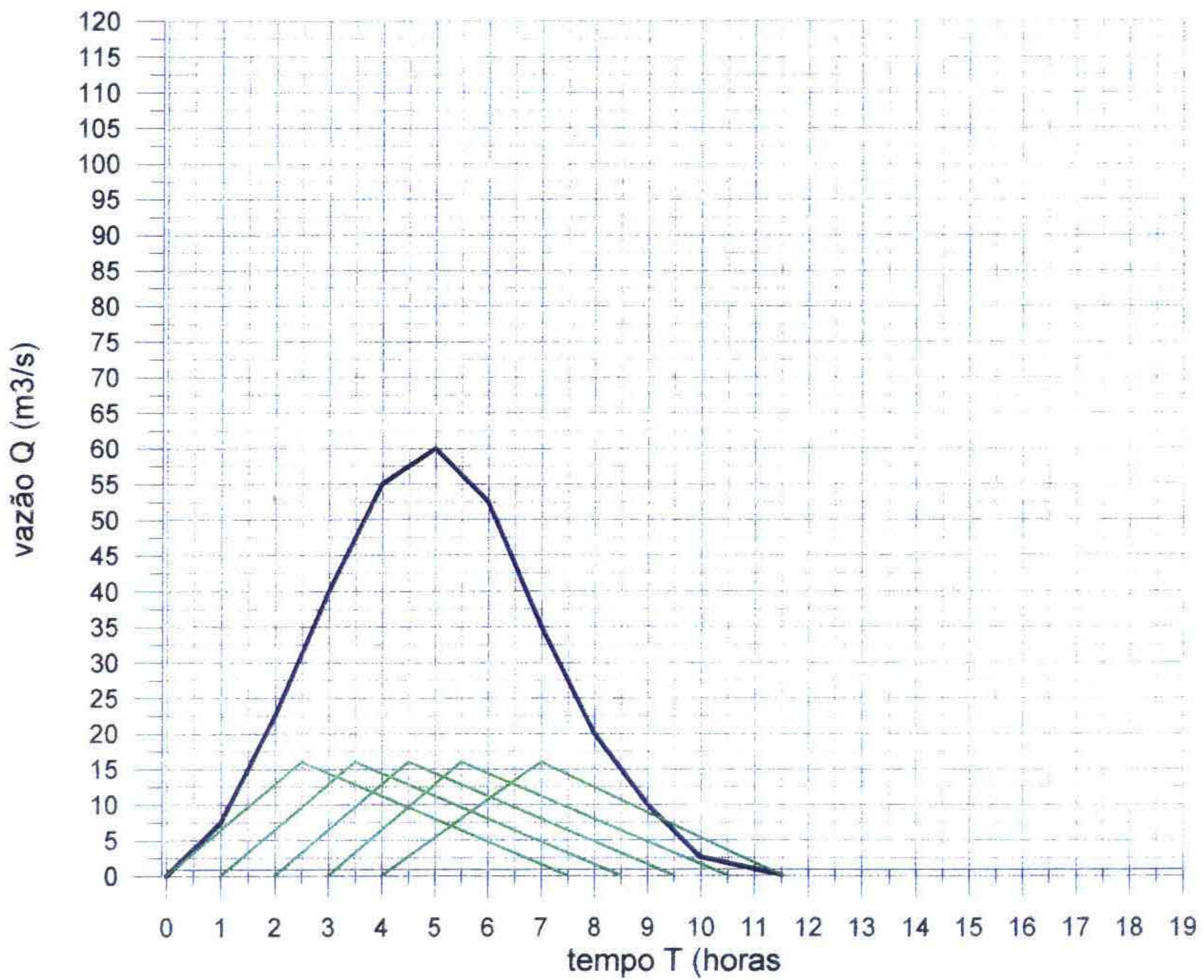
- P = Precipitação ou chuva de projeto em mm

- N = Número de deflúvio que define o complexo hidrológico solo x vegetação

Para as condições da área em estudo, obteve-se $N = 77$ (Solo Tipo C - vegetação, floresta pouco densa, caatinga arbustiva)

Aplicando-se a fórmula acima para as precipitações da **Tabela 7**, obteve-se o excesso de chuva para os diversos períodos de retorno em estudo. Os valores constam na **Tabela 8**

FIGURA 8 - HIDROGRAMA DE UMA CHUVA DE 5 mm E DURAÇÃO IGUAL AO TEMPO DE CONCENTRAÇÃO



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TABELA 10 - CHUVAS DE PROJETO

PERÍODO DE RETORNO (ANOS)	CHUVA DE PROJETO (MM)
500	71
100	79
500	96
1.000	103
10.000	126

TABELA 11
VALORES DE PRECIPITAÇÃO EXCEDENTE PARA PERÍODOS DE RETORNO DE 50, 100 500, 1.000 E 10.000 ANOS.

PERÍODO DE RETORNO (ANOS)	PRECIPITAÇÃO EXCEDENTE Pe (mm)
50	23,7
100	29,2
500	41,7
1.000	47,1
10.000	65,8

TABELA 12
HIDROGRAMAS AFLUENTES AO AÇUDE SOUZA PARA PERÍODOS DE RETORNO DE 50, 100, 500, 1.000 E 10.000 ANOS

TEMPO (HORAS)	VALORES DE Q EM M3/S				
	PERÍODOS DE RETORNO TR (ANOS)				
	50	100	500	1000	10000
0	0	0	0	0	0
1	38,92	47,95	68,47	77,34	111,76
2	108,36	133,50	190,65	215,34	311,76
3	199,74	246,10	351,45	396,96	570,59
4	274,26	337,90	482,55	545,04	776,47
5	292,88	360,85	515,33	582,06	823,53
6	257,33	317,05	452,78	511,41	717,65
7	176,04	216,90	309,75	349,86	488,24
8	99,87	123,05	175,72	198,48	247,06
9	47,40	58,40	83,40	94,20	129,41
10	11,85	14,60	20,85	23,55	35,29
10,88	0	0	0	0	0



3.4.3 - Cálculo do Hidrograma Total Afluente

O hidrograma total afluente foi calculado em duas fases

a) Calculou-se o hidrograma para uma chuva de duração igual ao tempo de concentração da bacia para uma altura de chuva efetiva igual a 5 mm (Ver **Figura 8**)

B) Posteriormente calculou-se o hidrograma para as chuvas de período de retorno de 50, 100, 500, 1 000 e 10 000 anos, multiplicando-se as ordenadas do hidrograma obtido na fase anterior, pela relação $Pe(T)/5$, onde $Pe(T)$ é a precipitação excedente (**Tabela 11**), correspondente ao período de retorno T. Os valores obtidos constam na **Tabela 12**, e estão plotados na **Figura 8**

3.4.4 - Determinação do Amortecimento de Cheia

Alterando-se a vazão de entrada de um reservatório de acumulação, a vazão de saída não é instantaneamente alterada do mesmo valor. O nível da água tende a manter-se horizontal e o acréscimo de vazão na entrada é traduzido em uma elevação da superfície líquida.

A vazão de saída deste reservatório fica afixada pela altura (h) acima da cota da soleira do vertedouro a qual determina também o volume de acumulação (v) e a área (A) do espelho d'água do reservatório (**Figura 9**)

Consideremos neste estudo a vazão sobre o vertedouro como a única saída do reservatório.

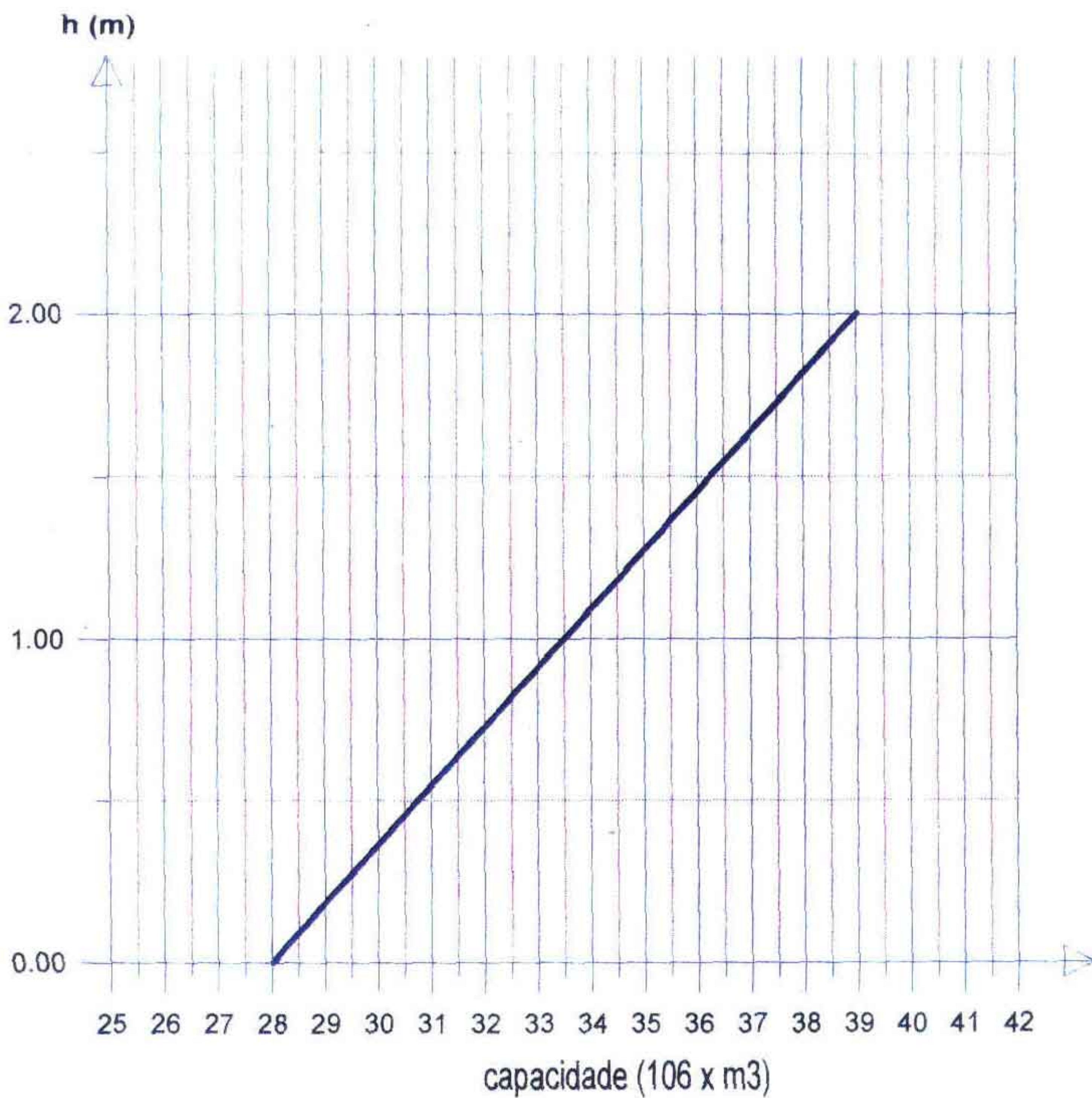
3.4.4.1 - Considerações Teóricas

Suponha-se que se queira calcular a hidrógrafa de saída $Q_s(t)$ de um reservatório criado por uma barragem cujo vertedor tem largura (L) e não tenha comportas. Os dados são

$Q_a(t)$, a relação $v(h) = a(h + b)^m$ e a relação $Q_s(h) = C L h^{3/2}$

Assim

FIGURA 9 - CURVA ALTURA D'ÁGUA - VOLUME





$Q_a - Q_s = \Delta V / \Delta t$ onde

$$Q_a = (Q_{ai} + Q_{ai+1}) / 2$$

$$Q_s = (Q_{si} + Q_{si+1}) / 2$$

$$\Delta V = V_{i+1} - V_i$$

$$\Delta t = t_{i+1} - t_i$$

Os índices (i) referem-se ao início do intervalo Δt , (tempo t_i) e os índices (i+1) ao fim do intervalo Δt , (tempo t_{i+1})

Substituindo os dados nas equações acima, tem-se

$$Q_a + a/\Delta t (h_i + b)^m - \frac{1}{2} C L h_i^{3/2} = a/\Delta t (h_{i+1} + b)^m + \frac{1}{2} C L h_{i+1}^{3/2} \quad (\text{eq. 1})$$

Fazendo-se

$$Y_1(h_i) = a/\Delta t (h_i + b)^m - \frac{1}{2} C L h_i^{3/2} \quad (\text{eq. 2})$$

$$Y_2(h_{i+1}) = a/\Delta t (h_{i+1} + b)^m - \frac{1}{2} C L h_{i+1}^{3/2} \quad (\text{eq. 3})$$

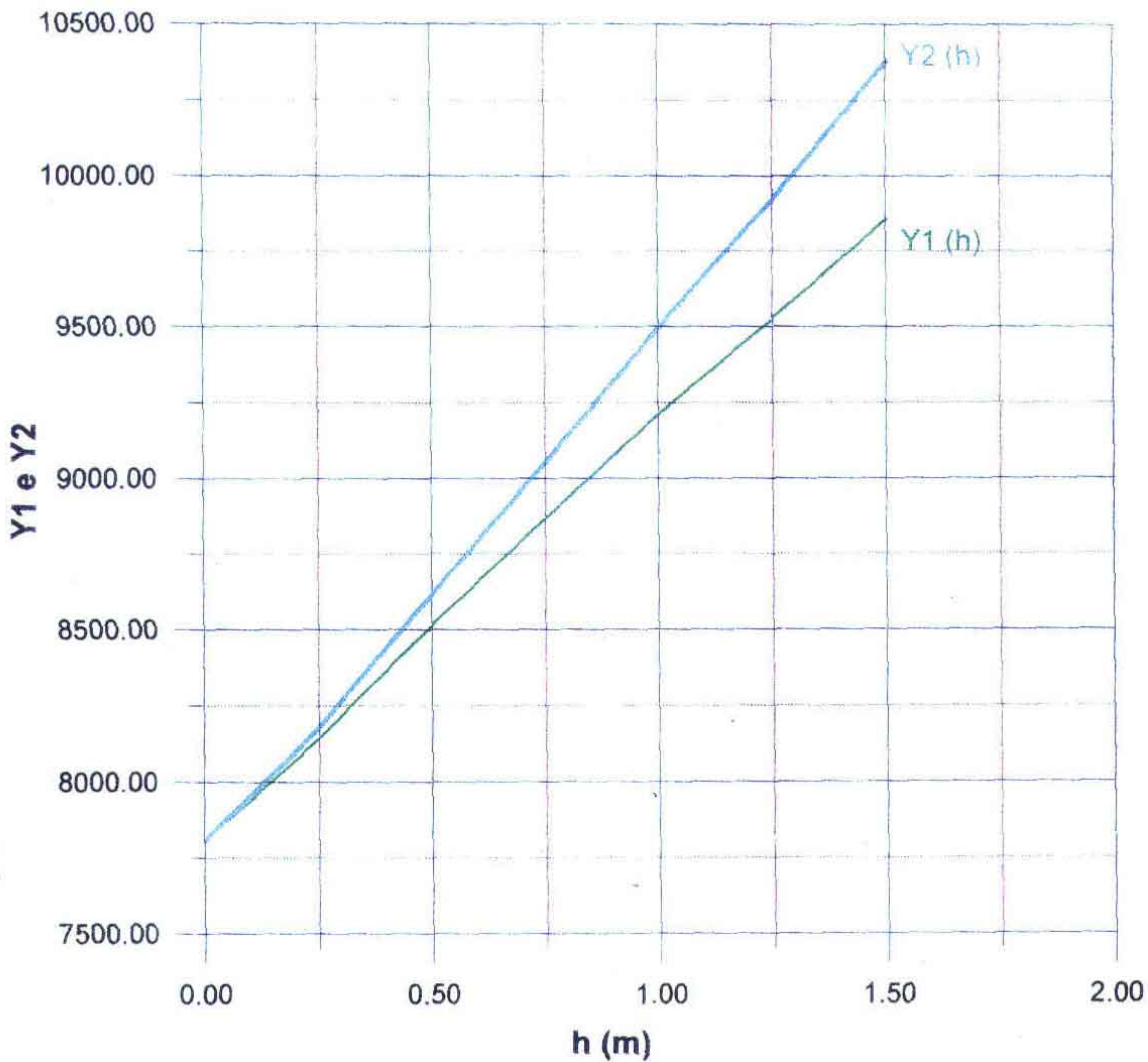
A equação (1) fica

$$Q_a + Y_1(h_i) = Y_2(h_{i+1}) \quad (\text{eq. 4})$$

Para maior facilidade das operações tabela-se $Y_1(h)$ e $Y_2(h)$, (**Figura 10**) ou faz-se gráficos

Iniciando com um valor de Q_a conhecido no intervalo de tempo $t_1 - t_0$ e um valor de $h = h_0$, pela equação (4) tira-se $Y_2(h_1)$ e pelo gráfico desta função (ou tabela) tira-se h_1 . Com esse valor, calcula-se $Q_{s1}(h_1)$. E assim sucessivamente, calcula-se $Q_s(h)$ que é o objetivo dessa integração

FIGURA 10 - FUNÇÕES Y1 (h) e Y2 (h)



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3.4.4.2 - Determinação das Hidrógrafas das Vazões Efluentes

Conhecendo-se as hidrógrafas das vazões afluentes (para períodos de retorno de 1 000 e 10 000 anos) plotadas na **Figura 11**, e sabendo-se que a vazão efluente é dada pela lei dos vertedores $Q = C L h^{3/2}$, pode-se determinar as hidrógrafas das vazões efluentes

Pela equação (2), tem-se

$$Y1(h) = V_i / \Delta t - \frac{1}{2} C L h_i^{3/2}$$

Fazendo $C = 2.19$, $L = 130,00\text{m}$ e $t = 1 \text{ hora} = 3\ 600 \text{ seg.}$, tem-se

$$Y1(h) = V_i / 3\ 600 - 142,35 h_i^{3/2}$$

Aplicando-se o mesmo procedimento na equação (3), tem-se

$$Y2(h) = V_i / 3\ 600 + 142,35 h_{i+1}^{3/2}$$

Substituindo os valores de h e V , tirados do gráfico da **Figura 10**, pode-se construir a **Tabela 13** e os gráficos da **Figura 10**

Para a determinação das hidrógrafas das vazões efluentes, utilizou-se o seguinte procedimento

a) Cálculo de h inicial

Para $t = 0$, $Q_a = Q_s = 0,00 \text{ m}^3/\text{seg}$

$$Q_s = C L h^{3/2} \quad 0,00 = 2,19 \times 130,00 \times h_o^{3/2} \quad H_o = 0,00\text{m}$$

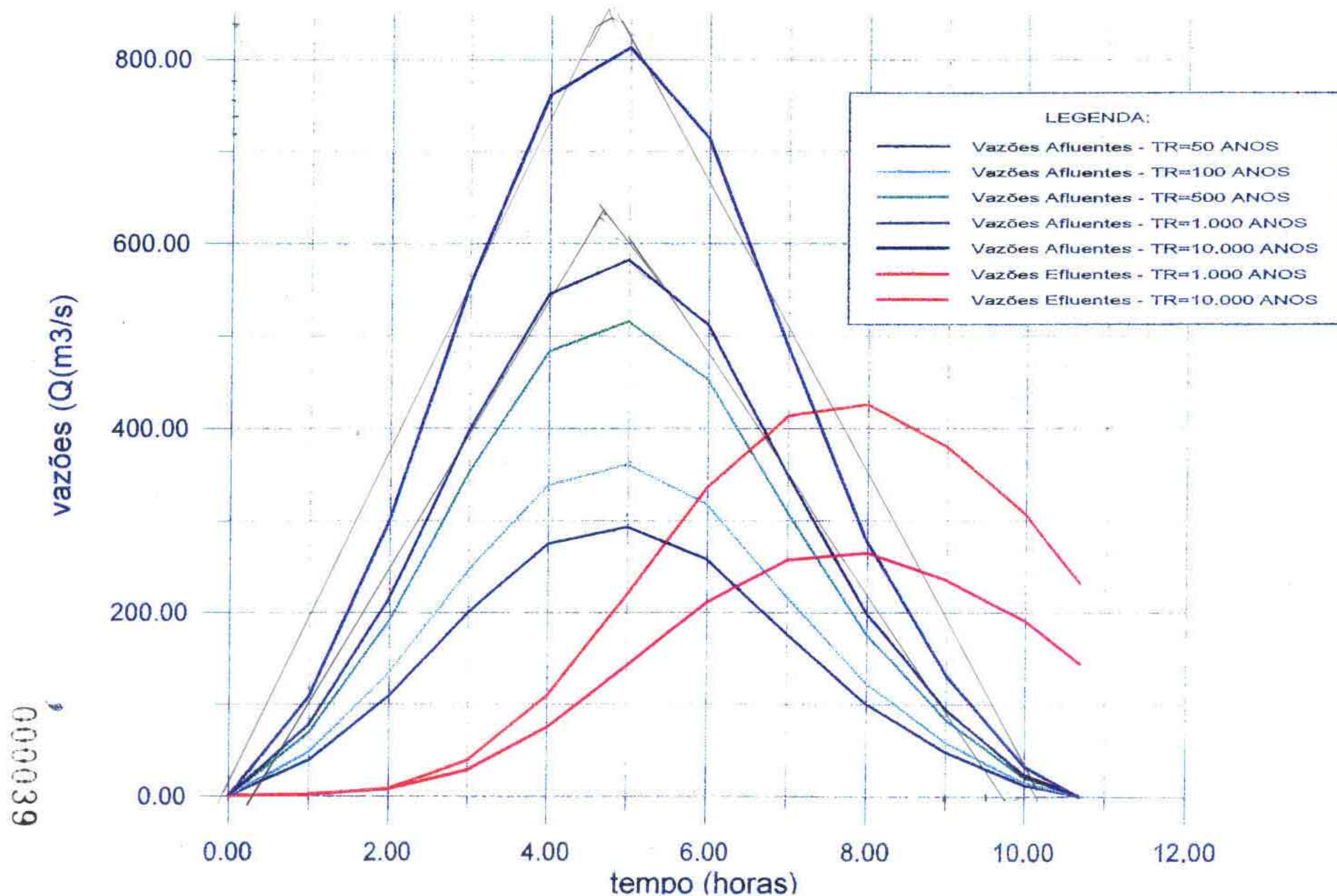
b) Determinação de Q_s

$$Y2(h) = Q_a + Y1(h) \quad (\text{eq 5})$$

$$Q_s + 142,35 (h_{i+1}^{3/2} - h_i^{3/2}) \quad (\text{eq 6})$$

$$Q_{s_{i+1}} = 2Q_s - Q_{s_i} \quad (\text{eq 7})$$

FIGURA 11 - HIDROGRAMAS TOTAIS AFLUENTES PARA AS CHUVAS DE PERÍODOS DE RETORNO DE 50, 100, 500, 1.000 e 10.000 ANOS E EFLUENTES PARA CHUVAS DE PERÍODO DE RETORNO DE 1.000 e 10.000 ANOS



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Roteiro de Cálculo:

- 1 Conhecido h pela **Figura 10**, determina-se $Y1 (h)$
- 2 Com $Y1 (h)$, pela equação (5), determina-se $Y2 (h)$
- 3 Com $Y2 (h)$, pela **Figura 10**, determina-se h_{i+1}
- 4 Com h_{i+1} , pela equação (6), determina-se Q_s
- 5 Com Q_s , pela equação (7), determina-se Q_{s+1}
- 6 Repete-se novamente as operações de 1 a 5, fazendo-se $h_i = h_{i+1}$

c) Substituindo-se os valores numéricos, construiu-se as **Tabelas 13 e 14**

D) Com os dados das **Tabelas 14 e 15**, plotou-se as hidrógrafas de vazões efluentes

Os valores encontrados nos conduzem aos resultados mostrados na **Tabela 16**

3.5 - Dimensionamento Hidrológico do Reservatório

3.5.1 - Metodologia

O dimensionamento hidrológico de açude Souza foi determinado, numa primeira alternativa, com base no método de Campos, 1987

O referido método apresenta um modelo gráfico aplicado a reservatórios de águas superficiais situadas em regiões com rios intermitentes sujeitos a altas taxas de evaporação. Tem como suporte teórico a Teoria Estocástica dos Reservatórios ou Teoria do Armazenamento de Moran. Segundo o autor, este método apresenta com novidade na Teoria, a introdução de uma matriz de evaporação por levar em conta as perdas devido a esse fenômeno

O modelo gráfico contempla as seguintes variáveis: capacidade do reservatório, volume anual regularizado e probabilidade de esvaziamento da reserva. As variáveis de entrada do modelo são: volume afluente médio anual, coeficiente de variação dos deflúvios anuais, lâmina de evaporação e um fator que representa a forma da bacia hidráulica



TABELA 13 : VALORES DE Y1(h) E Y2(h)

h (m)	v (10 ³ x m ³)	Y1	Y2
0,00	28.092	7.803,33	7.803,33
0,25	29.376	8.141,93	8.177,52
0,50	30.833	8.614,39	8.615,06
0,75	32.250	8.865,87	9.050,79
1,00	33.668	9.209,87	9.494,57
1,25	35.000	9.523,28	9.921,16
1,50	36.417	9.854,32	10.377,35

TABELA 14
VALORES DA HIDRÓGRAFA DA VAZÃO EFLUENTE PARA PERÍODO DE RETORNO DE 1.000 ANOS
E LARGURA DO SANGRADURO DE 130,00 M

t (h)	hi (m)	Y1 (h)	Qa (m ³ /seg)	Y2 (h)	h _{i+1}	Qs (m ³ /seg)	Qsi (m ³ /seg)	Qsi + 1
0 - 1	0	7.803,33	38,67	7.842,00	0,05	1,59	0	3,18
1 - 2	0,05	7.838,33	146,34	7.984,67	0,12	7,51	3,18	11,84
2 - 3	0,12	7.950,00	306,16	8.256,15	0,29	28,15	11,84	44,46
3 - 4	0,29	8.199,86	471,09	8.670,86	0,51	74,08	44,46	103,70
4 - 5	0,51	8.800,87	563,55	9.064,22	0,73	141,48	103,70	179,26
5 - 6	0,73	8.801,48	546,73	9.348,21	0,90	210,35	179,26	241,44
6 - 7	0,90	9.033,93	430,63	9.464,56	0,97	257,09	241,44	272,74
7 - 8	0,97	9.129,64	274,17	9.403,81	0,93	264,18	272,74	255,67
8 - 9	0,93	9.074,95	146,34	9.221,29	0,83	234,56	255,67	213,45
9 - 10	0,83	8.938,21	58,85	8.997,09	0,70	190,21	213,45	166,97
10 - 10,66	0,70	8.760,46	11,78	8.772,24	0,56	143,75	166,97	120,52



TABELA 15
VALORES DA HIDRÓGRAFA DA VAZÃO EFLUENTE PARA PERÍODO DE RETORNO DE 10.000 ANOS E LARGURA DO SANGRADOURO DE 130,00 m

t (h)	hi (m)	Y1 (h)	Qa (m3/seg)	Y2 (h)	h _{i+1}	Qs (m3/seg)	Qsi (m3/seg)	Qsi + 1
0 - 1	0	7.803,33	54,02	7.857,35	0,03	0,74	0	1,48
1 - 2	0,03	7.844,35	204,44	8.048,79	0,14	8,44	1,48	15,40
2 - 3	0,14	7.994,76	427,70	8.422,46	0,36	38,30	15,40	61,20
3 - 4	0,36	8.295,57	658,00	8.953,57	0,67	108,85	61,20	156,49
4 - 5	0,67	8.719,44	787,30	9.506,74	0,99	218,81	156,49	281,14
5 - 6	0,99	9.156,99	763,81	9.920,80	1,23	335,29	281,14	389,43
6 - 7	1,23	9.485,15	601,61	10.086,76	1,33	412,63	389,43	435,82
7 - 8	1,33	9.621,88	383,02	10.004,90	1,28	425,14	435,82	414,47
8 - 9	1,28	9.553,51	204,44	9.757,95	1,14	379,15	414,47	343,82
9 - 10	1,14	9.362,09	82,25	9.444,34	0,96	306,35	343,82	268,88
10 - 10,68	0,96	9.115,97	16,45	9.132,42	0,77	230,90	268,88	192,92

TABELA 16 : REDUÇÃO DA ONDA DE CHEIA NO ACUDE SOUZA

ONDA DE CHEIA (ANOS)	REDUÇÃO (%)
1.000	46,9
10.000	53,6



3.5.2 - Estudo das Disponibilidades

3.5.2.1 - Volume Afluente Médio Anual e Coeficiente de Variação

Da **Tabela 17**, Série Histórica de Vazões, obteve-se o volume afluente médio anual (μ) que é de 25 410 000 m³. O coeficiente de variação (cv) dos valores de deflúvios anuais é de 1,20.

A lâmina média de escoamento do rio Juriti foi calculada por correlação com a bacia representativa do reservatório de Pentecoste. A correlação foi obtida através da aplicação de dois coeficientes

$$C_0 = (AS/AP)^{-0.075} \text{ onde}$$

$$AS = \text{Área da bacia hidrográfica de Souza} = 220 \text{ Km}^2$$

$$AP = \text{Área da bacia hidrográfica de Pentecoste} = 3 330 \text{ Km}^2$$

$$C_1 = PS/PP \text{ onde}$$

$$PS = \text{precipitação média em Souza} = 643,8 \text{ mm}$$

$$PP = \text{precipitação média em Pentecoste} = 498,2 \text{ mm}$$

Então, a lâmina média escoada na bacia do açude Souza é igual a

$$LS = C_0 \times C_1 \times LP, \text{ onde:}$$

$$LS = \text{lâmina escoada em Souza em mm}$$

$$LP = \text{lâmina escoada em Pentecoste em mm}$$

$$LS = (220/3 330)^{-0.075} \times (643,78/498,2) \times 72,9 \text{ mm} = 115,50 \text{ mm}$$

O volume afluente médio anual (μ) é obtido através da relação

$$\mu = LS \times AS$$

$$\mu = 0,11550 \times 220 000 000 = 25 410 000,00 \text{ m}^3$$

O coeficiente de variação dos deflúvios anuais (cv) foi tomado igual ao reservatório de Pentecoste, isto é, cv = 1,20.



**TABELA 17 - SÉRIE HISTÓRICA DE VAZÕES AFLUENTES
RESERVATÓRIO PENTECOSTE**

ANOS	MESES												TOTAL
	JAN	FEV	MAR	ABR	MAI	JUN	JUL	AGO	SET	OUT	NOV	DEZ	
1912	25,3	272	445	339,3	370,7	29,3	4,9	0,3	0	0	0	0	1486,8
1913	0	259,1	521,4	86,7	117,6	9,5	4,1	0,3	0	0	0	23,1	1021,8
1914	18,9	1,8	32,9	40,3	7,4	4,2	2,7	0,1	0	0	0	0	108,3
1915	0	0	0	0	0	0	0	0	0	0	0	0	0
1916	0,1	0,3	200,7	64,7	172,2	59	6,4	0,5	0	0	0	0	503,9
1917	95	137,5	663,6	131,4	368,1	14,1	6,3	0,5	0	0	0	0	1416,5
1918	0	0,1	6,1	24	3,3	1,1	0	0	0	0	0	0	34,6
1919	21	0	0	0	0	0	0	0	0	0	0	0	21
1920	0	0	328,1	291,9	27,8	10,8	4,2	0,2	0	0	0	0	663
1921	0	110	544,5	380,6	543,3	12,2	6,5	1,3	0	0	0	0	1598,4
1922	0	0	0	146,7	40,3	5,6	2,5	0,1	0	0	0	25,6	220,8
1923	0	0	3,3	0,9	0,1	0	0	0	0	0	0	0	4,3
1924	213,2	130,3	683	892,2	224,9	36	9,9	1,5	0	0	0	0	2191
1925	0	0	90,4	77,8	75,3	6	0,8	0	0	0	0	0	250,3
1926	0	93,9	589,7	410,1	72,8	8,6	2	0	0	0	0	0	1177,1
1927	0	1,3	19,1	21,3	24,9	2,6	0,2	0	0	0	0	0	69,4
1928	0	0	0,3	0,9	3,8	0	0,2	0	0	0	0	0	5,2
1929	0	91,2	137,5	190,3	13,6	6,8	2,1	0	0	0	0	0	441,5
1930	0	0	0	15,9	2,3	0,3	0	0	0	0	0	0	18,5
1931	49,7	39,5	28,9	13,5	6,5	1,8	0	0	0	0	0	0	139,9
1932	0	79,9	3,4	1	0	0	0	0	0	0	0	0	84,3
1933	10,6	1	4,4	183,1	10,9	3,4	0,1	0	0	0	0	0	213,5
1934	0	133,5	319,2	110,1	141,7	11,5	3,7	0,1	0	0	0	0	719,8
1935	0	92,8	58,5	536,2	86,4	33,5	7,2	0,9	0	0	0	0	815,5
1936	0	0	0,1	0	0	0	0	0	0	0	0	0	0,1
1937	0	20,4	1,9	103,5	6,2	3,8	0,7	0	0	0	0	0	136,5
1938	0	0	117	371,9	14,7	10,8	3	0	0	0	0	0	517,4
1939	0	95,4	158,5	75,4	14,4	8,3	2,1	0	0	0	0	0	354,1



CONCREMAT
ENGENHARIA E TECNOLOGIA S.A

CONTINUAÇÃO

ANOS	MESES												TOTAL
	JAN	FEV	MAR	ABR	MAI	JUN	JUL	AGO	SET	OUT	NOV	DEZ	
1940	0	2,6	71,7	493,6	454,4	46,8	10,2	2,7	0	0	0	0	1082
1941	0	0	117,3	60,7	25	5,9	1,2	0	0	0	0	0	210,1
1942	0	2,9	18,4	13	2,8	0,3	0	0	0	0	0	0	37,4
1943	0	10	15,1	151,8	7,1	1,8	0	0	0	0	0	0	185,8
1944	0	0	13,8	21,9	6,9	3,6	0,2	0	0	0	0	0	46,4
1945	0	164,1	164,4	495,8	244,8	14,2	10,2	1,9	0	0	0	0	1095,4
1946	0	19,8	42,1	42	3,9	0,3	0	0	0	0	0	0	108,1
1947	0	5	160,4	121,5	282,5	9,2	2,9	0	0	0	0	0	581,5
1948	0	0	121,4	59,4	70,8	9,1	4,3	0,3	0	0	0	0	265,3
1949	0	0	60	144,3	225,7	12,1	4,7	0,3	0	0	0	0	447,1
1950	0	1,3	98,6	829,8	215,1	9,7	2,4	0	0	0	0	0	1156,9
1951	0	0	0	87,3	4	26,4	1	0	0	0	0	0	118,7
1952	0	0	15,6	70	13	2,8	0,1	0	0	0	0	0	101,5
1953	0	9,3	52,2	78	20,7	4,8	0,8	0	0	0	0	0	165,8
1954	0	17,1	34,4	5	7,7	2,2	0	0	0	0	0	0	66,4
1955	0	0,2	40	103,9	53,2	9,9	2,7	0	0	0	0	0	209,9
1956	0	41,1	147	276,3	12,5	4,9	1	0	0	0	0	0	482,8
1957	0	0	166,9	516,9	13,8	6,7	1,3	0	0	0	0	0	705,6
1958	0	0	0	0	0	0	0	0	0	0	0	0	0
1959	51	1,8	197,7	16,8	11,5	6	1,5	0	0	0	0	0	286,3
1960	0	0	205,6	10,1	6,5	1,5	0	0	0	0	0	0	223,7
1961	4,3	442,4	639,8	538,2	89,5	7,7	2,2	0	0	0	0	0	1724,1
1962	0	0	196,7	224,8	11,4	4,6	0,4	0	0	0	0	0	437,9
1963	9,8	1,4	219,2	263,8	11,5	3,5	0,1	0	0	0	0	0	509,3
1964	8,8	95,6	170,3	815,9	98,3	11	4,5	0,3	0	0	0	0	1204,7
1965	0	0	95,7	309,7	61,8	8,9	3,8	0,1	0	0	0	0	480
1966	0	0	0	0	0,3	0	0	0	0	0	0	0	0,3
1967	0	68,1	145	476,2	153,3	8,7	2,4	0	0	0	0	0	853,7
1968	1,8	0	131,3	105,4	254	12,1	3,2	0	0	0	0	0	507,8
1969	0	4,7	7,9	100,5	11,1	5,7	9,6	0	0	0	0	0	139,5
1970	8,2	0	13,7	27	3,2	0	0	0	0	0	0	0	52,1
1971	0	0	80,6	53,8	16,9	9,7	5	0,6	0	0	0	0	166,6

000045



CONCREMAT
ENGENHARIA E TECNOLOGIA S.A

CONTINUAÇÃO
MESES

ANOS	JAN	FEV	MAR	ABR	MAI	JUN	JUL	AGO	SET	OUT	NOV	DEZ	TOTAL
1972	0	0	0,1	0,1	0,6	0	0	0	0	0	0	0	0,8
1973	1,6	6,5	24,6	126,5	114,7	101,5	10,5	20,8	0	0	0	0	406,7
1974	0	113,2	477,1	678,1	529,1	289	21,7	8,3	0	0	0	0	2116,5
1975	8,9	7,1	57,3	38,6	40	0	0	0	0	0	0	0	151,9
1976	2,9	5,4	17,2	24,3	4,5	0	0	0	0	0	0	0	54,3
1977	7	19,7	49,7	26,3	58,7	16,1	8,2	0	0	0	0	0	185,7
1978	0	2,8	56,1	74,1	31,5	3,5	5,6	3,3	0	0	0	0	176,9
1979	0	0	4,4	0	7,3	0	0	0	0	0	0	0	11,7
1980	0	7,9	117,7	1,3	0	0	0	0	0	0	0	0	126,9
1981	0	0	166	57,2	2	0	0	0	0	0	0	0	225,2
1982	1,1	2,7	9,4	12,9	3,4	0	0	0	0	0	0	0	29,5
1983	0	0	0	0	0	0	0	0	0	0	0	0	0
1984	0	2,2	18,3	103,4	36,3	0	0	0	0	0	0	0	160,2
1985	7,6	180	595,4	631,5	154,2	29,2	17	8,5	8,4	3,1	1,5	9,8	1646,2
1986	3,3	73,4	316	384,1	128	28,5	11,8	7,1	0	0	0	0	952,2
1987	2,3	6,4	102,9	26,6	4,4	18	7,4	3	0	0	0	0	171
1988	8,5	5,2	29,7	299	226	10,1	8,8	1	0	0	0	0	588,3
1989	3,3	2,4	23	401,1	241	13,1	62,8	2,6	0	0	0	0	799
1990	0	0	1,3	0	11	0	0	0	0	0	0	0	12,3
TOTAL	564,2	2882,3	10466,5	13908,2	6335,1	1038,3	299,1	66,6	8,4	3,1	1,5	108,2	35681,5
MEDIA	6,6	33,9	121,7	163,6	75,4	12,2	3,6	0,8	0,1	0,0	0,0	1,3	424,8

000046



3.5.2.2 - Lâmina de Evaporação (Ev)

Os dados de evaporação média foram extraídos da publicação de Hargreaves, para a região de Canindé, e seus dados encontram-se na **Tabela 18**. A lâmina média anual evaporada é de 2 498 mm

3.5.2.3. - Fator de Forma da Bacia (α)

O fator de forma da bacia foi obtido através de correlação entre o volume (v) e a altura d'água (h), da curva COTA X VOLUME, pela equação $v = \alpha h^3$. O valor de α encontrado foi de 8 283

3.5.2.4 - Fator Adimensional de Evaporação (FE)

$FE = [3 \times (\alpha)^{1/3} \times Ev] / (\mu)^{1/3}$, onde

- Ev = lâmina evaporada durante a estação seca (JUN/JAN) - Ev = 1,792 m

- μ = volume afluente médio anual = 25 410 000 m³

Portanto FE = 0.40

3.5.2.5 - Relação Volume Regularizado X Capacidade de Reserva:

Utilizando-se os parâmetros FE = 0,40, Cv = 1.20 e $\mu = 25,41 \times 10^6$ m³ e aplicando a metodologia adotada, calculou-se a relação entre o volume, com garantias de 80, 85, 90 e 95%, e a capacidade do reservatório. Os valores estão apresentados na **Tabela 19**



TABELA 20 : VALORES DA EVAPORAÇÃO PARA A REGIÃO DE CANINDÉ

EVAPORAÇÃO MÉDIA mm												
JAN	FEV	MAR	ABR	MAI	JUN	JUL	AGO	SET	OUT	NOV	DEZ	TOTAL
241	199	186	161	160	165	193	218	234	251	244	246	2.498

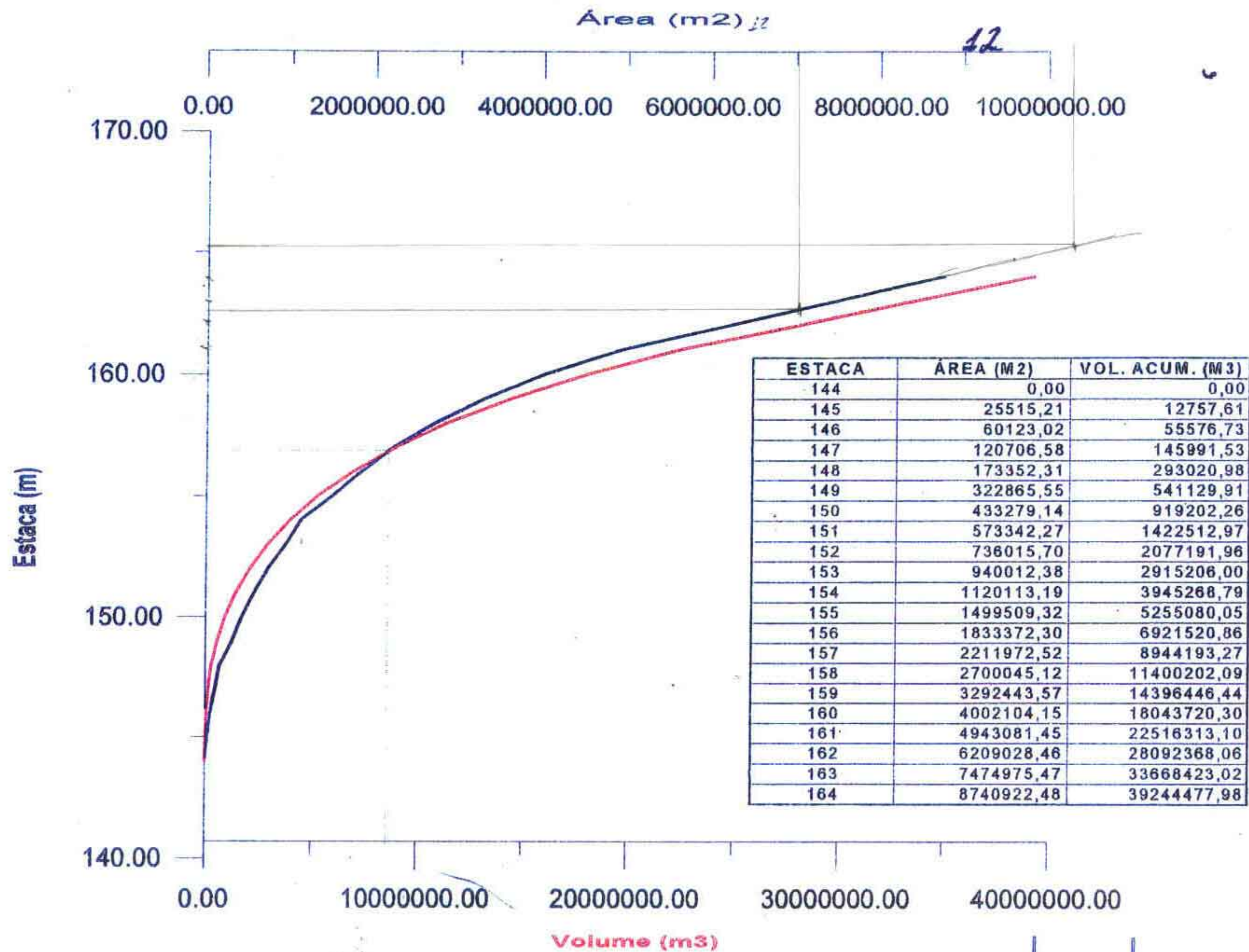
TABELA 21

VALORES DOS VOLUMES ANUAIS REGULARIZADOS COM GARANTIAS DE 80, 85, 90 E 95%, PARA O AÇUDE SOUZA

GARANTIAS (%)	CAPACIDADE DO AÇUDE (hm ³)	VOLUME REGULARIZADSO ANUAL (hm ³ /ANO)
80%	25,41	5,971
	28,00	6,302
	38,12	7,623
85%	25,41	5,209
	28,00	5,463
	38,12	6,480
90%	25,41	3,939
	28,00	4,243
	38,12	5,463
95%	25,41	2,541
	28,00	2,846
	38,12	4,116

OBS A capacidade de acumulação do açude Souza é de aproximadamente 30 000 000 m³. considerando a cota da soleira do sangradouro com 162,50 Portanto obtemos vazões regularizadas anuais de 6,566 hm³/ano, 5,666 hm³/ano, 4,487 hm³/ano e 3.100 hm³/ano, com garantias de 80, 85, 90, e 95% respectivamente (**Tabela - 21**)

CURVA COTA x ÁREA x VOLUME



000049

54000



3.6 - Estudo de Simulação do Reservatório

Visando o aprimoramento dos estudos efetuados no dimensionamento do reservatório (Açude Souza), resolvemos efetuar a simulação do balanço hídrico a partir das informações disponíveis, a seguir especificadas

- Série de vazões afluentes obtida mediante a utilização de modelo matemático MODHAC, de tipo chuva - deflúvio, com escala temporal mensal e dados de precipitação obtidos mediante técnicas Thissen - Monte Carlo.
- Dados médios mensais de evaporação e precipitação na vizinhança da bacia hidráulica ou em local hidrometeorologicamente semelhante
- Curva Cota - Área - Volume, decorrente dos estudos topográficos da bacia hidráulica (**Figura 12**)

A seguir são descritas as metodologias específicas utilizadas nas principais etapas

3.6.1 - Obtenção da Série Fluviométrica de Vazões Mensais

O modelo de transformação chuva - deflúvio utilizado foi o denominado MODHAC, este modelo foi desenvolvido no Instituto de Pesquisa Hidráulicas por Antônio E Lanna e M Schwarzbach ("MODHAC - Modelo Hidrológico Auto-Calibrável", IPH, UFRGS, Porto Alegre, 1988)

Este modelo simula o processo de transformação chuva-deflúvio de forma contínua, utilizando dados de precipitação diária e dando como resultado deflúvio mensais

O processo de separação do escoamento é realizado no modelo mediante uma analogia hidráulica composta de três reservatórios lineares, correspondente aos armazenamentos de água superficial, sub-superficial e subterrâneo. O modelo utiliza 9 parâmetros relacionados com as capacidades máximas dos referidos reservatórios e as taxas de esvaziamento dos mesmos



- Série de evapotranspirações potenciais diárias.

A deficiente disponibilidade de dados hidrometeorológicos no Estado do Ceará não permite a utilização de séries históricas de evapotranspirações potenciais necessárias para a simulação do ciclo hidrológico

Não obstante, a baixa variabilidade da evapotranspiração permite substituir as séries históricas por sequências diárias desta variável obtidas com base nas médias mensais de longo período calculada mediante o método de Hargreaves

A estação utilizada foi Canindé, com características hidroclimáticas semelhantes à bacia em estudo. Os valores mensais da evaporação estão apresentados na

Tabela 20



3.6.2 - Calibração do Modelo.

A simulação do ciclo hidrológico mediante modelos matemáticos do tipo do MODHAC exige a adoção de um conjunto de parâmetros que refletem, basicamente, as características da bacia hidrográfica, sua capacidade de armazenamento d'água, tanto a nível superficial, a nível de umidade na parcela superior do solo como também a nível de armazenamento subterrâneo profundo. Dado o caráter do modelo utilizado, estes parâmetros refletem também a "velocidade" ou fração d'água armazenada que é liberada em cada um dos armazenamentos na unidade de tempo da simulação.

O procedimento habitual para a seleção destes parâmetros consiste na calibração/validação deste conjunto de parâmetros mediante o confronto dos resultados obtidos na simulação (séries de vazões calculadas pelo modelo) e dados observados da realidade (série histórica de vazões observadas).

Este confronto é inexecutável no caso do local do barramento em estudo por carecer de dados fluviométricos no mesmo e, incluso em bacias vizinhas semelhantes à estudada.

Assim a alternativa escolhida para a seleção dos parâmetros foi utilizar o conjunto de parâmetros selecionados pelo PERH para simular bacias hidrográficas vizinhas.

O açude São Mateus, com bacia hidrográfica semelhante localizada também na alta bacia do Curu (vizinho à cidade de Canindé) foi objeto de simulação hidrológica nos estudos de base do PERH com um conjunto de parâmetros obtidos a partir de calibrações do modelo MODHAC para o posto fluviométrico denominado São Luís do Curu, localizado na mesma bacia hidrográfica mas com área da bacia hidrográfica com ordem de grandeza superior à estudada.

Este critério foi mantido na ausência de melhores subsídios para obtenção do conjunto de parâmetros do modelo. A **Tabela 22**, apresenta os nomes, uma breve descrição e os valores dos parâmetros utilizados na simulação.



TABELA - 22

Parâmetro	Descrição	Valor adotado
RSPX	Capacidade máxima do reservatório superficial	85 30
RSSX	Capacidade máxima do reservatório subsuperficial	182 50
RSBX	Capacidade máxima do reservatório subterrâneo	0 00
IMIN	Infiltração mínima	4 26
IMAX	Permeabilidade do solo	16 14
IDEC	Coefficiente de infiltração	0 5404
ASB	Exponente da lei de esvaziamento do reservatório subterrâneo	0 0
CEVA	Parâmetro da lei que controla a evapotranspiração real do solo	0 3101
ASP	Exponente da lei de esvaziamento do reservatório superficial	0 0001
ASS	Exponente da lei de esvaziamento do reservatório subsuperficial	0 6276

3.6.3 - As Séries Geradas.

O conjunto de parâmetros obtidos mediante calibração representa as características hidrológicas da região em estudo. O único aspecto a ressaltar é com referência à escala da bacia em estudo e a bacia onde foram obtidos os parâmetros do modelo.

Em termos gerais duas bacias morfológica e pedologicamente comparáveis apresentam variação exclusivamente nos parâmetros que refletem o traslado das vazões nos cursos de água.

Se o intervalo da simulação supera amplamente os tempos de concentração de ambas bacias este aspecto resulta irrelevante. Assim os parâmetros obtidos para a bacia do posto fluviométrico de São Luís do Curu podem ser utilizados na geração de séries temporais de vazões para a bacia da Barragem de Souza sempre que se mantenham os intervalos de simulação mensais.

A série temporal gerada foi de uma extensão de 76 anos (1913/1988) período para o qual se dispunha de dados pluviométricos na região. A **Tabela 23** e a **Figura 13** apresentam os resultados obtidos mediante a simulação.



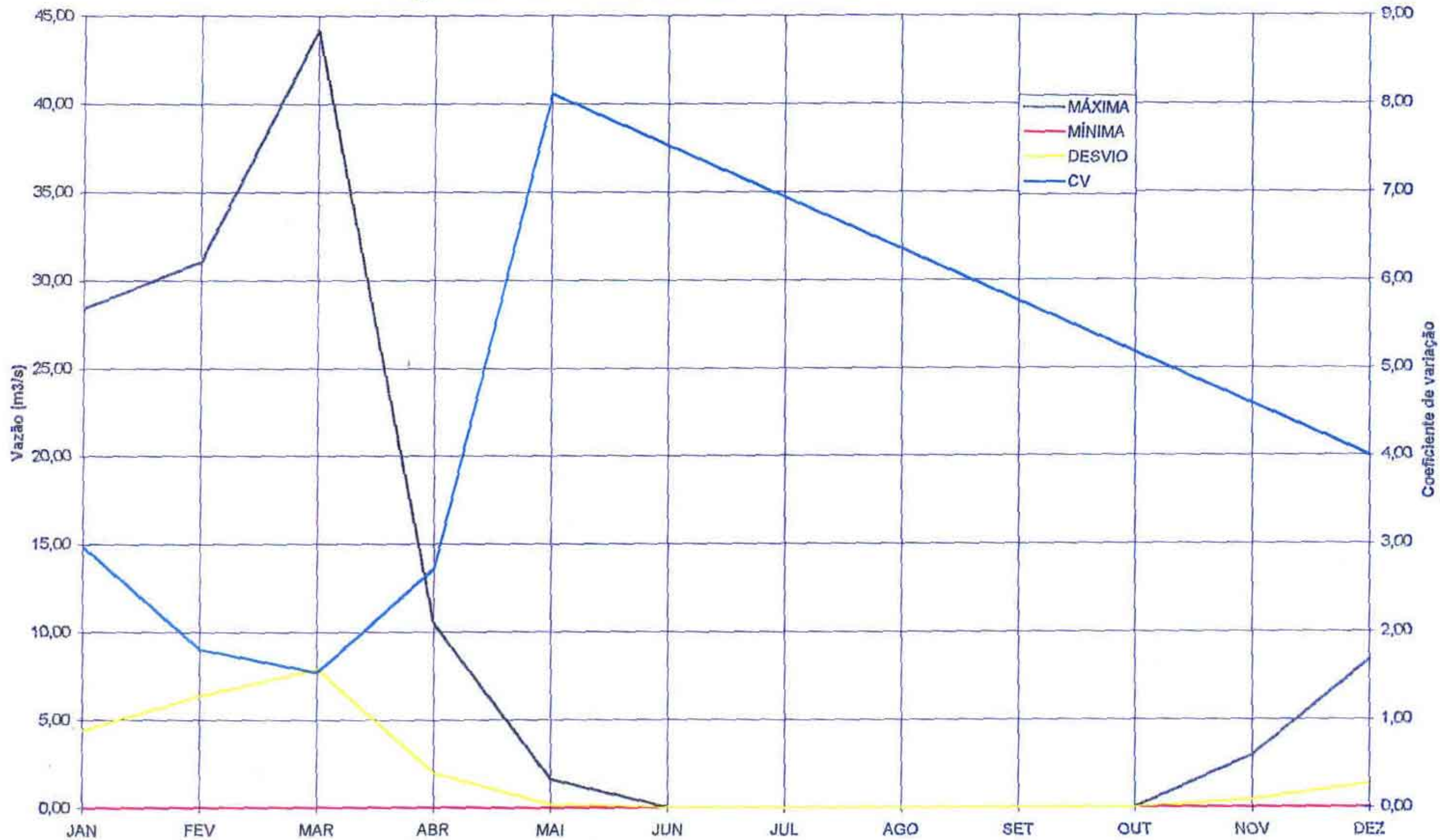
Tabela 23 Vazão média mensal da bacia do reservatório de Souza (m3/s)

ANO	JAN	FEV	MAR	ABR	MAI	JUN	JUL	AGO	SET	OUT	NOV	DEZ	Q. ANUAL	<Med	NP
912	9,3	5,01	8,32	0,13	0,01	0	0	0	0	0	0	0	1,892		
913	4,25	4,9	13,74	0,58	0,01	0	0	0	0	0	2,43	6,41	2,866		
914	0,03	0	0	0	0	0	0	0	0	0	0	0	0,003	b	
915	0	0	0	0	0	0	0	0	0	0	2,92	0	0,245	b	
916	0	4,31	0,08	2,79	0	0	0	0	0	0	0	0	0,567	b	3
917	5,2	10,7	3,32	5,35	0,01	0	0	0	0	0	0	0	1,885		
918	0	0	0,01	0,01	0	0	0	0	0	0	0	0	0,002	b	
919	0	0	0	0	0	0	0	0	0	0	0	0	0,000	b	2
920	0	5,88	8,78	0,01	0	0	0	0	0	0	0	0	1,175		
921	0,01	0,01	0,01	0,99	0,01	0	0	0	0	0	0	0	0,985	b	1
922	0	0	14,95	0,04	0	0	0	0	0	0	0	0	1,273		
923	0	0,01	3,21	0	0	0	0	0	0	0	0	0,62	0,326	b	1
924	14,32	0,56	18,27	10,47	0,03	0	0	0	0	0	0	5,9	4,175		
925	0,13	0	0,01	0,01	0	0	0	0	0	0	0	0	0,013	b	1
926	4,3	18,15	13,31	0,05	0	0	0	0	0	0	0	0	2,892		
927	0	0,01	0,01	0	0	0	0	0	0	0	0	0	0,002	b	
928	0	8,67	2,27	0,01	0	0	0	0	0	0	0	0	0,874	b	2
929	5,96	8,77	5,85	0,01	0	0	0	0	0	0	0	0	1,677		
930	0	2,78	7,83	0,01	0	0	0	0	0	0	0	0	0,888	b	
931	0	0,01	0,01	0	0	0	0	0	0	0	0	0	0,002	b	
932	0	0	0	0	0	0	0	0	0	0	0	0	0,000	b	
933	2,2	0,01	2,12	0,34	0	0	0	0	0	0	0	0	0,396	b	
934	0,21	6,39	1,13	0,01	0	0	0	0	0	0	0	0	0,605	b	5
935	9,33	0,23	10,34	1,26	0	0	0	0	0	0	0	0	1,792		
936	7,68	0,22	0	0	0	0	0	0	0	0	0	0	0,669	b	
937	0	0	5,05	0,01	0	0	0	0	0	0	0	0	0,431	b	
938	0	6,13	4,91	0,01	0	0	0	0	0	0	0	0	0,888	b	
939	0	0,01	0,1	0,01	0	0	0	0	0	0	0	0	0,010	b	4
940	0,01	9,48	3,9	0,35	1,63	0	0	0	0	0	0	0	1,225		
941	2,67	4,62	0	0	0	0	0	0	0	0	0	0	0,597	b	
942	0	0	0	0	0	0	0	0	0	0	0	0	0,000	b	
943	0	0	0	0	0	0	0	0	0	0	0	0	0,000	b	
944	0	0	0	0	0	0	0	0	0	0	0	0	0,000	b	
945	0,01	0,01	0,21	6,71	0	0	0	0	0	0	0	0	0,571	b	6
946	0	0	0	0	0	0	0	0	0	0	0	0	0,000	b	
947	0	12,26	5,77	0,01	0	0	0	0	0	0	0	0	1,431		
948	0	5,6	3,99	2,04	0	0	0	0	0	0	0	0	0,936		
949	0	0	4,31	0,01	0	0	0	0	0	0	0	0	0,367	b	1
950	0	3,88	12,43	0,03	0	0	0	0	0	0	0	0	1,356		
951	0	0	0,01	0	0	0	0	0	0	0	0	0	0,001	b	
952	0	0	0,01	0	0	0	0	0	0	0	0	0	0,001	b	
953	0	0	0	0	0	0	0	0	0	0	0	0	0,000	b	
954	0	0	0,01	0	0	0	0	0	0	0	0	0	0,001	b	
955	0,59	0	0,01	0,01	0	0	0	0	0	0	0	0	0,052	b	
956	0	0	1,7	0,01	0	0	0	0	0	0	0	0	0,145	b	6
957	0	14,09	21,68	0,86	0	0	0	0	0	0	0	0,31	3,018		
958	2,29	0	0	0	0	0	0	0	0	0	0	0	0,194	b	
959	0	0	0	0	0	0	0	0	0	0	0	0	0,000	b	2
960	0	31,07	0,75	0,01	0	0	0	0	0	0	0	3,45	2,744		
961	0,01	4,4	1,75	0,01	0	0	0	0	0	0	0	0	0,488	b	
962	0,01	0	0,01	0,01	0	0	0	0	0	0	0	0	0,003	b	2
963	0,6	11,43	10,33	0,01	0	0	0	0	0	0	0	0	1,806		
964	4,12	12,06	10,49	3,62	0,02	0	0	0	0	0	0	0	2,472		
965	0	0	4,81	0,01	0,01	0	0	0	0	0	0	0	0,410	b	
966	0	0	2,69	0,01	0	0	0	0	0	0	0	0	0,229	b	2
967	0	0	13,12	0,54	0	0	0	0	0	0	0	0	1,159		
968	0	15,3	1,56	5,06	0,01	0	0	0	0	0	0	0	1,723		
969	0	1,59	1,72	0	0	0	0	0	0	0	0	0	0,268	b	
970	0	0	0	0	0	0	0	0	0	0	0	0	0,000	b	
971	0	0	3,23	0,74	0	0	0	0	0	0	0	0	0,335	b	
972	0	0	0	0	0	0	0	0	0	0	0,52	0,01	0,044	b	4
973	0	0,01	22,58	0,01	0	0	0	0	0	0	0	5,02	2,346		
974	0,15	0,01	44,16	9,08	0,01	0	0	0	0	0	0	0	4,513		
975	0	0,01	0,01	0	0	0	0	0	0	0	0	0,25	0,025	b	1
976	28,39	17,42	0	0	0	0	0	0	0	0	0	0	3,748		
977	0	0,01	9,07	0,24	0	0	0	0	0	0	0	0	0,791	b	1
978	0	0,65	16,15	1,1	0	0	0	0	0	0	0	0	1,512		
979	0	0	0	0,01	0	0	0	0	0	0	0	0	0,046	b	
980	1,57	0	0	0	0	0	0	0	0	0	0	0	0,159	b	
981	0	1,57	4,25	0,03	0	0	0	0	0	0	0	0	0,482	b	
982	0	0,8	0,75	0	0	0	0	0	0	0	0	0	0,125	b	4
983	0,01	0	11,79	0	0	0	0	0	0	0	0	0	1,092		
984	0	28,71	30,37	0,37	0,01	0	0	0	0	0	0	1,02	4,900		
985	17,75	0,01	15,4	1,54	0	0	0	0	0	0	0	0	2,945		
986	0,01	3,27	8,01	0,01	0	0	0	0	0	0	0	0	0,933		
987	0	4,8	0,47	0	0	0	0	0	0	0	0	0	0,408	b	1
988	0	7,44	5,45	0,03	0	0	0	0	0	0	0	0	1,039		
MEDIA	1,476	3,531	5,110	0,717	0,023	0,000	0,000	0,000	0,000	0,000	0,078	0,337	0,328	soma	49
MAXIMA	28,390	31,070	44,180	10,470	1,630	0,000	0,000	0,000	0,000	0,000	2,980	8,410	4,900	média	2,579
MINIMA	0,000	0,000	0,000	0,000	0,000	0,000	0,000	0,000	0,000	0,000	0,000	0,000	0,000	desvio	1,742
DESVIO	4,390	6,365	7,888	1,954	0,187	0,000	0,000	0,000	0,000	0,000	0,441	1,347	1,171		
CV	2,98	1,80	1,54	2,72	8,11							4,00	1,26		
COEFIC. DE ESCOAMENTO				0,2045											
DEFLUVIO MEDIO (hm3/ano)				29,69261											
DEFLUVIO MAXIMO (hm3/ano)				157,0913											
DEFLUVIO MINIMO (hm3/ano)				0,00											



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Figura 13 - Parâmetro da série de vazões médias mensais



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3.7 - Determinação da Capacidade do Reservatório e da Vazão Regularizada.

O dimensionamento hidrológico de reservatórios superficiais visa obter uma primeira aproximação da capacidade do reservatório e, associada à mesma, uma dada vazão regularizada.

O tamanho do reservatório indica, de forma indireta, os custos associados com a estocagem de água, pois define a altura e outras dimensões da barragem a ser construída. O valor da vazão regularizada correspondente representa o benefício obtido com esta ação.

Os valores obtidos neste estudo serão a base para a análise econômica do empreendimento.

3.7.1 - Operação do Reservatório.

A análise preliminar da topografia e morfologia detalhada do boqueirão de Sousa qualificaram esta barragem como ponto apropriado para realizar estudos mais detalhados.

A oferta d'água de um reservatório corresponde a vazão por ele regularizada associada a uma determinada garantia.

Para sua avaliação, o procedimento consiste na simulação da operação do reservatório utilizando a série histórica de vazões afluentes e/ou séries estocasticamente geradas por modelos apropriados.

A simulação baseia-se na equação do balanço hídrico num reservatório, dada por

$$V_{t+1} = V_t + Q_a - E \left(\frac{A_t + A_{t-1}}{2} \right) - Q_{ab}$$

Sempre que o reservatório apresenta um volume inferior à retirada desejada, apenas uma parte desta é satisfeita, sendo esse evento denominado falha.

A vazão regularizada está portanto associada a um determinado nível de garantia avaliado em função do número de falhas (f) e do número de períodos simulados (n) como sendo

$$g = \left(1 - \frac{f}{n} \right) 100$$



O nível de garantia adotado usualmente em estudos hidrológicos no Brasil é de 90%. Os reservatórios da região semi-árida do nordeste do Brasil apresentam uma estrutura temporal de ocorrência de falhas caracterizada por uma alta concentração daquelas nos períodos secos, ou seja quando ocorre uma falha esta geralmente nunca ocorre isoladamente. Para o caso da barragem de Sousa a série de vazões afluentes de 76 anos apresenta 49 anos com deflúvios inferiores à média e estes valores se distribuem segundo períodos que tem por média 2.6 anos de duração (ver **Tabela 23**)

As falhas que estes reservatórios apresentam podem estender-se ao longo de vários meses, ou anos, o que coloca em xeque o conceito de vazão regularizada com 90% de garantia.

Nestes casos é bastante arriscado contar com uma vazão com garantia de 90%, dado que durante os restantes 10% todas as atividades sócio-econômicas da região abastecidas pelos reservatórios podem entrar em colapso. Justifica-se portanto a introdução de um outro conceito que, traduzido em regra de operação, venha a minorar tais riscos (PERH-CE, 1991).

Trata-se do conceito de volume de alerta de um reservatório, este corresponde ao volume a partir do qual apenas um percentual da vazão regularizada pode ser retirada. A introdução deste conceito na regra de operação resulta em

$$Q_{ab} = \begin{cases} Q_r \Leftrightarrow V_{i+1} \geq V_a \\ \%Q_r \Leftrightarrow V_a \geq V_i \geq V_{min} \\ 0 \Leftrightarrow \left[V_i + Q_a - E \left(\frac{A + A_{i-1}}{2} \right) \right] \leq V_{min} \\ Q_r < \%Q_r \Leftrightarrow V_{i+1} \geq V_{min} \end{cases}$$
$$V_i \leq V_{max} \rightarrow i = 1, 2, \dots, n$$

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onde V_{i+1} , V_i , A_{i+1} , A_i são, respectivamente, os volumes e as áreas do espelho d'água do reservatório para os instantes de tempo i e $i+1$ da simulação, Q_{ab} é a vazão retirada, E_i é a parcela de evaporação, Q_a é a vazão afluente ao reservatório, todas respectivamente no intervalo de tempo i da simulação, V_{min} e V_{max} , respectivamente o volume mínimo operacional e a capacidade máxima do



reservatório. Qv, a vazão de falhas e n o número de intervalos de tempo simulados

Esta foi a metodologia adotada para a obtenção da vazão regularizada Os critérios de volume de alerta estão associados ao suprimento de uma vazão 50% inferior à vazão regularizada

Procedeu-se ao cálculo da curva de vazões regularizadas utilizando como base a série temporal de vazões afluentes geradas mediante o modelo MODHAC e um modelo de balanço de massa de reservatórios

Foram pesquisados capacidades diferentes do reservatório utilizando-se o NEC 1 a **Tabela 24**, apresenta os resultados obtidos

Tabela 24

Vazões regularizadas para diferentes capacidades do reservatório de Sousa.

VOLUME (m ³)	COTA M	Q 90(SEM VA/(m ³ /s))	Q90(COMVA/M ³ S)
18 043 720	160,00	0,207	0,126
20 280 020	160,50	0,229	0,148
22 516 310	161,00	0,241	0,156
25 304 340	161,50	0,270	0,175
28 092 370	162,00	0,284	0,196
30 880 400	162,50	0,303	0,206
33 668 420	163,00	0,334	0,231
36 456 450	163,50	0,352	0,252
39 244 480	164,00	0,361	0,261

3.7.2 - Simulação Hidrológica - Hidráulica do Evento Extremo.

3.7.2.1 - Simulação Hidrológica - Hidráulica do Evento Extremo - Metodologia.

Os modelos de transformação chuva - deflúvio para a simulação de eventos extremos diferem sensivelmente dos modelos utilizados para geração de séries temporais de vazões

Em primeiro lugar o intervalo de tempo da simulação deve ser o suficientemente pequeno como para permitir refletir o pico da cheia gerada pelo evento extremo Por outro lado o processo de infiltração de água no solo é simulado mediante

métodos que se limitam a separar a parcela infiltrada da água disponível para escoamento direto

O modelo matemático utilizado para este estudo foi o HEC - 1. as metodologias utilizadas foram as seguintes

- Composição de um hietograma de chuvas intensas

O critério seguido para a composição deste hietograma foi o seguinte

O menor intervalo de tempo com estimativa de chuvas intensas compõe a parte central do hietograma, os valores de intensidade são calculados de forma tal que seja possível encontrar para cada intervalo de tempo uma intensidade de chuva igual à calculada como máxima para aquela duração e para uma dado período de retorno, este procedimento encontra-se incorporado ao modelo HEC-1

- Separação do Escoamento mediante o método TR-55 do SCS, conhecido também como Método do Número de Curva de perdas ou "Curve Number"

Com base no zoneamento de solos realizado durante PERH procedeu-se a identificação dos valores de CN correspondentes. O valor de CN estimado para toda a bacia foi de 74, este valor corresponde a áreas com predominância de solos rasos e pouco permeáveis (tipo D) com precipitações antecedentes que ainda não atingiram a saturação. Optou-se por este valor, mesmo considerado levemente baixo, porque durante o evento extremo a ser simulado seria superado amplamente o intervalo de chuva correspondente ao "tempo de concentração da bacia", saturando completamente o solo às primeiras 6hs do evento

Os valores iniciais de CN foi de 77, tendo-se optado agora por valores mais conservadores e que não afetem os resultados finais



- Simulação do escoamento na bacia e nos canais principais mediante onda cinemática

A bacia hidrográfica em estudo foi subdividida em 19 sub-bacias, a **Figura 07** apresenta a discretização em sub-bacias e planos de escoamento utilizada na simulação da bacia hidrográfica boqueirão de Souza

Os valores de declividade das áreas de escoamento não hierarquizado (denominadas "overlands") e dos canais naturais foram estimados mediante análise da cartografia disponível (cartas em escala 1:100 000). Os valores de rugosidade para canais naturais e para "overlands" foi estimado a partir de análise "in loco" e valores de tabelas obtidos em vários locais e estudos anteriores. Estes parâmetros são apresentados na **Tabela 21**

- Composição dos hidrogramas das sub-bacias

A composição dos Hidrogramas das sub-bacias foi obtido mediante o método da onda cinemática que consiste basicamente na solução do sistema de equações diferenciais composto pela equação de continuidade, tanto nas seções de stream como numa seção de overland, e a equação de Manning como relação funcional entre o tirante hidráulico e a vazão

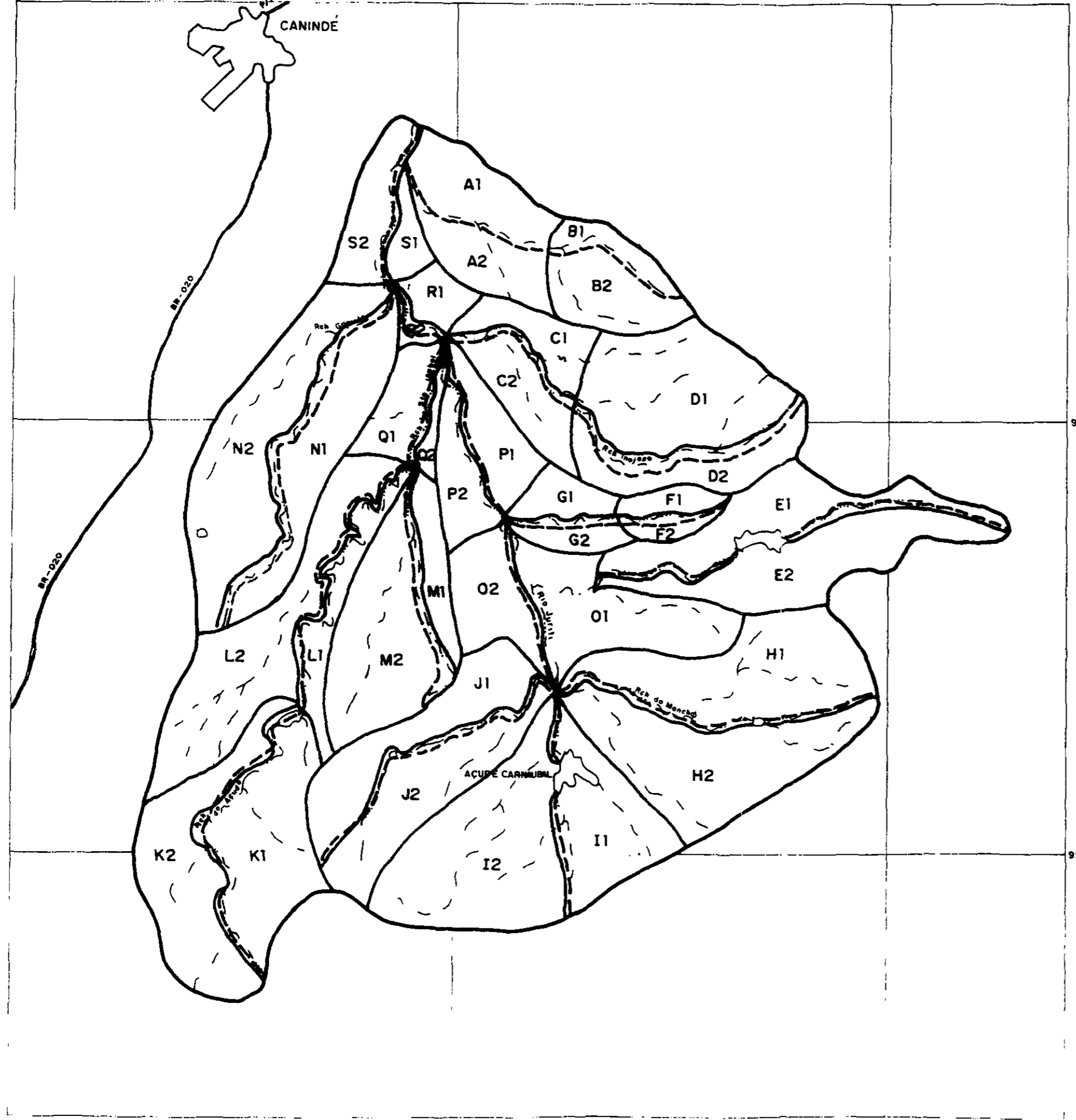
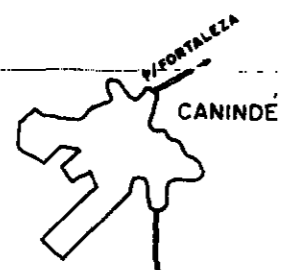
- Simulação da passagem da cheia pelo reservatório

A simulação da passagem da cheia pelo reservatório foi realizada pelo método de Puls, este método resolve a equação de continuidade ($Q_i - Q_o = dS/dt$), mediante uma discretização com intervalo de tempo igual ao da hidrógrafa afluente (neste caso 5 min), as condições iniciais foram, reservatório cheio até a cota da soleira do vertedouro e vazão efluente inicial nula

460

470

480



SUB-BACIA	ÁREA Km ²
A1	5.1
A2	4.8
B1	2.2
B2	5.0
C1	4.4
C2	4.0
D1	12.5
D2	4.2
E1	7.2
E2	10.5
F1	2.1
F2	1.3
G1	3.3
G2	2.2
H1	11.4
H2	13.3
I1	7.6
I2	14.1
J1	8.1
J2	7.4
K1	10.6
K2	11.1
L1	4.2
L2	13.1
M1	4.6
M2	9.8
N1	8.3
N2	13.6
O1	9.9
O2	5.3
P1	4.6
P2	4.7
Q1	2.5
Q2	3.4
R1	2.6
R2	1.5
S1	1.5
S2	3.1
BACIA	219.3

LEGENDA

- LIMITE DE BACIA
- LIMITE DE SUB-BACIA
- RIO OU RIACHO
- BR

000001

FIGURA 7

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**BARRAGEM SOUZA
MAPA DA BACIA HIDROGRÁFICA**

DATA	ESCALA
MAIO / 96	1 / 100 000

000001



Tabela - 24 : Parâmetros utilizados no modelo de onda cinemática.

NOME DA SUB-BACIA	OVERLAND	COMPRIMENTO DOS STREAMS (km)	DECLIVIDADES DOS STREAMS (ad)	ÁREAS DAS SUB-BACIAS (km ²)	COMPRIMENTO DAS SUB-BACIAS (km)	DECLIVIDADE DAS SUB-BACIAS (ad)
A	1	5	0,0133	4,56	0,91	0,040
	2				4,29	0,96
B	1	4	0,1000	1,97	0,49	0,080
	2				4,47	1,12
C	1	4,5	0,0080	3,94	0,87	0,040
	2				3,58	0,80
D	1	6	0,0150	11,18	1,86	0,120
	2				3,76	0,63
E	1	8	0,0080	6,44	0,81	0,080
	2				9,39	1,17
F	1	1,5	0,0100	1,88	1,25	0,016
	2				1,16	0,78
G	1	3	0,0100	2,95	0,98	0,016
	2				1,97	0,66
H	1	9	0,0133	10,20	1,13	0,027
	2				11,90	1,32
I	1	4	0,0062	6,80	1,70	0,027
	2				12,62	3,16
J	1	6	0,0044	7,25	1,21	0,020
	2				6,62	1,10
K	1	7,1	0,0160	9,48	1,34	0,063
	2				9,93	1,40
L	1	8,5	0,0047	3,76	0,44	0,027
	2				11,72	1,38
M	1	5,5	0,0133	4,12	0,75	0,016
	2				8,77	1,69
N	1	9,5	0,0053	7,43	0,78	0,020
	2				12,17	1,28
O	1	6,5	0,0062	8,86	1,36	0,020
	2				4,74	0,73
P	1	5	0,0050	4,12	0,82	0,020
	2				4,21	0,84
Q	1	3	0,0050	2,24	0,76	0,016
	2				3,04	1,01
R	1	2	0,0050	2,33	1,16	0,016
	2				1,34	0,87
S	1	3	0,0050	1,34	0,45	0,016
	2				2,77	0,92



3.7.2.2 - Simulação Hidrológica - Hidráulica do Evento Extremo Resultados Obtidos.

Os resultados obtidos mediante a simulação podem ser apreciados nos "output file" gerados pelo modelo HEC1, apresentados em anexo

Um resumo destes resultados pode ser apreciado na Tabela 21, onde são mostradas as vazões de pico de afluência e efluência do reservatório para os eventos extremos com recorrência de 500, 1 000 e 10 000 anos, utilizando a curva cota- área - volume apresentada na Figura 6 vertedouros com as seguintes características

1ª alternativa

Largura: 130 m

Cota da soleira 162,5 m

Tipo CREAGER

Coefficiente do vertedouro 2,21

Altura de Sangria 1m

Descarga - Decamilenar Amortecida

A predeterminação da largura do sangradouro em 100 e 130m, adveio da necessidade de reservar o maior volume de água possível, atendendo as condições topográficas e geológicas locais

Visando também proporcionar maior vazão, foi projetado referido sangradouro em perfil tipo CREAGER, com bacia de dissipação a jusante

2ª Alternativa

Largura: 100m

Cota de soleira: 162,5m

Tipo CREAGEM

Coefficiente de vertedouro. 2,21

Altura da sangria 1,92m

Descarga - Decamilenar Amortecida

3.8 - Interferência do Açude Souza Sobre a Barragem Pereira de Miranda

O Açude Souza, apresenta as características a seguir:

Área da bacia Hidrográfica	220Km ²
Volume afluente Médio	29,69Hm ³ /ano
Volume armazenado(cota 162,50)	30,88 Hm ³
Vazão regularizada	0,304m ³ /s(90% de garantia)



Em contrapartida com o Açude Pereira de Miranda, que apresenta as seguintes características

Área da bacia Hidrográfica	2 840Km ²
Volume afluente Médio	180Hm ³
Volume armazenado	395Hm ³
Vazão regularizada	4,63m ³ /s

Conforme podemos observar, o Açude Souza está dimensionado para conter apenas a vazão afluente média anual sendo portanto uma barragem sujeita a sangrias frequentes. Sua bacia hidrográfica, com 220Km² é equivalente a menos de 10% da bacia hidrográfica do Açude Pereira de Miranda e sua descarga regularizada é menor que 10% da descarga regularizada do Açude Pereira de Miranda, sendo portanto a sua interferência sobre este de pequena monta e dificilmente detectável por programas de simulação, com vista à escassez de informações existentes.

O Açude Souza, fornece uma vazão regularizada com 90% de garantia de 300l/s, valor este bem inferior a vazão de garantia do Açude Pereira de Miranda.

Com vista ao exposto concluímos que a influência do Açude Souza sobre a barragem Pereira de Miranda é desprezível ou pelo menos de uma ordem de grandeza inferior aos erros de estimativa das variáveis envolvidas.

Para melhor avaliar a influência do Açude Souza sobre a Açude Pereira de Miranda, foi feita uma avaliação, de conformidade com a metodologia a seguir

3.8.1 Metodologia

Para avaliar o impacto de um açude situado a montante de outro se procede à simulação da operação do reservatório impactada nas duas hipótese

- a) sem o reservatório impactante,
- b) com o reservatório impactante,



Para a simulação referida no item "b" se procede à subtração da área do Açude impactante na área da bacia hidrográfica contribuinte do Açude impactado, podendo-se considerar os volumes vertidos pelo reservatório impactante como contribuição eventual

No caso do açude Souza, o reservatório impactado é o denominado Pereira de Miranda, cujas principais características aparecem acima.

O resumo das simulações com e sem o reservatório de Souza podem ser apreciados nas tabelas

Para uma vazão regularizada com 90% de garantia observaram-se diminuições de 6% da vazão regularizado pelo reservatório Pereira de Miranda para a situação mais conservadora, isto é. ausência total de vertimentos no reservatório de Souza

Já para a vazão regularizada com volume de alerta (conforme metodologia do PERH) a diminuição de vazão foi de 3,6%. Ambos resultados são absolutamente desprezíveis pois se enquadram na ordem de grandezas dos erros admissíveis neste tipo estudo (Tabela 25 e 26)



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TABELA 25

VALORES DA SIMULAÇÃO DO AÇUDE PEREIRA DE MIRANDA COMO 2840 KM
DE BACIA HIDROGRÁFICA (INCLUI A BACIA HIDROGRÁFICA DO AÇUDE SOUSA)

VAZÃO REGULARIZADA (M3/S)	GARANTIA	VOLUME DE ALERTA	VAZÃO DE EMERGÊNCIA	PERCENTAGEM DE ATENDIMENTO NA EMERGÊNCIA	NRO FALHAS
4,15	90,04	18.500.000,00	2,08	13,03	92 *
4,15	90,04	27.928.450,00	2,08	13,03	92
3,90	90,48	37.358.900,00	1,95	28,63	88
3,67	90,91	46.785.350,00	1,83	42,74	84
3,67	90,15	56.213.800,00	1,83	52,19	91
3,58	90,15	65.642.250,00	1,79	60,08	91
3,36	90,80	75.070.700,00	1,68	74,24	85
3,32	90,48	84.499.150,00	1,66	80,92	88 **
3,12	90,91	93.927.600,00	1,56	93,47	84
3,12	90,15	103.356.100,00	1,56	94,80	91
2,93	90,58	112.784.500,00	1,47	98,79	87
2,89	90,26	122.213.000,00	1,45	99,73	90
2,72	90,69	131.641.400,00	1,36	100,00	86
2,56	90,91	141.069.900,00	1,28	100,00	84
2,56	90,04	150.498.300,00	1,28	100,00	92

* VAZÃO REGULARIZADA COM 90% DE GARANTIA PARA ÁREA DE 2840 KM -

** VAZÃO RUGULARIZADA COM 90 % DE GARANTIA E VOLUME DE ALERTA CONFORME CRITERIO DO PERH PARA ÁREA DE 2840 KM

000066



TABELA 26

VALORES DA SIMULAÇÃO DO AÇUDE PEREIRA DE MIRANDA COMO 2620 KM
DE BACIA HIDROGRÁFICA (EXCLUI A BACIA HIDROGRÁFICA DO AÇUDE SOUSA)

VAZÃO REGULARIZADA (M3/S)	GARANTIA	VOLUME DE ALERTA	VAZÃO DE EMERGÊNCIA	PERCENTAGEM DE ATENDIMENTO NA EMERGÊNCIA	NRO FALHAS
3,90	90,80	18.500.000,00	1,95	17,04	85 +
3,90	90,80	27.928.450,00	1,95	17,04	85
3,85	90,48	37.356.900,00	1,93	27,08	88
3,62	90,80	46.785.350,00	1,81	43,72	85
3,62	90,04	56.213.800,00	1,81	52,85	92
3,41	90,91	65.642.250,00	1,70	63,05	84
3,41	90,15	75.070.700,00	1,70	70,71	91
3,20	90,69	84.499.150,00	1,60	84,37	86 ++
3,16	90,26	93.927.600,00	1,58	89,65	90
2,97	90,80	103.356.100,00	1,48	96,52	85
2,93	90,26	112.784.500,00	1,47	98,47	90
2,76	90,91	122.213.000,00	1,38	100,00	84
2,76	90,04	131.641.400,00	1,38	100,00	92
2,59	90,48	141.069.900,00	1,29	100,00	88
2,43	90,69	150.498.300,00	1,22	100,00	86

* VAZÃO REGULARIZADA COM 90% DE GARANTIA PARA ÁREA DE 2620 KM -

** VAZÃO RUGULARIZADA COM 90 % DE GARANTIA E VOLUME DE ALERTA CONFORME CRITERIO DO PERH PARA ÁREA DE 2620 KM

000067



Visando atender as recomendações do Painel, foi feito o estudo de redução da largura do sangradouro para 100m.

Foi estudada a laminação da cheia milenar, atingindo-se a lâmina máxima na cota 163,65 m com 1,15 m (Tabela 27).

Para a cheia centenária atingiu-se a cota 164,42 com lâmina de 2,12, abaixo da cota do coroamento da barragem (Tabela 28).

Para este estudo foram utilizadas as cheias obtidas por simulação pelo HEC 1, por se julgarem mais precisas e apresentarem resultados mais conservadores com maior segurança para a obra.

Os valores de escoamento mensal foram comparados com os valores gerados no estudo do DNOCS para a bacia do Curu, reservatório de Pentecoste.

Estes estudos forneceram os seguintes resultados, vazões afluentes ao Reservatório de Pentecoste.

$$Q \text{ média} = 424,8 \times 10^6 \text{ m}^3/\text{ano}$$

$$A_p = \text{Área da bacia hidrográfica} = 2840 \text{ Km}^2$$

Por correlação de áreas, teremos:

Q_s Vazão afluente ao reservatório de Souza

A_s = área de bacia hidrográfica do Açude Souza = 220 Km

$$Q_s = \frac{A_s}{A_p} \times Q_p$$
$$Q_s = \frac{220}{2840} \times 424,8 \times 10^6 = 32,9 \times 10^6 \text{ m}^3 / \text{ano}$$

Valor muito semelhante ao encontrado nos estudos de simulação



000068



TABELA - 27

FLOOD ROUTING
SOUZA 10.000 ANOS VERT DE 100 M
B: QXT 800M B: HXV B: HXQ2
NA inicial 162,6 intervalo de tempo 1 horas
tempo inicial do hidrograma 04 dias

DIA	COTA	VAZÃO AFLUENTE	VAZÃO EFLUENTE
0.06	162.51	11.78	0.34
0.12	162.57	130.03	5.50
0.17	162.78	422.63	37.53
0.21	163.04	582.97	102.46
0.25	163.33	600.93	197.14
0.29	163.52	529.48	289.31
0.33	163.62	440.38	308.83
0.37	163.65	363.67	321.81
0.42	163.64	307.65	315.50
0.46	163.62	269.27	308.88
0.50	163.58	241.00	291.53
0.54	163.54	217.87	274.86
0.58	163.48	195.94	258.80
0.62	163.44	174.89	238.81
0.67	163.40	152.88	220.27
0.71	163.34	130.84	201.48
0.75	163.29	119.73	182.87
0.79	163.24	94.66	166.34
0.83	163.19	79.52	148.79
0.87	163.14	68.40	133.78
0.92	163.10	59.31	120.30
0.96	163.06	51.33	106.22
1.00	163.02	44.23	97.36
1.04	162.98	39.20	88.75
1.08	162.96	34.15	82.08
1.12	162.94	30.17	75.91
1.17	162.92	27.12	70.27
1.21	162.90	24.07	65.06
1.25	162.88	21.13	60.24
1.29	162.86	19.06	55.84
1.33	162.84	17.09	51.79
1.37	162.82	15.54	48.09
1.42	162.81	14.61	44.73
1.46	162.79	13.69	41.83
1.50	162.78	12.04	38.81
1.54	162.77	11.04	36.22
1.58	162.76	9.99	33.82
1.62	162.75	9.04	31.59
1.67	162.73	8.05	29.51
1.71	162.72	7.99	27.84
1.75	162.71	7.05	25.88
1.79	162.71	6.99	24.28
1.83	162.70	6.07	22.78



TABELA - 28

FLOOD ROUTING

FLOOD ROUTING SOUZA 500 ANOS (M)

VERT 130M CRISTA 162.50

B: QXT 800M B: HXV B: HXQ1

NA Inicial 162.5 Intervalo de tempo 1 horas

tempo Inicial do Hidrograma 04 dias

DIA	COTA	VAZÃO AFLUENTE	VAZÃO EFLUENTE
0.08	162.50	4.69	-1.97
0.12	162.52	30.49	-0.68
0.17	162.64	228.36	10.31
0.21	162.98	744.29	68.82
0.25	163.55	1,027.18	218.30
0.29	164.01	1,007.48	372.66
0.33	164.29	854.10	477.85
0.37	164.40	683.20	524.76
0.42	164.42	552.20	531.06
0.46	164.38	464.88	515.80
0.50	164.32	403.15	480.10
0.54	164.24	352.82	459.27
0.58	164.18	312.89	427.01
0.62	164.03	201.79	378.66
0.67	163.96	253.70	352.45
0.71	163.88	229.28	325.68
0.75	163.81	189.34	298.30
0.79	163.72	158.17	270.88
0.83	163.64	131.96	244.48
0.87	163.56	109.75	221.08
0.92	163.49	91.65	201.95
0.96	163.41	77.53	179.39
1.00	163.35	66.39	158.03
1.04	163.29	59.37	140.63
1.08	163.24	48.23	126.34
1.12	163.19	42.25	113.80
1.17	163.14	37.21	102.41
1.21	163.10	33.16	92.58
1.25	163.06	29.17	83.83
1.29	163.03	26.12	76.12
1.33	162.99	23.07	69.57
1.37	162.97	20.13	64.82
1.42	162.95	18.09	60.44
1.46	162.93	17.03	56.47
1.50	162.91	15.09	52.78
1.54	162.89	14.03	49.40
1.58	162.88	12.04	46.22
1.62	162.86	11.04	43.28
1.67	162.85	10.05	40.57
1.71	162.83	10.04	38.12
1.75	162.82	9.04	35.83
1.79	162.80	7.99	33.87
1.83	162.79	8.04	31.73
1.87	162.78	6.99	29.90
1.92	162.77	7.04	28.24
1.96	162.76	5.99	26.85
2.00	162.75	6.04	25.19
2.04	162.74	5.13	23.80

000070



3.9 - Estudo probabilístico de enchimento do Reservatório

O estudo probabilístico de enchimento de um reservatório é função de utilização de sua disponibilidade hídrica.

O açude Souza, projetado para uma capacidade de acumulação de $30.880.400\text{m}^3$ apresenta bons índices de enchimento, face à vazão anual afluenta média da ordem de $29.692.000\text{m}^3$.

A bacia apresentou em 77 anos 49 anos de fluviu abaixo da média, isto é uma probabilidade de 63,6% de ocorrerem sangrias.

Para uma retirada contínua de $0,3\text{m}^3/\text{ano}$ e perdas por evaporação teremos 29 anos de sangria num totas de 77 anos, com probabilidade de sangria de 37,6%, ou seja, a barragem deverá atingir sua cota máxima de sangria uma vez a cada três anos.

Se considerarmos, a barragem sem retirada (volume regularizado nulo) e, somente teremos a evaporação uma possibilidade de enchimento de ordem de 48,1%.



ANO	QAFL 10 ³ M ³ /ano	Sangria c/ retirada 0,3m ³ /s	Sem retirada no ANO	QAFL		
1912	62,5	s	s	1960	s	s
1913	92,4	s	s	1961	-	s
1914	0,1	-	-	1962	-	-
1915	7,7	-	-	1963	s	s
1916	17,9	-	-	1964	s	s
1917	62,6	s	s	1965	-	s
1918	0,0	-	-	1966	-	-
1919	0,0	-	-	1967	s	s
1920	37,1	s	s	1968	s	s
1921	2,7	-	-	1969	-	-
1922	40,1	s	s	1970	-	-
1923	10,3	-	-	1971	-	-
1924	131,7	s	s	1972	-	-
1925	0,4	-	-	1973	-	s
1926	91,2	s	s	1974	s	s
1927	0,0	-	-	1975	-	-
1928	27,6	s	s	1976	s	s
1929	52,9	s	s	1977	s	s
1930	28,0	s	s	1978	s	s
1931	0,0	-	-	1979	-	-
1932	0,0	-	-	1980	-	-
1933	12,5	-	-	1981	-	-
1934	19,1	-	-	1982	-	-
1935	56,5	s	s	1983	-	s
1936	21,1	s	s	1984	s	s
1937	13,6	-	-	1985	s	s
1938	28,0	s	s	1986	s	s
1939	3,1	-	-	1987	-	s
1940	38,8	s	s	1988	s	s
1941	18,8	s	s			
1942	0,0	-	-	Com retirada de 0,3m ³ /s 29 anos de sangria 77 anos de observação 37,6 de possibilidade de enchimento sem retirada 37 anos de sangria 77 anos de observação 98,1% de possibilidade de enchimento		
1943	0,0	-	-			
1944	0,0	-	-			
1945	15,0	-	-			
1946	0,0	-	-			
1947	25,1	s	s			
1948	29,5	s	s			
1949	11,6	-	-			
1950	42,6	s	s			
1951	0,0	-	-			
1952	0,0	-	-			
1953	0,0	-	-			
1954	0,0	-	-			
1955	1,6	-	-			
1956	4,6	-	-			
1957	96,2	s	s			
1958	1,1	-	-			
1959	0,0	-	-			

s = sim

3.10 - ESTUDO DOS CUSTOS DA VAZÃO REGULARIZADA

000672



A barragem Souza, apresentou um valor orçado para construção de R\$ 3.239.694,88, tendo com 90% de garantia e volume de alerta de $6,9\text{Hm}^3$ uma vazão anual regularizada de $0,303\text{m}^3/\text{s}$, equivalente a $9.555.400\text{m}^3/\text{ano}$.

O valor do m^3 regularizado, calculado foi de R\$ 0,34 enquanto o do m^3 estocado foi de R\$ 0,10.

3.11 - ESTUDO ECONÔMICO DE ALTERNATIVAS DE SANGRADOURO

O sangradouro da barragem Souza foi inicialmente estudado, para perfil CREAGER, com 100 e 130m de largura. Devido a fatores geológicos, com Rocha Sã a grande profundidade a largura de sangria fixada em 100m

Não foi efetuado estudo comparativo de custos dentre estas 2 (duas) alternativas, visto que por se tratarem de perfis idênticos, a 2ª alternativa terá um custo de aproximadamente 30% a menos a 1ª alternativa.

Posteriormente, após reunião do painel foram desenvolvidas 2 (duas) hipóteses finais a 1ª em perfil CREAGER, na cota 159, com fundação indo até a cota 156, e com lage de 20m de extensão ancorada na Rocha Sã e a 2ª, com fundação diretamente ancorada na Rocha Sã, aprofundada até a cota 153.

1ª Alternativa - somente o perfil verdadeiro e lage			TOTAL
Concreto ciclópico	900m ³	112,79	101.511,00
Concreto simples	200m ³	110,97	22.184,00
Concreto estrutural armado simples	600m ³	123,10	73.860,00
Ancoragem de estrutura	900m	267,90	241.110,00
Juntas Fungenband	301m	37,77	11.368,77
Armadura para lage	100Kg	1,39	139,00
Escavação material 2ª categoria	150m ³	2,18	1.327,00
TOTAL			450.499,77

2ª Alternativa

Concreto ciclópico 12% pedra de mão	1.050,00m ³	112,79	118.429,50
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Concreto simples	200,00m ³	110,97	22.184,00
Juntas Fungenband	50m	37,77	2.266,20
Escavação mat 2ª categoria	250m ³	2,18	545,00
Escavação mat 3ª categoria	50m ³	11,65	582,50
TOTAL			144.607,20

Tornando-se a 2ª alternativa bastante mais econômica.

Estas alternativas, por medida de segurança serão apresentadas aos consultores, estando no orçamento da barragem o orçamento da 1ª alternativa.

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Simulação Modelo TR 500

000075

ID BACIA HIDROGRAFICA DO RESERVATORIO DE SOUSA
 ID MODELO DE ONDA CINEMATICA DA BACIA TR-500
 IT 30 15APR96 1500 300
 IO 30
 IM
 *

*DIAGRAM

KK	BACIA ABAMICO COMPONENTE E																			
KM	ESC SUPERFICIAL SUBBACIA E																			
KO	3																			
BA	15	83																		
PH	0	5	220	0	19	4	40	0	77	3	98	0	110	0	148	0	164	0	194	0
LS																				
UK	810		05		40		40	6												
UK	1170		05		40		59	4												
RR	8000		006		045						TRAP		0							40
KK	BACIA ABAMICO COMPONENTE H																			
KM	ESC SUPERFICIAL SUBBACIA H																			
KO	3																			
BA	22																			
UK	1130		016		40		46	1												
UK	1320		016		40		53	9												
RR	9000		008		045						TRAP		0							40
KK	BACIA ABAMICO COMPONENTE I																			
KM	ESC SUPERFICIAL SUBBACIA I																			
KO	3																			
BA	19	42																		
UK	1700		016		40		35	0												
UK	3150		016		40		65	0												
RR	4000		0040		045						TRAP		0							40
KK	BACIA ABAMICO COMPONENTE J																			
KM	ESC SUPERFICIAL SUBBACIA J																			
KO	3																			
BA	13	87																		
UK	1210		013		40		52	3												
UK	1100		013		40		47	7												
RR	6000		.0030		045						TRAP		0							40
KK	BACIA EM ABAMICO COMPLETA																			
HC	4																			
KK	BACIA F-G COMPONENTE F																			
KM	ESC SUPERFICIAL SUBBACIA F																			
KO	3																			
BA	3	04																		
UK	1250		010		40		61	8												
UK	780		010		40		38	2												
RR	1500		0088		045						TRAP		0							40
KK	BACIA F - G COMPONENTE G																			
KM	ESC SUPERFICIAL SUBBACIA G																			
KO	3																			
BA	4	92																		
UK	980		010		40		59	9												
UK	660		018		40		40	1												
RR	3000		.0088		045						TRAP		0							40
KK	BACIA EM ABAMICO COMPLETA + BACIA F - G																			
HC	2																			
KK	BACIA P																			
KM	ESC SUPERFICIAL BACIA P																			
KO	3																			
BA	8	33																		
UK	820		013		40		49	4												
UK	840		013		40		50	6												
RR	5000		0030		045						TRAP		0							40
KK	BACIA K-L COMPONENTE K																			
KM	ESC SUPERFICIAL SUBBACIA K																			
KO	3																			
BA	19	41																		
UK	1340		003		40		48	8												
UK	1400		003		40		51	2												
RR	7100		0080		045						TRAP		0							40
KK	BACIA K - L COMPONENTE L																			
KM	ESC SUPERFICIAL SUBBACIA L																			
KO	3																			
BA	15	48																		
UK	440		020		40		24	3												
UK	1380		020		40		75	7												
RR	8500		.0030		045						TRAP		0							40
KK	BACIA M																			
KM	ESC SUPERFICIAL BACIA M																			
KO	3																			
BA	12	89																		
UK	750		010		40		31	9												
UK	1590		010		40		68	1												
RR	5500		0080		045						TRAP		0							40
KK	BACIA M + BACIA K - L																			
HC	2																			
KK	BACIA Q																			
KM	ESC SUPERFICIAL SUBBACIA Q																			
KO	3																			
BA	5	28																		
UK	750		013		40		42	4												
UK	1010		013		40		57	6												
RR	3000		0030		045						TRAP		0							40
KK	BACIA D-C COMPONENTE D																			
KM	ESC SUPERFICIAL SUBBACIA D																			
KO	3																			
BA	14	94																		
UK	1860		080		40		74	8												
UK	630		080		40		25	2												
RR	6000		0080		045						TRAP		0							40
KK	BACIA D - C COMPONENTE C																			
KM	ESC SUPERFICIAL SUBBACIA C																			
KO	3																			
BA	7	52																		
UK	870		030		40		24	3												
UK	800		030		40		75	7												
RR	4500		0060		045						TRAP		0							40
KK	BACIA Q + BACIA P + BACIA D-C																			
HC	3																			
KK	BACIA R																			
KM	ESC SUPERFICIAL SUBBACIA R																			
KO	3																			
BA	3	67																		
UK	1160		013		40		63	4												

000076

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UK 670 013 40 36 6
RK 2000 0030 045 TRAP 0 40 YES
KK BACIA B-A COMPONENTE B
KM ESC SUPERFICIAL SUBBACIA B
KO 3
BA 6 44
UR 490 050 40 30 5
UK 1120 050 40 69 5
RK 4000 0100 045 TRAP 0 40
KK BACIA B - A COMPONENTE A
KM ESC SUPERFICIAL SUBBACIA A
KO 3
BA 8 85
UR 910 030 40 51 5
UK 860 030 40 48 5
RK 5000 0080 045 TRAP 0 40 YES
KK BACIA N
KM ESC SUPERFICIAL SUBBACIA N
KO 3
BA 19 60
UR 780 013 40 37 9
UK 1280 013 40 62 1
RK 9500 0030 045 TRAP 0 40
KK BACIA R + BACIA N + BACIA A-B
HC 3
KK BACIA S
KM ESC SUPERFICIAL SUBBACIA S
KO 1
BA 4 11
UR 450 010 40 32 6
UK 920 010 40 67 3
RK 3000 0030 045 TRAP 0 40 YES
KK BARRAGEM DE SOUSA
KO 1
RS 1 STOR 30000 -1
SV 18043 22516 28092 3 33668 4 39244 4
SE 97 98 99 100 101 102
SS 100 5 130 1 4 1 5
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* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* SEPTEMBER 1990 *
* VERSION 4 0 *
* RUN DATE 05/05/1996 TIME 18 09 10 *
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* U S ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
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XXXXXXXX XXXX XXXXX X
X X X X X X
X X X X X X
X X XXXXXXXX XXXXX XXX

```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC10S, HEC1DB, AND HEC1KW

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION. NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS-WRITE STAGE FREQUENCY, DSS READ TIME SERIES AT DESIRED CALCULATION INTERVAL, LOSS RATE:GREEN AND AMPT INFILTRATION, KINEMATIC WAVE, NEW FINITE DIFFERENCE ALGORITHM.

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	BACIA HIDROGRAFICA DO RESERVATORIO DE SOUSA									
2	ID	MODELO DE ONDA CINEMATICA DA BACIA TR-500									
3	IT	30	15APR96	1500	300						
4	IO	30									
5	IM										
		*DIAGRAM									
6	KK	BACIA ABANICO COMPONENTE E									
7	KM	ESC SUPERFICIAL SUBBACIA E									
8	KO	3									
9	BA	15	83								
10	PH	0 5	220 0	19 4	40 0	77 3	98 0	110 0	148 0	164 0	194 0
11	LS		74			74					
12	UK	810	05	40	40 6						
13	UK	1170	05	40	59 4						
14	RK	8000	006	045		TRAP	0	40			
15	KK	BACIA ABANICO COMPONENTE H									
16	KM	ESC SUPERFICIAL SUBBACIA H									
17	KO	3									
18	BA	22	1								
19	UK	1130	016	40	46 1						
20	UK	1328	016	40	53 9						
21	RK	9000	008	045		TRAP	0	40			
22	KK	BACIA ABANICO COMPONENTE I									
23	KM	ESC SUPERFICIAL SUBBACIA I									
24	KO	3									
25	BA	19	42								
26	UK	1700	016	40	35 0						
27	UK	3150	016	40	65 0						
28	RK	4000	0040	045		TRAP	0	40			
29	KK	BACIA ABANICO COMPONENTE J									
30	KM	ESC SUPERFICIAL SUBBACIA J									
31	KO	3									
32	BA	13	87								
33	UR	1210	013	40	52 3						

000077

34	UK	1100	013	40	47.7						
35	RR	6000	0030	045		TRAP	0	40			
36	KK	BACIA EM ABANICO COMPLETA									
37	HC	4									
38	KK	BACIA F-G COMPONENTE F									
39	KM	ESC SUPERFICIAL SUBBACIA F									
40	KO	3									
41	BA	3.04									
42	UK	1250	010	40	61.8						
43	UK	780	010	40	38.2						
44	RR	1500	0080	045		TRAP	0	40			
						HEC-1 INPUT					

1

PAGE 2

LINE	ID	1	2	3	4	5	6	7	8	9	10
45	KK	BACIA F - G COMPONENTE G									
46	KM	ESC SUPERFICIAL SUBBACIA G									
47	KO	3									
48	BA	4.92									
49	UK	980	010	40	59.9						
50	UK	660	010	40	48.1						
51	RR	3000	0080	045		TRAP	0	40	YES		
52	KK	BACIA EM ABANICO COMPLETA + BACIA F - G									
53	HC	2									
54	KK	BACIA P									
55	KM	ESC SUPERFICIAL BACIA P									
56	KO	3									
57	BA	8.33									
58	UK	820	013	40	49.4						
59	UK	840	013	40	50.6						
60	RR	5000	0030	045		TRAP	0	40	YES		
61	KK	BACIA K-L COMPONENTE K									
62	KM	ESC SUPERFICIAL SUBBACIA K									
63	KO	3									
64	BA	19.41									
65	UK	1340	003	40	48.8						
66	UK	1480	003	40	51.2						
67	RR	7100	0080	045		TRAP	0	40			
68	KK	BACIA K - L COMPONENTE L									
69	KM	ESC SUPERFICIAL SUBBACIA L									
70	KO	3									
71	BA	15.48									
72	UK	440	028	40	24.3						
73	UK	1380	020	40	75.7						
74	RR	8500	0030	045		TRAP	0	40	YES		
75	KK	BACIA M									
76	KM	ESC SUPERFICIAL BACIA M									
77	KO	3									
78	BA	12.89									
79	UK	750	010	40	31.9						
80	UK	1590	010	40	68.1						
81	RR	5500	0080	045		TRAP	0	40			
82	KK	BACIA M + BACIA K - L									
83	HC	2									
84	KK	BACIA Q									
85	KM	ESC SUPERFICIAL SUBBACIA Q									
86	KO	3									
87	BA	5.28									
88	UK	750	013	40	42.4						
89	UK	1010	013	40	57.6						
90	RR	3000	0030	045		TRAP	0	40	YES		
						HEC-1 INPUT					

1

PAGE 3

LINE	ID	1	2	3	4	5	6	7	8	9	10
91	KK	BACIA D-C COMPONENTE D									
92	KM	ESC SUPERFICIAL SUBBACIA D									
93	KO	3									
94	BA	14.94									
95	UK	1860	080	40	74.8						
96	UK	630	080	40	25.2						
97	RR	6000	0080	045		TRAP	0	40			
98	KK	BACIA D - C COMPONENTE C									
99	KM	ESC SUPERFICIAL SUBBACIA C									
100	KO	3									
101	BA	7.52									
102	UK	870	030	40	24.3						
103	UK	800	030	40	75.7						
104	RR	4500	0060	045		TRAP	0	40	YES		
105	KK	BACIA Q + BACIA P + BACIA D-C									
106	HC	3									
107	KK	BACIA R									
108	KM	ESC SUPERFICIAL SUBBACIA R									
109	KO	3									
110	BA	3.67									
111	UK	1160	013	40	63.4						
112	UK	670	013	40	36.6						
113	RR	2000	0030	045		TRAP	0	40	YES		
114	KK	BACIA B-A COMPONENTE B									
115	KM	ESC SUPERFICIAL SUBBACIA B									
116	KO	3									
117	BA	6.44									
118	UK	490	050	40	30.5						
119	UK	1120	050	40	69.5						

000078

120	RK	4000	0100	045		TRAP	0	40		
121	KK	BACIA B - A COMPONENTE A								
122	KM	ESC SUPERFICIAL SUBBACIA A								
123	KO	3								
124	BA	8 85								
125	UK	910	030	40	51 5					
126	UK	060	030	40	40.5					
127	RK	5000	0080	045		TRAP	0	40	YES	
128	KK	BACIA N								
129	KM	ESC SUPERFICIAL SUBBACIA N								
130	KO	3								
131	BA	19 60								
132	UK	780	013	40	37 9					
133	UK	1280	013	40	62 1					
134	RK	9500	0030	045		TRAP	0	40		
						HEC-1 INPUT				

1
PAGE 4

LINE	ID	1	2	3	4	5	6	7	8	9	10
135	KK	BACIA R + BACIA N + BACIA A-B									
136	HC	3									
137	KK	BACIA S									
138	KM	ESC SUPERFICIAL SUBBACIA S									
139	KO	1									
140	BA	4 11									
141	UK	450	010	40	32 6						
142	UK	920	010	40	67 3						
143	RK	3000	0030	045		TRAP	0	40	YES		
144	KK	BARRAGEM DE SOUSA									
145	KO	1									
146	RS	1	STOR	30000	-1						
147	SV	18043	22516	20092 3	33668.4	39244.4					
148	SE	97	98	99	100	101	102				
149	SS	100 5	130	1 4	1 5						
150	ZZ										

1

SCHMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE (V) ROUTING (----) DIVERSION OR PUMP FLOW
 NO () CONNECTOR (<----) RETURN OF DIVERTED OR PUMPED FLOW

6

15

22

29

36 BACIA

38

V
V

45

52

BACIA

V

54

V

61

V
V

68

75

82

BACIA

V
V

84

91

V
V

98

105

BACIA

V

107

V

114

V
V

121

128

135

BACIA

V

V

000079

137

V
V
144 BARR

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

.....
FLOOD HYDROGRAPH PACKAGE (HEC-1)
SEPTEMBER 1990
VERSION 4 0
RUN DATE 05/05/1996 TIME 18 09 10
.....

.....
U S ARMY CORPS OF ENGINEERS
HYDROLOGIC ENGINEERING CENTER
609 SECOND STREET
DAVIS, CALIFORNIA 95616
(916) 756-1104
.....

BACIA HIDROGRAFICA DO RESERVATORIO DE SOUSA
MODELO DE ONDA CINEMATICA DA BACIA TR-500

4 IO OUTPUT CONTROL VARIABLES
IPRNT 30 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0 HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA
MMIN 30 MINUTES IN COMPUTATION INTERVAL
IDATE 15APR96 STARTING DATE
ITIME 1500 STARTING TIME
NQ 300 NUMBER OF HYDROGRAPH ORDINATES
NDATE 21APR96 ENDING DATE
NTIME 2030 ENDING TIME
ICENT 19 CENTURY MARK

COMPUTATION INTERVAL .50 HOURS
TOTAL TIME BASE 149 50 HOURS

METRIC UNITS
DRAINAGE AREA SQUARE KILOMETERS
PRECIPITATION DEPTH MILLIMETERS
LENGTH, ELEVATION METERS
FLOW CUBIC METERS PER SECOND
STORAGE VOLUME CUBIC METERS
SURFACE AREA SQUARE METERS
TEMPERATURE DEGREES CELSIUS

8 KO OUTPUT CONTROL VARIABLES
IPRNT 3 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

9 BA SUBBASIN CHARACTERISTICS
IAREA 15 83 SUBBASIN AREA

PRECIPITATION DATA

10 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
HYDRO-35 TP-40 TP-49
5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
19 40 40 00 77 30 98.00 110.00 148 00 164 00 194 00 00 .00 00 .00
STORM AREA = 220 00

11 LS SCS LOSS RATF
SIRTL 17 85 INITIAL ABSTRACTION
CRVNR 74 00 CURVE NUMBER
RTIME 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
SIRTL 17 85 INITIAL ABSTRACTION
CRVNR 74 00 CURVE NUMBER
RTIME 00 PERCENT IMPERVIOUS AREA

12 UK KINEMATIC WAVE
OVERLAND-FLOW ELEMENT NO. 1
L 810 OVERLAND FLOW LENGTH
S 0580 SLOPE
N 400 ROUGHNESS COEFFICIENT
PA 40 6 PERCENT OF SUBBASIN
DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

13 UK OVERLAND-FLOW ELEMENT NO. 2
L 1170 OVERLAND FLOW LENGTH
S 0500 SLOPE
N 400 ROUGHNESS COEFFICIENT
PA 59 4 PERCENT OF SUBBASIN
DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

14 RK KINEMATIC WAVE
MAIN CHANNEL
L 8000 CHANNEL LENGTH
S 0060 SLOPE
N 045 CHANNEL ROUGHNESS COEFFICIENT
CA 15 83 CONTRIBUTING AREA
SHAPE TRAP CHANNEL SHAPE
WD 00 BOTTOM WIDTH OR DIAMETER
Z 40 00 SIDE SLOPE
NDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
RUPSTQ NO ROUTE UPSTREAM HYDROGRAPH

000080

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT	DX	PEAK	TIME TO PEAK	VOLUME	MAXIMUM CELERITY
			(MIN)	(M)	(CMS)	(MIN)	(MM)	(MPS)

PLANE1	56	1 67	18 55	162 05	38.57	805 08	101 54	15
PLANE2	56	1 67	24 65	234 08	47 66	831 75	101 41	16
MAIN	32	1 33	25 16	2667 54	76 39	864 79	101 35	1 78

CONTINUITY SUMMARY (1000 CU-M)-INFLOW= 0000E+00 EXCESS= 1612E+04 OUTFLOW= 1599E+04 BASIN STORAGE= 1187E+01 PERCENT ERROR= 7

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	32	1 33	30 00	75 84	870 00	101.41
------	----	------	-------	-------	--------	--------

*** *** *** *** ***

HYDROGRAPH AT STATION

TOTAL RAINFALL - 177 18, TOTAL LOSS - 75 05, TOTAL EXCESS - 102 13

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
(CU M/S)	(HR)		6-HR	24-HR	72-HR	149 50-HR
+	76	14 50	48	18	6	3
		(CU M/S)				
		(MM)	66 048	98.598	101 150	101 359
		(1000 CU M)	1046	1561	1601	1605

CUMULATIVE AREA - 15 83 SQ KM

17 KO

OUTPUT CONTROL VARIABLES

IPRNT	3	PRINT CONTROL
IPLOT	0	PLOT CONTROL
QSCAL	0	HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

18 BA

SUBBASIN CHARACTERISTICS

IAREA	22 10	SUBBASIN AREA
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PRECIPITATION DATA

10 PH

DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
19 40	40 00	77 30	98 00	110.00	148.00	164 00	194 00	00	00	00	00

STORM AREA - 220.00

11 LS

SCS LOSS RATE

SRTL	17 85	INITIAL ABSTRACTION
CRVNR	74 00	CURVE NUMBER
RTIMP	00	PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT

SRTL	17.85	INITIAL ABSTRACTION
CRVNR	74 00	CURVE NUMBER
RTIMP	00	PERCENT IMPERVIOUS AREA

KINEMATIC WAVE

19 UK

OVERLAND-FLOW ELEMENT NO. 1

L	1130	OVERLAND FLOW LENGTH
S	0160	SLOPE
N	400	ROUGHNESS COEFFICIENT
PA	46 1	PERCENT OF SUBBASIN
DXMIN	5	MINIMUM NUMBER OF DX INTERVALS

20 UK

OVERLAND-FLOW ELEMENT NO. 2

L	1320	OVERLAND FLOW LENGTH
S	0160	SLOPE
N	400	ROUGHNESS COEFFICIENT
PA	53 9	PERCENT OF SUBBASIN
DXMIN	5	MINIMUM NUMBER OF DX INTERVALS

KINEMATIC WAVE

21 RR

MAIN CHANNEL

L	9000	CHANNEL LENGTH
S	.0080	SLOPE
N	045	CHANNEL ROUGHNESS COEFFICIENT
CA	22 10	CONTRIBUTING AREA
SHAPE	TRAP	CHANNEL SHAPE
WD	00	BOTTOM WIDTH OR DIAMETER
Z	40 00	SIDE SLOPE
MDXMIN	2	MINIMUM NUMBER OF DX INTERVALS
WUPSTQ	NO	ROUTE UPSTREAM HYDROGRAPH

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT	DX	PEAK	TIME TO PEAK	VOLUME	MAXIMUM CELERITY
			(MIN)	(M)	(CMS)	(MIN)	(MM)	(MPS)
PLANE1	32	1 67	27 47	161 48	40 14	918.48	101 30	10
PLANE2	32	1 67	30.00	188.63	42 33	920.98	101.21	10
MAIN	37	1 33	25.15	3000.98	76 77	953 46	101 19	1 99

CONTINUITY SUMMARY (1000 CU-M)-INFLOW= 0000E+00 EXCESS= 2251E+04 OUTFLOW= 2229E+04 BASIN STORAGE= 4756E+01 PERCENT ERROR= 8

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	37	1 33	30 00	76 30	960 00	101 17
------	----	------	-------	-------	--------	--------

*** *** *** *** ***

HYDROGRAPH AT STATION

TOTAL RAINFALL - 177 18, TOTAL LOSS - 75 05, TOTAL EXCESS - 102 13

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
-----------	------	--	----------------------	--	--	--

000081

+ (CU M/S) (HR) (CU M/S)
 + 76 16 00 (MM) 50. 24 9 4
 (1000 CU M) 56 474 95.452 100.592 101 113
 1248 2109 2223 2235

CUMULATIVE AREA - 22 10 SQ KM

24 KO OUTPUT CONTROL VARIABLES
 IPRNT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

25 BA SUBBASIN CHARACTERISTICS
 TAREA 19 42 SUBBASIN AREA

PRECIPITATION DATA

10 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
 HYDRO-35 TP-49
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
 19.40 40 00 77 30 98 00 110 00 148 00 164 00 194 00 00 00 00 00

STORM AREA - 220 00

11 LS SCS LOSS RATE
 STRTL 17.05 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STRTL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

26 UK KINEMATIC WAVE OVERLAND-FLOW ELEMENT NO. 1
 L 1700 OVERLAND FLOW LENGTH
 S 0160 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 35 0 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

27 UK KINEMATIC WAVE OVERLAND-FLOW ELEMENT NO. 2
 L 3150 OVERLAND FLOW LENGTH
 S 0160 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 65 0 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

28 RK KINEMATIC WAVE MAIN CHANNEL
 L 4000 CHANNEL LENGTH
 S .0040 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 19.42 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD .00 BOTTOM WIDTH OR DIAMETER
 Z 40.00 SIDE SLOPE
 DXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ NO ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	32	1 67	30.00	100.95	20 13	984 18	101 23	10
PLANE2	32	1 67	30 00	242.39	24 41	1212 43	100 90	11
MAIN	26	1 33	17 12	1333.77	39 95	1011 92	101 00	1 31

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 0000E+00 EXCESS- 1978E+04 OUTFLOW- 1955E+04 BASIN STORAGE- 1173E+02 PERCENT ERROR- 6

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 26 1 33 30 00 39 95 1020 00 101 00

HYDROGRAPH AT STATION

TOTAL RAINFALL - 177 10, TOTAL LOSS - 75 05, TOTAL EXCESS - 102 13

PEAK FLOW TIME MAXIMUM AVERAGE FLOW
 + (CU M/S) (HR) (CU M/S) 6-HR 24-HR 72-HR 149 50-HR
 + 40 17 00 (MM) 37 20 7 4
 (1000 CU M) 40 809 89 469 99 664 100.952
 793 1737 1935 1960

CUMULATIVE AREA - 19 42 SQ KM

31 KO OUTPUT CONTROL VARIABLES
 IPRNT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

32 BA SUBBASIN CHARACTERISTICS
 TAREA 13.07 SUBBASIN AREA

000082

PRECIPITATION DATA

10 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
 HYDRO-35 TP-40 TP-49
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
 19 40 40 00 77 30 98 00 110 00 148 00 164 00 194 00 .00 00 .00 00
 STORM AREA - 220 00

11 LS SCS LOSS RATE
 STRFL 17 85 INITIAL ABSTRACTION
 CRVWBR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA
 LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STRFL 17 85 INITIAL ABSTRACTION
 CRVWBR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

33 UR KINEMATIC WAVE
 OVERLAND-FLOW ELEMENT NO. 1
 L 1210 OVERLAND FLOW LENGTH
 S 0130 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 52 3 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS
 34 UR OVERLAND-FLOW ELEMENT NO. 2
 L 1100 OVERLAND FLOW LENGTH
 S 0130 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 47 7 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

35 RK KINEMATIC WAVE
 MAIN CHANNEL
 L 6000 CHANNEL LENGTH
 S 0030 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 13 87 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 DXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ NO ROUTE UPSTREAM HYDROGRAPH

COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	29	1 67	20 07	151 30	25 57	926 36	101 29	09
PLANE2	29	1 67	29 45	157 19	24 92	984 34	101 32	.09
MAIN	22	1 33	27 45	2000 66	46 95	956 84	101 23	1 22

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 0080E+00 EXCESS- 1413E+04 OUTFLOW- 1399E+04 BASIN STORAGE- 3254E+01 PERCENT ERROR- 7

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 22 1 33 30 00 46 86 960 00 101 28

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HYDROGRAPH AT STATION

TOTAL RAINFALL - 177 18, TOTAL LOSS - 75 05, TOTAL EXCESS - 102 13

PEAK FLOW + (CU M/S)	TIME (HR)	6-HR (CU M/S)	24-HR (MM)	72-HR (1000 CU M)	149 50-HR (1000 CU M)
47	16 00	36	55 807	100 679	101 227
		774	1322	1396	1404

CUMULATIVE AREA - 13 87 SQ KM

40 KO OUIPUT CONIROL VARIABLES
 IPRT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

41 BA SUBBASIN CHARACTERISTICS
 TAREA 3 04 SUBBASIN AREA

PRECIPITATION DATA

10 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
 HYDRO-35 TP-40 TP-49
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
 19 40 40 00 77 30 98 00 110 00 148 00 164 00 194 00 00 .00 00 00
 STORM AREA - 220 00

11 LS SCS LOSS RATE
 STRFL 17.85 INITIAL ABSTRACTION
 CRVWBR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA
 LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT

000083

STRIL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

KINEMATIC WAVE
 42 UK OVERLAND-FLOW ELEMENT NO 1
 L 1250 OVERLAND FLOW LENGTH
 S 0100 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 61 8 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS
 43 UK OVERLAND-FLOW ELEMENT NO 2
 L 780 OVERLAND FLOW LENGTH
 S 0100 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 38 2 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

KINEMATIC WAVE
 44 RK MAIN CHANNEL
 L 1500 CHANNEL LENGTH
 S 0080 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 3 04 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 NDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ NO ROUTE UPSTREAM HYDROGRAPH

COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	N	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	25	1 67	28.92	138.93	5 88	965 42	101 32	88
PLANE2	25	1 67	24 51	111 47	5.01	898 86	101.43	88
MAIN	37	1 33	7 10	508 16	9 98	933 22	101 39	1 20

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 0008E+08 EXCESS- 3096E+03 OUTFLOW- 3072E+03 BASIN STORAGE- 6957E+00 PERCENT ERROR- 6

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 37 1 33 30 00 9 98 930 00 101 30

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HYDROGRAPH AT STATION

TOTAL RAINFALL - 177 18 TOTAL LOSS - 75 05, TOTAL EXCESS - 102 13

PEAK FLOW (CU M/S)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	149 50-HR
10	15 50	8	3	1	1
		(MM) 56 255	95 555	100 700	101 248
		(1000 CU M) 171	290	306	308

CUMULATIVE AREA - 3 04 SQ KM

47 RO OUTPUT CONTROL VARIABLES
 IPRMT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

48 BA SUBBASIN CHARACTERISTICS
 IAREA 4 92 SUBBASIN AREA

PRECIPITATION DATA

10 PH DEPTHS FOR 8-PERCENT HYPOTHETICAL STORM
 HYDRO-35 TP-40 TP-49
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
 19 40 40 00 77 30 98 00 110 00 148 00 164.00 194 00 00 00 00 00 00

STORM AREA - 220 00

11 LS SCS LOSS RATE
 STRIL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STRIL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

KINEMATIC WAVE
 49 UK OVERLAND-FLOW ELEMENT NO 1
 L 980 OVERLAND FLOW LENGTH
 S 0100 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 59.9 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS
 50 UK OVERLAND-FLOW ELEMENT NO 2
 L 660 OVERLAND FLOW LENGTH
 S 0100 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 48 1 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

008884

51 RK

KINEMATIC WAVE
 MAIN CHANNEL
 L 3000 CHANNEL LENGTH
 S 0080 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 4 92 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 NDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ YES ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	25	1 67	26 19	122 54	11 02	909 03	101 38	08
PLANE2	25	1 67	24 68	110 04	9 11	864 22	101 36	07
MAIN	37	1 33	10 99	1000 33	28 38	909 94	101 34	1 52

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 3067E+03 EXCESS- 5011E+03 OUTFLOW- 8040E+03 BASIN STORAGE- 8424E+00 PERCENT ERROR- 4

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 37 1 33 30 00 28 16 930 00 101 25

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HYDROGRAPH AT STATION

TOTAL RAINFALL - 177 18 TOTAL LOSS - 75 05, TOTAL EXCESS - 102 13

PEAK FLOW (CU M/S)	TIME (HR)	6-HR (CU M/S)	MAXIMUM 24-HR (MM)	AVERAGE FLOW 72-HR (1000 CU M)	149 50-HR (MM)
28	15 50	21	9	3	1
		58 333	96 175	100 735	101 201
		464	766	802	806

CUMULATIVE AREA - 7 96 SQ KM

56 KO

OUTPUT CONTROL VARIABLES
 IPRMT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

57 BA

SUBBASIN CHARACTERISTICS
 TAREA 8 33 SUBBASIN AREA

PRECIPITATION DATA

10 PH

DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35			TP-40				TP-49				
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
19 40	40 00	77 30	98 00	110 00	140 00	164 00	194 00	00	00	00	.00

STORM AREA - 220 00

11 LS

SCS LOSS RATE
 STRL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STRL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

58 UK

KINEMATIC WAVE
 OVERLAND-FLOW ELEMENT NO 1
 L 820 OVERLAND FLOW LENGTH
 S 0130 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 49 4 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

59 UK

OVERLAND-FLOW ELEMENT NO 2
 L 840 OVERLAND FLOW LENGTH
 S 0130 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 50 6 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

60 RK

KINEMATIC WAVE
 MAIN CHANNEL
 L 5000 CHANNEL LENGTH
 S 0030 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 8 33 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 NDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ YES ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

000085

ELEMENT	ALPHA	N	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	29	1 67	26 48	136 71	18 33	879.39	101 36	09
PLANE2	29	1 67	27 07	140 85	18 59	883 91	101 33	09
MAIN	22	1 33	15 58	1667 21	274 43	957 18	100 89	1.79

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 7981E+04 EXCESS- 8484E+03 OUTFLOW- 8799E+04 BASIN STORAGE- 2433E+01 PERCENT ERROR- 3

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	22	1 33	30 00		273 77	960 00	100 88	
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HYDROGRAPH AT STATION

TOTAL RAINFALL - 177 18, TOTAL LOSS - 75 05, TOTAL EXCESS - 102 13

PEAK FLOW (CU M/S)	TIME (HR)	6-HR (CU M/S)	24-HR (1000 CU M)	72-HR (1000 CU M)	149.50-HR (1000 CU M)
274	16 00	217	96	34	16
		53 551	94 344	100 201	100 831
		4686	8256	8769	8824

CUMULATIVE AREA - 87 51 SQ KM

63 KO OUTPUT CONTROL VARIABLES
 IPRMT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUMOFF DATA

64 BA SUBBASIN CHARACTERISTICS
 TAREA 19 41 SUBBASIN AREA

PRECIPITATION DATA

HYDRO-35		DEPIHS FOR 0-PERCENT HYPOTHETICAL STORM						TP-49			
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
19 40	40 00	77 30	98 00	110.00	148.00	164 00	194 00	00	00	00	00

STORM AREA - 220 00

11 LS SCS LOSS RATE
 SIRTLL 17.85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 SIRTLL 17.85 INITIAL ABSTRACTION
 CRVNR 74.00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

65 UK KINEMATIC WAVE
 OVERLAND-FLOW ELEMENT NO. 1
 L 1340 OVERLAND FLOW LENGTH
 S 0030 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 48.8 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

66 UK OVERLAND-FLOW ELEMENT NO. 2
 L 1400 OVERLAND FLOW LENGTH
 S 0030 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 51.2 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

67 RK KINEMATIC WAVE
 MAIN CHANNEL
 L 7100 CHANNEL LENGTH
 S 0080 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 19 41 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 NDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTO NO ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	N	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	14	1 67	27 85	78 85	18.61	1204.95	101.11	05
PLANE2	14	1 67	27 31	77.80	18.98	1226.01	101 08	.05
MAIN	37	1 33	23 87	2367 44	36.64	1231 43	101 03	1.66

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 0000E+00 EXCESS- .1977E+04 OUTFLOW- 1955E+04 BASIN STORAGE- 1416E+02 PERCENT ERROR- 4

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	37	1 33	30 00		36.62	1230 00	101 03	
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000086

HYDROGRAPH AT STATION

TOTAL RAINFALL - 177 18 TOTAL LOSS - 75 05. TOTAL EXCESS - 102 13

PEAK FLOW (CU M/S)	TIME (HR)	6-HR (CU M/S)	24-HR (MM)	72-HR (MM)	149 50-HR (MM)
37	20 50	34	37 681	20	7
			731	87 317	99 423
				1695	1930
					1960.

CUMULATIVE AREA - 19 41 SQ KM

70 KO OUTPUT CONTROL VARIABLES
 IPRMT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

71 BA SUBBASIN CHARACTERISTICS
 TAREA 15 48 SUBBASIN AREA

PRECIPITATION DATA

HYDRO-35		DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM								TP-49	
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
19 40	40 00	77 30	98 00	110 00	140 00	164.00	194.00	.00	.00	.00	.00

STORM AREA - 220 00

11 LS SCS LOSS RATE
 STRL 17 85 INITIAL ABSTRACTION
 CRVNR 74.00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STRL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

72 UK KINEMATIC WAVE OVERLAND-FLOW ELEMENT NO 1
 L 440. OVERLAND FLOW LENGTH
 S 0200 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 24 3 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

73 UK KINEMATIC WAVE OVERLAND-FLOW ELEMENT NO 2
 L 1380 OVERLAND FLOW LENGTH
 S 0200 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 75 7 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

74 RK KINEMATIC WAVE MAIN CHANNEL
 L 8500 CHANNEL LENGTH
 S 0030 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 15 48 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 DXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ YES ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	N	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	35	1 67	16 57	88.03	24 51	787.31	101.61	09
PLANE2	35	1 67	29 74	197 21	43 79	907 40	101 26	11
MAIN	22	1 33	27 03	2125.70	72 56	973 51	100 81	1 31

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 1953E+04 EXCESS- 1577E+04 OUTFLOW- 3505E+04 BASIN STORAGE- 4498E+01 PERCENT ERROR- 6

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	22	1 33	30 00		72 40	990 00	100 78
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HYDROGRAPH AT STATION

TOTAL RAINFALL - 177 18, TOTAL LOSS - 75 05. TOTAL EXCESS - 102 13

PEAK FLOW (CU M/S)	TIME (HR)	6-HR (CU M/S)	24-HR (MM)	72-HR (MM)	149 50-HR (MM)
72	16 50	63	38 881	36	7
			1357	89 133	99 518
				3110	3472
					3514

CUMULATIVE AREA - 34 89 SQ KM

77 KO OUTPUT CONTROL VARIABLES

000087

JPRINT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

78 BA SUBBASIN CHARACTERISTICS
 TAREA 12 89 SUBBASIN AREA

PRECIPITATION DATA

10 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
 HYDRO-35 TP-40 TP-49
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
 19 40 40 00 77 30 98 00 110 00 148 00 164 00 194 00 00 00 00 00

STORM AREA - 220 00

11 LS SCS LOSS RATE
 STIRL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STIRL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

79 UK KINEMATIC WAVE
 OVERLAND-FLOW ELEMENT NO 1
 L 750 OVERLAND FLOW LENGTH
 S 0100 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 31 9 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS
 80 UK OVERLAND-FLOW ELEMENT NO 2
 L 1590 OVERLAND FLOW LENGTH
 S 0100 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 68 1 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

81 RK KINEMATIC WAVE
 MAIN CHANNEL
 L 5500 CHANNEL LENGTH
 S .0080 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 12 89 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD .00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 WDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ NO ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	25	1 67	27.64	125.04	18 00	887.19	101 30	08
PLANE2	25	1 67	30 00	159 05	22.98	1039 92	101 21	08
MAIN	37	1 33	18 57	1833 93	35.43	944 72	101 21	1 65

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 0000E+00 EXCESS- 1313E+04 OUTFLOW- 1300E+04 BASIN STORAGE- 4098E+01 PERCENT ERROR- .6

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 37 1 33 30 00 35 35 930 00 101 24

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HYDROGRAPH AT STATION

TOTAL RAINFALL - 177 18, TOTAL LOSS - 75 05, TOTAL EXCESS - 102 13

PEAK FLOW (CU M/S)	TIME (HR)	6-HR MAXIMUM AVERAGE FLOW	24-HR	72-HR	149 50-HR	
35	15 50	30	14	5	2	
		(MM)	58.957	93 826	100.449	101.189
		(1000 CU M)	657.	1209	1295	1304

CUMULATIVE AREA - 12 89 SQ KM

86 KO OUTPUT CONTROL VARIABLES
 JPRINT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

87 BA SUBBASIN CHARACTERISTICS
 TAREA 5 28 SUBBASIN AREA

PRECIPITATION DATA

10 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
 HYDRO-35 TP-40 TP-49
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
 19 40 40 00 77 30 98 00 110 00 148 00 164 00 194 00 00 00 00 00

STORM AREA - 220 00

11 LS SCS LOSS RATE

000088

STRTI 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STRTL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

88 UR KINEMATIC WAVE
 OVERLAND-FLOW ELEMENT NO 1
 L 750 OVERLAND FLOW LENGTH
 S 0130 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 42 4 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS
 89 UR OVERLAND-FLOW ELEMENT NO 2
 L 1810 OVERLAND FLOW LENGTH
 S 0130 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 57 6 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

90 RK KINEMATIC WAVE
 MAIN CHANNEL
 L 3000 CHANNEL LENGTH
 S 0030 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 5 28 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD .00 BOTTOM WIDTH OR DIAMETER
 Z 40.00 SIDE SLOPE
 NDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ YES ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	29	1 67	24 62	125.04	10 35	863.66	101 36	08
PLANE2	29	1 67	27.27	144.33	12 06	916 69	101.33	09
MAIN	22	1 33	11.41	1000 33	121 71	978 97	100.78	1.46

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 4802E+04 EXCESS- 5377E+03 OUTFLOW- 5329E+04 BASIN STORAGE- 1575E+01 PERCENT ERROR- .2

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 22 1 33 30.00 121 39 990 00 100 77

HYDROGRAPH AT STATION

TOTAL RAINFALL - 177 18, TOTAL LOSS - 75 05, TOTAL EXCESS - 102 13

PEAR FLOW (CU M/S)	TIME (HR)	MAXIMUM AVERAGE FLOW 6-HR (CU M/S)	24-HR (MM)	72-HR (MM)	149 50-HR (MM)
121	16 50	104	42 514	56	20
		2256	90 464	5289	5344.

CUMULATIVE AREA - 53 06 SQ KM

93 KO OUTPUT CONTROL VARIABLES
 IPRNT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

94 BA SUBBASIN CHARACTERISTICS
 TAREA 14 94 SUBBASIN AREA

PRECIPITATION DATA

10 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
 HYDRO-35 TP-40 TP-49
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
 19 40 40 00 77 30 98 00 110 00 148 00 164 00 194 00 00 00 00 00

STORM AREA - 220.00

11 LS SCS LOSS RATE
 STRTL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STRTL 17 85 INITIAL ABSTRACTION
 CRVNR 74.00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

95 UR KINEMATIC WAVE
 OVERLAND-FLOW ELEMENT NO 1
 L 1868 OVERLAND FLOW LENGTH
 S 0800 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 74 8 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS
 96 UR OVERLAND-FLOW ELEMENT NO. 2
 L 630 OVERLAND FLOW LENGTH
 S 0800 SLOPE

000089

N 400 ROUGHNESS COEFFICIENT
 PA 25 2 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS
 97 RK KINEMATIC WAVE
 MAIN CHANNEL
 L 6000 CHANNEL LENGTH
 S 0080 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 14 94 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 NDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ NO ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	71	1 67	29 40	372.12	51 51	867.83	101.22	.21
PLANE2	71	1 67	12 55	126 04	28 31	768.76	101.75	.17
MAIN	37	1 33	17 56	2000.66	64 45	861.76	101 32	1.91

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 0000E+00 EXCESS- 1522E+04 OUTFLOW- 1509E+04 BASIN STORAGE- 1364E+01 PERCENT ERROR- 8

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 37 1 33 30 00 64 44 870.00 101 22

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HYDROGRAPH AT STATION

TOTAL RAINFALL - 177 18, TOTAL LOSS - 75 05, TOTAL EXCESS - 102 13

PEAK FLOW (CU M/S)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	149 50-HR
64	14 50	45	17	6	3
		(MM) 65 171	98 086	100 911	101 168
		(1000 CU M) 974	1465	1508	1511

CUMULATIVE AREA - 14 94 SQ KM

100 KO OUTPUT CONTROL VARIABLES
 IPRINT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

101 BA SUBBASIN CHARACTERISTICS
 TAREA 7 52 SUBBASIN AREA

PRECIPITATION DATA

HYDRO-35		DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM								TP-49					
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY				
19 40	40 00	77 30	98 00	110 00	148 00	164.00	194 00	.00	.00	.00	.00				

STORM AREA - 220 00

11 LS SCS LOSS RATE
 STRL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STRL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

102 UK KINEMATIC WAVE
 OVERLAND-FLOW ELEMENT NO 1
 L 970 OVERLAND FLOW LENGTH
 S 0300 SLOPE
 N .400 ROUGHNESS COEFFICIENT
 PA 24 3 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

103 UK OVERLAND-FLOW ELEMENT NO 2
 L 800 OVERLAND FLOW LENGTH
 S 0300 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 75 7 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

104 RK KINEMATIC WAVE
 MAIN CHANNEL
 L 4500 CHANNEL LENGTH
 S 0060 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 7 52 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 NDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ YES ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP

000090

CONTINUITY SUMMARY (1000 CU-M) INFLOW- 1506E+04 EXCESS- 7659E+03 OUTFLOW- 2264E+04 BASIN STORAGE- 7091E+00 PERCENT ERROR- 3

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 32 1 33 30 00 99 14 870 00 101 19

HYDROGRAPH AT STATION

TOTAL RAINFALL - 177 18, TOTAL LOSS - 75 05, TOTAL EXCESS - 102 13

PEAK FLOW (CU M/S)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	149 50-HR
99	14 50	68	25	9	4
		(MM)	98 075	100.890	101 139
		(1000 CU M)	1459	2203	2266

CUMULATIVE AREA - 22 46 SQ KM

109 KO OUTPUT CONTROL VARIABLES
 IPRWT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

110 BA SUBBASIN CHARACTERISTICS
 TAREA 3 67 SUBBASIN AREA

PRECIPITATION DATA

HYDRO-35		DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM						TP-49			
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
19 40	40 00	77 30	98 00	110.00	148.00	164.00	194 00	00	00	00	00

STORM AREA - 720 00

111 LS SCS LOSS RATE
 STRFL 17 85 INITIAL ABSTRACTION
 CRVWR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STRFL 17.85 INITIAL ABSTRACTION
 CRVWR 74.00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

KINEMATIC WAVE

111 UK OVERLAND-FLOW ELEMENT NO 1
 L 1160 OVERLAND FLOW LENGTH
 S 0130 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 63 4 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

112 UK OVERLAND-FLOW ELEMENT NO 2
 L 670 OVERLAND FLOW LENGTH
 S 0130 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 36.6 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

KINEMATIC WAVE

113 RK MAIN CHANNEL
 L 2000 CHANNEL LENGTH
 S 0030 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 3.67 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 NDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ YES ROUTE UPSTREAM HYDROGRAPH

COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	29	1 67	30 00	165.77	8 43	915.72	101 23	09
PLANE2	29	1 67	26 93	134 04	6 50	849.45	101 29	08
MAIN	22	1 33	5.52	666 89	488 01	943 22	100.77	2 05

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 1638E+05 EXCESS- 3738E+03 OUTFLOW- 1674E+05 BASIN STORAGE- .1622E+01 PERCENT ERROR- .1

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 22 1 33 30.00 485.95 960.00 100 76

HYDROGRAPH AT STATION

TOTAL RAINFALL - 177.18, TOTAL LOSS - 75.05, TOTAL EXCESS - 102.13

PEAK FLOW (CU M/S)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	149 50-HR
486	16 00	390	180	64	31
		(MM) 50 495	93 342	99 989	100 787
		(1000 CU M) 8417	15560	16668	16788

CUMULATIVE AREA - 166 70 SQ KM

116 KO OUTPUT CONTROL VARIABLES
 IPRINT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

117 BA SUBBASIN CHARACTERISTICS
 TAREA 6.44 SUBBASIN AREA

PRECIPITATION DATA

5-MIN	HYDRO-35				DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM				TP-49		
	15-MIN	60-MIN	2-HR	3-HR	TP-48	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY
19 40	40 00	77 30	98 00	110 00	148 00	164 00	194.00	00	00	00	00

STORM AREA - 220.00

11 LS SCS LOSS RATE
 SIRTL 17.85 INITIAL ABSTRACTION
 CRVWBR 74.00 CURVE NUMBER
 RIIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 SIRTL 17.85 INITIAL ABSTRACTION
 CRVWBR 74.00 CURVE NUMBER
 RIIMP 00 PERCENT IMPERVIOUS AREA

KINEMATIC WAVE

118 UK OVERLAND-FLOW ELEMENT NO. 1
 L 490 OVERLAND FLOW LENGTH
 S 0500 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 30.5 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

119 UK OVERLAND-FLOW ELEMENT NO. 2
 L 1120 OVERLAND FLOW LENGTH
 S 0500 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 69.5 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

KINEMATIC WAVE

120 RK MAIN CHANNEL
 L 4000 CHANNEL LENGTH
 S 0100 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 6.44 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40.00 SIDE SLOPE
 NDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ NO ROUTE UPSTREAM HYDROGRAPH

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	N	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
FLANE1	.56	1.67	12.42	98.03	14.84	779.60	101.76	13
FLANE2	.56	1.67	23.84	224.07	23.06	825.58	101.36	16
MAIN	.41	1.33	12.73	1333.77	33.02	824.74	101.48	1.76

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 0000E+00 EXCESS- 6559E+03 OUTFLOW- .6514E+03 BASIN STORAGE- 4270E+00 PERCENT ERROR- 6

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	41	1.33	30.00	32.63	810.00	101.47
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HYDROGRAPH AT STATION

TOTAL RAINFALL - 177.18, TOTAL LOSS - 75.05, TOTAL EXCESS - 102.13

PEAK FLOW (CU M/S)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	149 50-HR
33	13 50	21	7	3	1
		(MM) 68 780	99 227	101 250	181.419
		(1000 CU M) 443	639	652	653

CUMULATIVE AREA - 6.44 SQ KM

123 KO OUTPUT CONTROL VARIABLES
 IPRINT 3 PRINT CONTROL

000092

IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

124 BA SUBBASIN CHARACTERISTICS
 TAREA 8 85 SUBBASIN AREA

PRECIPITATION DATA

10 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
 HYDRO-35 TP-40 TP-49
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
 19 40 40 00 77 30 98 00 110 00 148 00 164 00 194 00 .00 .00 .00 .00
 STORM AREA - 220 00

11 LS SCS LOSS RATE
 STRIL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STRIL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

125 UK KINEMATIC WAVE
 OVERLAND-FLOW ELEMENT NO 1
 L 910 OVERLAND FLOW LENGTH
 S 0300 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 51 5 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

126 UK OVERLAND-FLOW ELEMENT NO 2
 L 860 OVERLAND FLOW LENGTH
 S 0300 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 48 5 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

127 RK KINEMATIC WAVE
 MAIN CHANNEL
 L 5000 CHANNEL LENGTH
 S .0080 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 8 85 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 NDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ YES ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	43	1 67	24 73	182 06	23 07	832 33	101 32	12
PLANE2	43	1 67	23 60	172 06	22 20	824 40	101 37	12
MAIN	37	1 33	14 41	1667 21	75 64	854 81	101 43	1 93

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 6508E+03 EXCESS- .9013E+03 OUTFLOW- 1546E+04 BASIN STORAGE- 8552E+00 PERCENT ERROR- 4

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 37 1 33 30 00 74 74 870 00 101 46

HYDROGRAPH AT STATION

TOTAL RAINFALL - 177 18, TOTAL LOSS - 75 05, TOTAL EXCESS - 102 13

PEAK FLOW (CU M/S)	TIME (HR)	6-HR (CU M/S)	24-HR (MM)	72-HR (1000 CU M)	149 50-HR (1000 CU M)
75	14 50	48	17	6	3
		67 193	98 776	101 203	101 412
		1027	1510	1547	1551

CUMULATIVE AREA - 15 29 SQ KM

130 KO OUTPUT CONTROL VARIABLES
 IPRNT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

131 BA SUBBASIN CHARACTERISTICS
 TAREA 19 60 SUBBASIN AREA

PRECIPITATION DATA

10 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
 HYDRO-35 TP-40 TP-49
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
 19 40 40 00 77 30 98 00 110 00 148 00 164 00 194 00 .00 .00 .00 .00
 STORM AREA - 220 00

11 LS SCS LOSS RATE
 STRIL 17 85 INITIAL ABSTRACTION

000093

CRVNR 74 00 CURVE NUMBER
RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
SIRL 17 85 INITIAL ABSTRACTION
CRVNR 74 00 CURVE NUMBER
RTIMP 00 PERCENT IMPERVIOUS AREA

KINEMATIC WAVE
132 UR OVERLAND-FLOW ELEMENT NO 1
L 780 OVERLAND FLOW LENGTH
S 0130 SLOPE
N 400 ROUGHNESS COEFFICIENT
PA 37 9 PERCENT OF SUBBASIN
DXMIN 5 MINIMUM NUMBER OF DX INTERVALS
133 UR OVERLAND-FLOW ELEMENT NO 2
L 1280 OVERLAND FLOW LENGTH
S 0130 SLOPE
N 400 ROUGHNESS COEFFICIENT
PA 62 1 PERCENT OF SUBBASIN
DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

KINEMATIC WAVE
134 RK MAIN CHANNEL
L 9500 CHANNEL LENGTH
S 0030 SLOPE
N 045 CHANNEL ROUGHNESS COEFFICIENT
CA 19 60 CONTRIBUTING AREA
SHAPE TRAP CHANNEL SHAPE
WD 00 BOTTOM WIDTH OR DIAMETER
Z 40 00 SIDE SLOPE
NDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
RUPSTQ NO ROUTE UPSTREAM HYDROGRAPH

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	29	1 67	25 37	130.04	33 83	870.43	101.40	.09
PLANE2	29	1 67	29 68	160.05	41 12	938.54	101.31	.09
MAIN	22	1 33	30.00	2375.78	64.32	969.24	101.22	1 30

CONTINUITY SUMMARY (1000 CU-M)-INFLOW= 0000E+00 EXCESS= 1996E+04 OUTFLOW= .1977E+04 BASIN STORAGE= .4511E+01 PERCENT ERROR= 7

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 22 1 33 30 00 63 62 960 00 101 16

HYDROGRAPH AT STATION

TOTAL RAINFALL = 177 18, TOTAL LOSS = 75 05, TOTAL EXCESS = 102 13

PEAK FLOW (CU M/S)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR (CU M/S)	24-HR (MM)	72-HR (1000 CU M)	149 50-HR (101 103)
64	16 00	50.	22	8	4
		54 880	95 019	100.561	101 103
		1076	1862	1971	1982

CUMULATIVE AREA = 19 60 SQ KM

139 KO OUTPUT CONTROL VARIABLES
IPRNT 1 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

140 BA SUBBASIN CHARACTERISTICS
TAREA 4 11 SUBBASIN AREA

PRECIPITATION DATA

10 PH DEPIHS FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35		TP-40						TP-49			
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
19 40	40 00	77 30	98 00	110 00	140.00	164 00	194 00	00	00	.00	.00

STORM AREA = 220 00

11 LS SCS LOSS RATE
SIRL 17 85 INITIAL ABSTRACTION
CRVNR 74 00 CURVE NUMBER
RTIMP .00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
SIRL 17 85 INITIAL ABSTRACTION
CRVNR 74 00 CURVE NUMBER
RTIMP 00 PERCENT IMPERVIOUS AREA

KINEMATIC WAVE
141 UR OVERLAND-FLOW ELEMENT NO 1
L 450 OVERLAND FLOW LENGTH
S 0180 SLOPE
N 400 ROUGHNESS COEFFICIENT
PA 32 6 PERCENT OF SUBBASIN
DXMIN 5 MINIMUM NUMBER OF DX INTERVALS
142 UR OVERLAND-FLOW ELEMENT NO 2
L 920 OVERLAND FLOW LENGTH
S 0100 SLOPE
N 400 ROUGHNESS COEFFICIENT

000094

PA 67 3 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

143 RK KINEMATIC WAVE
 MAIN CHANNEL

L 3000 CHANNEL LENGTH
 S 0030 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 4 11 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 NOXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ YES ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	N	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	25	1 67	21 97	90.03	7.21	011 44	101.43	07
PLANE2	25	1 67	20 21	131 47	10.73	096 27	101 30	00
MAIN	22	1 33	7 02	1000 33	612 75	950 26	100 69	2 17

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 2025E+05 EXCESS- 4102E+03 OUTFLOW- 2064E+05 BASIN STORAGE- 2263E+01 PERCENT ERROR- 1

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 22 1 33 30 00 612 10 960 00 100 63

HYDROGRAPH AT STATION

DA	MON	HRMM	ORD	RAIN	LOSS	EXCESS	COMP Q	DA	MON	HRMM	ORD	RAIN	LOSS	EXCESS	COMP Q
15	APR	1500	1	00	00	00	0	10	APR	1800	151	00	.00	00	2.
15	APR	1530	2	1 10	1 10	00	0	10	APR	1830	152	00	.00	00	2.
15	APR	1600	3	1 13	1 13	00	0	10	APR	1900	153	00	.00	00	2.
15	APR	1630	4	1 17	1 17	00	0	10	APR	1930	154	.00	.00	.00	2.
15	APR	1700	5	1 20	1 20	00	0	10	APR	2000	155	.00	.00	.00	2.
15	APR	1730	6	1 25	1 25	00	0	10	APR	2030	156	.00	.00	.00	2.
15	APR	1800	7	1 29	1 29	00	0	10	APR	2100	157	.00	.00	.00	2.
15	APR	1830	8	1 35	1 35	00	0	10	APR	2130	158	.00	.00	.00	2.
15	APR	1900	9	1 40	1 40	00	0	10	APR	2200	159	.00	.00	.00	2.
15	APR	1930	10	1 46	1 46	00	0.	10	APR	2230	160	.00	.00	.00	2.
15	APR	2000	11	1 53	1 53	00	0	10	APR	2300	161	.00	.00	.00	2.
15	APR	2030	12	1 61	1 61	00	0	10	APR	2330	162	.00	.00	.00	2.
15	APR	2100	13	1 70	1 70	00	0	10	APR	0000	163	.00	.00	.00	2.
15	APR	2130	14	1 36	1 36	00	0	10	APR	0030	164	.00	.00	.00	1.
15	APR	2200	15	1 46	1 45	02	0	10	APR	0100	165	.00	.00	.00	1.
15	APR	2230	16	1 50	1 51	07	0	10	APR	0130	166	.00	.00	.00	1.
15	APR	2300	17	1 72	1 59	13	0	10	APR	0200	167	.00	.00	.00	1.
15	APR	2330	18	1 89	1 69	21	0	10	APR	0230	168	.00	.00	.00	1.
16	APR	0000	19	2 10	1 79	31	0	10	APR	0300	169	.00	.00	.00	1.
16	APR	0030	20	5 41	4 20	1 13	0	10	APR	0330	170	.00	.00	.00	1.
16	APR	0100	21	5 94	4 21	1 73	0	10	APR	0400	171	.00	.00	.00	1.
16	APR	0130	22	6 66	4 21	2 45	0	10	APR	0430	172	.00	.00	.00	1.
16	APR	0200	23	6 27	3 54	2 73	0	10	APR	0500	173	.00	.00	.00	1.
16	APR	0230	24	10 15	5 02	5 13	1	10	APR	0530	174	.00	.00	.00	1.
16	APR	0300	25	19 91	7 91	12 00	4	10	APR	0600	175	.00	.00	.00	1.
16	APR	0330	26	31 34	8 97	22 36	13	10	APR	0630	176	.00	.00	.00	1.
16	APR	0400	27	11 96	2 67	9 29	46	10	APR	0700	177	.00	.00	.00	1.
16	APR	0430	28	7 06	1 43	5 64	140	10	APR	0730	178	.00	.00	.00	1.
16	APR	0500	29	7 14	1 35	5 80	209	10	APR	0800	179	.00	.00	.00	1.
16	APR	0530	30	6 27	1 11	5 16	433	10	APR	0830	180	.00	.00	.00	1.
16	APR	0600	31	5 65	.95	4 71	530	10	APR	0900	181	.00	.00	.00	1.
16	APR	0630	32	2 23	.36	1 87	596.	10	APR	0930	182	.00	.00	.00	1.
16	APR	0700	33	1 99	.32	1 67	613	10	APR	1000	183	.00	.00	.00	1.
16	APR	0730	34	1 80	.20	1 52	599	10	APR	1030	184	.00	.00	.00	1.
16	APR	0800	35	1 64	.25	1 39	567	10	APR	1100	185	.00	.00	.00	1.
16	APR	0830	36	1 52	.23	1 29	526	10	APR	1130	186	.00	.00	.00	1.
16	APR	0900	37	1 41	.21	1 20	481	10	APR	1200	187	.00	.00	.00	1.
16	APR	0930	38	1 75	.26	1 49	437	10	APR	1230	188	.00	.00	.00	1.
16	APR	1000	39	1 65	.24	1 41	397	10	APR	1300	189	.00	.00	.00	1.
16	APR	1030	40	1 57	.23	1 35	361	10	APR	1330	190	.00	.00	.00	1.
16	APR	1100	41	1 50	.21	1 29	331	10	APR	1400	191	.00	.00	.00	1.
16	APR	1130	42	1 43	.20	1 23	306.	10	APR	1430	192	.00	.00	.00	1.
16	APR	1200	43	1 37	.19	1 18	285	10	APR	1500	193	.00	.00	.00	1.
16	APR	1230	44	1 32	.18	1 14	268	10	APR	1530	194	.00	.00	.00	1.
16	APR	1300	45	1 27	.17	1 10	253	10	APR	1600	195	.00	.00	.00	1.
16	APR	1330	46	1 23	.16	1 06	240	10	APR	1630	196	.00	.00	.00	1.
16	APR	1400	47	1 18	.16	1 03	228	10	APR	1700	197	.00	.00	.00	1.
16	APR	1430	48	1 15	.15	1 00	217.	10	APR	1730	198	.00	.00	.00	1.
16	APR	1500	49	1 11	.14	.97	206	10	APR	1800	199	.00	.00	.00	1.
16	APR	1530	50	.00	.00	.00	195	10	APR	1830	200	.00	.00	.00	1.
16	APR	1600	51	.00	.00	.00	185	10	APR	1900	201	.00	.00	.00	1.
16	APR	1630	52	.00	.00	.00	174.	10	APR	1930	202	.00	.00	.00	1.
16	APR	1700	53	.00	.00	.00	163.	10	APR	2000	203	.00	.00	.00	1.
16	APR	1730	54	.00	.00	.00	152	10	APR	2030	204	.00	.00	.00	1.
16	APR	1800	55	.00	.00	.00	141	10	APR	2100	205	.00	.00	.00	1.
16	APR	1830	56	.00	.00	.00	130.	10	APR	2130	206	.00	.00	.00	1.
16	APR	1900	57	.00	.00	.00	120	10	APR	2200	207	.00	.00	.00	1.
16	APR	1930	58	.00	.00	.00	110	10	APR	2230	208	.00	.00	.00	1.
16	APR	2000	59	.00	.00	.00	102	10	APR	2300	209	.00	.00	.00	1.
16	APR	2030	60	.00	.00	.00	94	10	APR	2330	210	.00	.00	.00	1.
16	APR	2100	61	.00	.00	.00	86	10	APR	0000	211	.00	.00	.00	1.
16	APR	2130	62	.00	.00	.00	79	10	APR	0030	212	.00	.00	.00	1.
16	APR	2200	63	.00	.00	.00	73	10	APR	0100	213	.00	.00	.00	1.
16	APR	2230	64	.00	.00	.00	68	10	APR	0130	214	.00	.00	.00	1.
16	APR	2300	65	.00	.00	.00	63	10	APR	0200	215	.00	.00	.00	1.
16	APR	2330	66	.00	.00	.00	59	10	APR	0230	216	.00	.00	.00	1.
17	APR	0000	67	.00	.00	.00	55	10	APR	0300	217	.00	.00	.00	1.
17	APR	0030	68	.00	.00	.00	51	10	APR	0330	218	.00	.00	.00	1.

000095

17 APR 0100	69	00	00	00	47	*	20 APR 0400	219	00	.00	.00	1
17 APR 0130	70	00	00	00	44	*	20 APR 0430	220	00	00	00	1
17 APR 0200	71	00	00	00	41	*	20 APR 0500	221	00	00	00	1
17 APR 0230	72	00	00	00	39	*	20 APR 0530	222	.00	00	00	1.
17 APR 0300	73	00	00	00	36	*	20 APR 0600	223	00	.00	00	1
17 APR 0330	74	00	00	00	34	*	20 APR 0630	224	00	.00	00	1
17 APR 0400	75	00	00	00	32	*	20 APR 0700	225	00	.00	00	1.
17 APR 0430	76	00	00	00	30	*	20 APR 0730	226	00	00	00	1
17 APR 0500	77	00	00	00	28	*	20 APR 0800	227	00	00	00	1
17 APR 0530	78	00	00	00	27	*	20 APR 0830	228	00	00	00	1
17 APR 0600	79	00	00	00	25	*	20 APR 0900	229	00	00	00	1
17 APR 0630	80	00	00	00	24	*	20 APR 0930	230	00	00	00	0.
17 APR 0700	81	00	00	.00	23	*	20 APR 1000	231	.00	00	00	0.
17 APR 0730	82	00	00	00	21	*	20 APR 1030	232	00	.00	00	0
17 APR 0800	83	00	00	00	20	*	20 APR 1100	233	00	00	00	0
17 APR 0830	84	00	00	00	19	*	20 APR 1130	234	00	.00	00	0
17 APR 0900	85	00	00	00	18	*	20 APR 1200	235	00	00	00	0
17 APR 0930	86	00	00	00	17	*	20 APR 1230	236	00	00	00	0
17 APR 1000	87	00	00	.00	16	*	20 APR 1300	237	00	00	00	0
17 APR 1030	88	00	00	00	16	*	20 APR 1330	238	00	.00	00	0
17 APR 1100	89	00	00	00	15	*	20 APR 1400	239	.00	00	00	0
17 APR 1130	90	00	00	00	14	*	20 APR 1430	240	.00	00	00	0
17 APR 1200	91	00	00	00	14	*	20 APR 1500	241	00	00	00	0
17 APR 1230	92	00	00	00	13	*	20 APR 1530	242	00	00	00	0
17 APR 1300	93	00	00	00	12	*	20 APR 1600	243	00	00	00	0
17 APR 1330	94	00	00	00	12	*	20 APR 1630	244	00	.00	00	0.
17 APR 1400	95	00	00	00	11	*	20 APR 1700	245	00	00	00	0
17 APR 1430	96	00	00	00	11	*	20 APR 1730	246	00	00	00	0
17 APR 1500	97	00	00	00	10	*	20 APR 1800	247	00	00	00	0
17 APR 1530	98	00	00	00	10	*	20 APR 1830	248	00	00	00	0
17 APR 1600	99	00	00	00	10	*	20 APR 1900	249	00	.00	00	0
17 APR 1630	100	00	00	00	9	*	20 APR 1930	250	.00	.00	00	0
17 APR 1700	101	00	00	00	9	*	20 APR 2000	251	00	00	00	0
17 APR 1730	102	00	00	00	8	*	20 APR 2030	252	.00	00	.00	0
17 APR 1800	103	00	00	00	8	*	20 APR 2100	253	00	00	00	0
17 APR 1830	104	00	00	00	8	*	20 APR 2130	254	00	00	00	0
17 APR 1900	105	00	00	00	8	*	20 APR 2200	255	00	00	00	0
17 APR 1930	106	00	00	00	7	*	20 APR 2230	256	00	00	00	0
17 APR 2000	107	00	00	00	7	*	20 APR 2300	257	00	00	00	0
17 APR 2030	108	00	00	00	7	*	20 APR 2330	258	00	00	00	0.
17 APR 2100	109	00	00	00	7	*	21 APR 0000	259	00	00	00	0
17 APR 2130	110	00	00	00	6	*	21 APR 0030	260	00	00	00	0
17 APR 2200	111	00	00	00	6	*	21 APR 0100	261	00	00	00	0
17 APR 2230	112	00	00	00	6	*	21 APR 0130	262	00	.00	00	0.
17 APR 2300	113	00	00	00	6	*	21 APR 0200	263	00	00	00	0
17 APR 2330	114	.00	00	00	5	*	21 APR 0230	264	.00	.00	.00	0.
18 APR 0000	115	00	00	00	5	*	21 APR 0300	265	00	00	00	0
18 APR 0030	116	00	00	00	5	*	21 APR 0330	266	00	00	00	0
18 APR 0100	117	00	00	00	5	*	21 APR 0400	267	00	00	00	0
18 APR 0130	118	00	00	00	5	*	21 APR 0430	268	00	00	00	0
18 APR 0200	119	00	00	00	5	*	21 APR 0500	269	00	00	00	0
18 APR 0230	120	00	00	00	5	*	21 APR 0530	270	00	.00	00	0.
18 APR 0300	121	00	00	00	4	*	21 APR 0600	271	.00	00	00	0
18 APR 0330	122	00	00	00	4	*	21 APR 0630	272	00	00	00	0
18 APR 0400	123	00	00	00	4	*	21 APR 0700	273	00	00	00	0.
18 APR 0430	124	00	00	00	4	*	21 APR 0730	274	00	.00	00	0
18 APR 0500	125	00	00	00	4	*	21 APR 0800	275	00	00	00	0
18 APR 0530	126	00	00	00	4	*	21 APR 0830	276	00	00	00	0
18 APR 0600	127	00	00	00	4	*	21 APR 0900	277	.00	.00	00	0.
18 APR 0630	128	00	00	00	4	*	21 APR 0930	278	00	00	00	0
18 APR 0700	129	00	00	00	3	*	21 APR 1000	279	00	.00	00	0
18 APR 0730	130	00	00	00	3	*	21 APR 1030	280	00	00	.00	0
18 APR 0800	131	00	00	00	3	*	21 APR 1100	281	00	00	00	0
18 APR 0830	132	00	00	00	3	*	21 APR 1130	282	.00	00	.00	0
18 APR 0900	133	00	00	00	3	*	21 APR 1200	283	00	00	00	0.
18 APR 0930	134	00	00	00	3	*	21 APR 1230	284	00	00	00	0
18 APR 1000	135	00	00	00	3	*	21 APR 1300	285	00	00	00	0.
18 APR 1030	136	00	00	00	3	*	21 APR 1330	286	00	00	00	0
18 APR 1100	137	00	00	00	3	*	21 APR 1400	287	00	00	00	0
18 APR 1130	138	00	00	00	3	*	21 APR 1430	288	00	.00	00	0
18 APR 1200	139	00	00	00	3	*	21 APR 1500	289	00	.00	00	0.
18 APR 1230	140	00	00	00	3	*	21 APR 1530	290	00	00	00	0
18 APR 1300	141	00	00	00	2	*	21 APR 1600	291	.00	00	00	0
18 APR 1330	142	00	00	00	2	*	21 APR 1630	292	00	00	00	0
18 APR 1400	143	00	00	00	2	*	21 APR 1700	293	.00	00	.00	0
18 APR 1430	144	00	00	00	2	*	21 APR 1730	294	00	00	00	0
18 APR 1500	145	00	00	00	2	*	21 APR 1800	295	00	00	00	0
18 APR 1530	146	00	00	00	2	*	21 APR 1830	296	00	00	00	0.
18 APR 1600	147	00	00	00	2	*	21 APR 1900	297	00	.00	00	0
18 APR 1630	148	00	00	00	2	*	21 APR 1930	298	00	00	00	0
18 APR 1700	149	00	00	00	2	*	21 APR 2000	299	00	00	00	0
18 APR 1730	150	00	00	00	2	*	21 APR 2030	300	00	00	00	0

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TOTAL RAINFALL - 177.18, TOTAL LOSS - 75.15, TOTAL EXCESS - 102.03

PEAK FLOW (CU M/S)	TIME (HR)	6-HR (CU M/S)	24-HR MAXIMUM AVERAGE FLOW	72-HR	149.50-HR
613	16 00	488	223	79	38
		(MM) 51 269	93 534	99 910	100 579
		(1000 CU M) 10546	19240.	20551.	20689

CUMULATIVE AREA - 205 70 SQ KM

145 KO OUTPUT CONTROL VARIABLES

IPRINT	1	PRINT CONTROL
IPL0T	0	PLOT CONTROL
QSCAL	0	HYDROGRAPH PLOT SCALE

HYDROGRAPH ROUTING DATA

146 RS STORAGE ROUTING

NSIPS	1	NUMBER OF SUBREACHES
IIYP	STOR	TYPE OF INITIAL CONDITION
RSVRIC	30000.00	INITIAL CONDITION
X	-1.00	WORKING R AND D COEFFICIENT

147 SV STORAGE 0 18043.0 22516.0 28092.3 33668.4 39244.4

000096

140 SE ELEVATION 97 00 98 00 99 00 100.00 101 00 102 00
 149 SS SPILLWAY
 CREL 100 50 SPILLWAY CREST ELEVATION
 SPWD 130 00 SPILLWAY WIDTH
 COQW 1 40 WEIR COEFFICIENT
 EXPW 1 50 EXPONENT OF HEAD

COMPUTED OUTFLOW-ELEVATION DATA

OUTFLOW	00	00	06	46	1 55	3 67	7.17	12 38	19 66	29 35
ELEVATION	97 00	100 50	100 50	100 52	100 54	100.57	100 62	100 67	100 73	100 80
OUTFLOW	41 79	57 33	76 31	99 07	125 96	157 32	193 49	234 83	281 67	334 36
ELEVATION	100 88	100 96	101 06	101 17	101 28	101 41	101 54	101 69	101 84	102 00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	00	18043 00	22516.00	28092.30	30880 35	30983 60	31112 67	31293 39	31525 71	31809.69
OUTFLOW	00	00	00	.00	00	46	1.55	3 67	7 17	12 38
ELEVATION	97 00	98 00	99 00	100 00	100 50	100 52	100 54	100 57	100 62	100 67
STORAGE	32145 30	32532 52	32971 39	33461 86	33668 40	34004.01	34597 72	35243 11	35940.11	36688 72
OUTFLOW	19 66	29 35	41 79	57 33	64 35	76 31	99 07	125.96	157 32	193 49
ELEVATION	100 73	100 80	100 88	100 96	101 00	101 06	101 17	101.28	101 41	101 54
STORAGE	37489 01	38340 86	39244 40							
OUTFLOW	234 83	281 67	334.36							
ELEVATION	101 69	101 84	102 00							

HYDROGRAPH AT STATION BARR

DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE
15	APR	1500	1	0	30000 0	100 3	17	APR	1700	101	55	33380 8	100 9	19	APR	1900	201	8	31588 4	100 6
15	APR	1530	2	0	30000 0	100 3	17	APR	1730	102	53	33339.9	100.9	19	APR	1930	202	8	31581.6	100 6
15	APR	1600	3	0	30000 0	100 3	17	APR	1800	103	52	33299 9	100 9	19	APR	2000	203	8	31575 0	100 6
15	APR	1630	4	0	30000 0	100 3	17	APR	1830	104	51	33260 6	100 9	19	APR	2030	204	8	31568.4	100.6
15	APR	1700	5	0	30000.0	100 3	17	APR	1900	105	50	33222 3	100 9	19	APR	2100	205	8	31562.0	100 6
15	APR	1730	6	0	30000 0	100 3	17	APR	1930	106	49	33184 7	100 9	19	APR	2130	206	8	31555.6	100.6
15	APR	1800	7	0	30000 0	100 3	17	APR	2000	107	47	33148.0	100.9	19	APR	2200	207	8	31549 3	100 6
15	APR	1830	8	0	30000 0	100 3	17	APR	2030	108	46	33112 1	100.9	19	APR	2230	208	7	31543 2	100.6
15	APR	1900	9	0	30000 0	100 3	17	APR	2100	109	45	33076.9	100.9	19	APR	2300	209	7	31537.1	100 6
15	APR	1930	10	0	30000 0	100 3	17	APR	2130	110	44	33042 6	100 9	19	APR	2330	210	7	31531 1	100.6
15	APR	2000	11	0	30000 0	100 3	17	APR	2200	111	43	33009.8	100.9	20	APR	0000	211	7	31525.2	100 6
15	APR	2030	12	0	30000 0	100 3	17	APR	2230	112	42	32976 1	100 9	20	APR	0030	212	7	31519 4	100.6
15	APR	2100	13	0	30000 0	100 3	17	APR	2300	113	41	32944.0	100 9	20	APR	0100	213	7	31513 6	100 6
15	APR	2130	14	0	30000 0	100 3	17	APR	2330	114	40	32912.5	100.9	20	APR	0130	214	7	31508 0	100 6
15	APR	2200	15	0	30000 0	100 3	18	APR	0000	115	39	32881 6	100.9	20	APR	0200	215	7	31502 3	100 6
15	APR	2230	16	0	30000 0	100 3	18	APR	0030	116	38	32851.4	100.9	20	APR	0230	216	7	31496 8	100.6
15	APR	2300	17	0	30000.0	100 3	18	APR	0100	117	38	32821 0	100 8	20	APR	0300	217	7	31491 3	100 6
15	APR	2330	18	0	30000 0	100 3	18	APR	0130	118	37	32792 7	100.8	20	APR	0330	218	7	31485 9	100.6
16	APR	0000	19	0	30000 0	100 3	18	APR	0200	119	36	32764.3	100.8	20	APR	0400	219	6	31480.6	100.6
16	APR	0030	20	0	30000 1	100 3	18	APR	0230	120	35	32736 5	100 8	20	APR	0430	220	6	31475.3	100 6
16	APR	0100	21	0	30000 1	100 3	18	APR	0300	121	34	32709 2	100 8	20	APR	0500	221	6	31470 1	100 6
16	APR	0130	22	0	30000 3	100 3	18	APR	0330	122	34	32682 5	100 8	20	APR	0530	222	6	31464.9	100 6
16	APR	0200	23	0	30000 6	100 3	18	APR	0400	123	33	32656 4	100 8	20	APR	0600	223	6	31459 8	100 6
16	APR	0230	24	0	30001 4	100 3	18	APR	0430	124	32	32630 8	100 8	20	APR	0630	224	6	31454 8	100.6
16	APR	0300	25	0	30003 8	100 3	18	APR	0500	125	31	32605 7	100 8	20	APR	0700	225	6	31449 8	100.6
16	APR	0330	26	0	30011 4	100 3	18	APR	0530	126	31	32581 2	100 8	20	APR	0730	226	6	31444.9	100 6
16	APR	0400	27	0	30038 1	100 3	18	APR	0600	127	30	32557 2	100.8	20	APR	0800	227	6	31440 0	100 6
16	APR	0430	28	0	30121 8	100 4	18	APR	0630	128	29	32533 7	100 8	20	APR	0830	228	6	31435 2	100 6
16	APR	0500	29	0	30314 7	100 4	18	APR	0700	129	29	32510.6	100.8	20	APR	0900	229	6	31430 5	100 6
16	APR	0530	30	0	30639 8	100 5	18	APR	0730	130	28	32488 0	100 8	20	APR	0930	230	6	31425 8	100.6
16	APR	0600	31	1	31076 4	100 5	18	APR	0800	131	28	32465 8	100 8	20	APR	1000	231	6	31421 2	100.6
16	APR	0630	32	8	31852 5	100 6	18	APR	0830	132	27	32444 1	100 8	20	APR	1030	232	6	31416 6	100 6
16	APR	0700	33	19	32114 1	100.7	18	APR	0900	133	27	32422.7	100 8	20	APR	1100	233	5	31412 1	100.6
16	APR	0730	34	32	32636 3	100 8	18	APR	0930	134	26	32401 7	100 8	20	APR	1130	234	5	31407 7	100 6
16	APR	0800	35	47	33125 7	100 9	18	APR	1000	135	26	32381 1	100 8	20	APR	1200	235	5	31403 3	100.6
16	APR	0830	36	61	33569 3	101 0	18	APR	1030	136	25	32361 0	100 8	20	APR	1230	236	5	31398 9	100 6
16	APR	0900	37	75	33961 6	101 1	18	APR	1100	137	25	32341 2	100 8	20	APR	1300	237	5	31394 6	100 6
16	APR	0930	38	88	34301 9	101 1	18	APR	1130	138	24	32321 7	100 8	20	APR	1330	238	5	31390 4	100 6
16	APR	1000	39	99	34593 2	101 2	18	APR	1200	139	24	32302 7	100 8	20	APR	1400	239	5	31386 2	100 6
16	APR	1030	40	109	34840 4	101 2	18	APR	1230	140	23	32284 0	100 8	20	APR	1430	240	5	31382 0	100.6
16	APR	1100	41	118	35049 6	101 2	18	APR	1300	141	23	32265 7	100 7	20	APR	1500	241	5	31378 0	100 6
16	APR	1130	42	125	35226 5	101 3	18	APR	1330	142	22	32247 7	100 7	20	APR	1530	242	5	31373 9	100 6
16	APR	1200	43	132	35376 5	101 3	18	APR	1400	143	22	32230 0	100 7	20	APR	1600	243	5	31369 9	100 6
16	APR	1230	44	138	35503 8	101 3	18	APR	1430	144	21	32212 7	100 7	20	APR	1630	244	5	31366 0	100 6
16	APR	1300	45	143	35612 1	101 3	18	APR	1500	145	21	32195 8	100.7	20	APR	1700	245	5	31362 1	100 6
16	APR	1330	46	147	35703 6	101 4	18	APR	1530	146	21	32179 2	100.7	20	APR	1730	246	5	31358 3	100 6
16	APR	1400	47	150	35780 5	101 4	18	APR	1600	147	20	32162 9	100.7	20	APR	1800	247	5	31354.5	100 6
16	APR	1430	48	153	35844 1	101 4	18	APR	1630	148	20	32146 9	100 7	20	APR	1830	248	5	31350 7	100 6
16	APR	1500	49	155	35895 5	101 4	18	APR	1700	149	19	32131 2	100 7	20	APR	1900	249	4	31347 0	100 6
16	APR	1530	50	157	35935 4	101 4	18	APR	1730	150	19	32115 7	100.7	20	APR	1930	250	4	31343 4	100.6
16	APR	1600	51	158	35964 5	101 4	18	APR	1800	151	19	32100 6	100 7	20	APR	2000	251	4	31339 7	100 6
16	APR	1630	52	159	35982 9	101 4	18	APR	1830	152	18	32085 6	100 7	20	APR	2030	252	4	31336 2	100.6
16	APR	1700	53	160	35990 9	101 4	18	APR	1900	153	18	32071 0	100 7	20	APR	2100	253	4	31332 6	100 6
16	APR	1730	54	160	35988 8	101 4	18	APR	1930	154	18	32056.5	100.7	20	APR	2130	254	4	31329 2	100 6
16	APR	1800	55	159	35976 9	101 4	18	APR	2000	155	17	32042 3	100 7	20	APR	2200	255	4	31325 7	100.6
16	APR	1830	56	158	35956 0	101 4	18	APR	2030	156	17	32028 4	100.7	2						

17 APR 0300	73	113	34940	0	101 2	*	19 APR 0500	173	13	31824	4	100 7	*	21 APR 0700	273	3.	31270.4	100 6
17 APR 0330	74	110	34870	9	101 2	*	19 APR 0530	174	12	31814	2	100 7	*	21 APR 0730	274	3	31267 6	100 6
17 APR 0400	75	108	34802	3	101 2	*	19 APR 0600	175	12	31804	1	100 7	*	21 APR 0800	275	3	31264 8	100 6
17 APR 0430	76	105	34734	5	101 2	*	19 APR 0630	176	12	31794	2	100 7	*	21 APR 0830	276	3.	31262 1	100 6
17 APR 0500	77	102	34667	7	101 2	*	19 APR 0700	177	12	31784	4	100 7	*	21 APR 0900	277	3	31259 4	100 6
17 APR 0530	78	99	34601	8	101 2	*	19 APR 0730	178	12	31774	6	100 7	*	21 APR 0930	278	3	31256 7	100 6
17 APR 0600	79	97	34536	9	101 2	*	19 APR 0800	179	12	31765	3	100 7	*	21 APR 1000	279	3	31254 1	100 6
17 APR 0630	80	94	34472	9	101 1	*	19 APR 0830	180	11	31756	0	100 7	*	21 APR 1030	280	3	31251 5	100 6
17 APR 0700	81	92	34410	0	101 1	*	19 APR 0900	181	11	31746	.8	100 7	*	21 APR 1100	281	3	31248 .9	100 6
17 APR 0730	82	89	34348	1	101 1	*	19 APR 0930	182	11	31737	.7	100 7	*	21 APR 1130	282	3.	31246 3	100 6
17 APR 0800	83	87	34287	3	101 1	*	19 APR 1000	183	11	31728	.8	100 7	*	21 APR 1200	283	3	31243 7	100 6
17 APR 0830	84	85	34227	6	101 1	*	19 APR 1030	184	11	31719	9	100 7	*	21 APR 1230	284	3	31241 2	100 6
17 APR 0900	85	83	34169	0	101 1	*	19 APR 1100	185	11	31711	3	100 6	*	21 APR 1300	285	3.	31238 .7	100 6
17 APR 0930	86	80	34111	6	101 1	*	19 APR 1130	186	10	31702	7	100 6	*	21 APR 1330	286	3	31236 2	100 6
17 APR 1000	87	78	34055	4	101 1	*	19 APR 1200	187	10	31694	3	100 6	*	21 APR 1400	287	3	31233 .8	100 6
17 APR 1030	88	76	34000	4	101 1	*	19 APR 1230	188	10	31685	.9	100 6	*	21 APR 1430	288	3.	31231 3	100 6
17 APR 1100	89	74	33946	4	101 0	*	19 APR 1300	189	10	31677	8	100 6	*	21 APR 1500	289	3	31228 9	100 6
17 APR 1130	90	72	33893	6	101 0	*	19 APR 1330	190	10	31669	7	100 6	*	21 APR 1530	290	3	31226 5	100 6
17 APR 1200	91	71	33841	8	101 0	*	19 APR 1400	191	10	31661	7	100 6	*	21 APR 1600	291	3.	31224 .2	100 6
17 APR 1230	92	69	33791	0	101 0	*	19 APR 1430	192	10	31653	9	100 6	*	21 APR 1630	292	3	31221 8	100 6
17 APR 1300	93	67	33741	4	101 0	*	19 APR 1500	193	9	31646	2	100 6	*	21 APR 1700	293	3	31219 5	100 6
17 APR 1330	94	65	33692	8	101 0	*	19 APR 1530	194	9	31638	.6	100 6	*	21 APR 1730	294	3	31217 .2	100 6
17 APR 1400	95	64	33645	3	101 0	*	19 APR 1600	195	9	31631	1	100 6	*	21 APR 1800	295	3	31214 9	100 6
17 APR 1430	96	62	33598	8	101 0	*	19 APR 1630	196	9	31623	7	100 6	*	21 APR 1830	296	3	31212 7	100 6
17 APR 1500	97	60	33553	2	101 0	*	19 APR 1700	197	9	31616	.4	100 6	*	21 APR 1900	297	3	31210 4	100 6
17 APR 1530	98	59	33508	7	101 0	*	19 APR 1730	198	9	31609	3	100 6	*	21 APR 1930	298	3	31208 2	100 6
17 APR 1600	99	57	33465	2	101 0	*	19 APR 1800	199	9	31602	2	100 6	*	21 APR 2000	299	3	31206 0	100 6
17 APR 1630	100	56	33422	6	101 0	*	19 APR 1830	200	8	31595	2	100 6	*	21 APR 2030	300	3	31203 9	100 6

PEAK FLOW	TIME	6-HR	MAXIMUM AVERAGE FLOW	149 50-HR
(CU M/S)	(HR)		24-HR	72-HR
160	26 00	157	128	65
		16 484	53 688	82 521
		3391	11044	16975
				34
				88 874
				18281

PEAK STORAGE	TIME	6-HR	MAXIMUM AVERAGE STORAGE	149 50-HR
(1000 CU M)	(HR)		24-HR	72-HR
35991	26 00	35931	35263	33521
				32297

PEAK STAGE	TIME	6-HR	MAXIMUM AVERAGE STAGE	149 50-HR
(METERS)	(HR)		24-HR	72-HR
101 42	26 00	101 41	101 29	100 97
				100 75

CUMULATIVE AREA = 205 70 SQ KM

RUNOFF SUMMARY, AVERAGE FLOW IN CUBIC METERS PER SECOND
AREA IN SQUARE KILOMETERS

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD	BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR	
HYDROGRAPH AT		75 89	14 50	48 40	18 06	6 18	15.83
HYDROGRAPH AT		76 34	16 00	57 78	24 42	8 58	22 10
HYDROGRAPH AT		39 97	17 00	36 69	20 11	7 47	19 42
HYDROGRAPH AT		46 89	16 00	35 84	15 30	5 39	13 87
4 COMBINED AT	BACIA	218 51	15 50	174 78	77 83	27 61	71 22
HYDROGRAPH AT		9 98	15 50	7 92	3 36	1 18	3 04
HYDROGRAPH AT		28 18	15 50	21 50	8 86	3 09	7.96
2 COMBINED AT	BACIA	246 69	15 50	196 02	86 67	30 70	79 18
HYDROGRAPH AT		273 94	16 00	216 96	95 56	33 83	87 51
HYDROGRAPH AT		36 64	20 50	33 86	19 62	7 45	19 41
HYDROGRAPH AT		72 44	16 50	62 80	35 99	13 40	34.89
HYDROGRAPH AT		35 37	15 50	30 41	14 00	5 00	12 89
2 COMBINED AT	BACIA	106 68	16 00	92 44	49 94	18 39	47 78
HYDROGRAPH AT		121 47	16 50	104 44	55 56	20 40	53 06
HYDROGRAPH AT		64 48	14 50	45 08	16 96	5 82	14 94
HYDROGRAPH AT		99 20	14 50	67 56	25.49	8 74	22 46
3 COMBINED AT	BACIA	476 58	15 50	381 46	176.43	62 97	163 03
HYDROGRAPH AT							

000098

Simulação Modelo TR 1.000

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.....
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* SEPTEMBER 1990
* VERSION 4 D
*
* RUN DATE 05/05/1996 TIME 10 10 55
*
.....

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.....
* U S ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
.....

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X X XXXXXX XXXXX X
X X X X X XXX
X X X X XXXX X
XXXXXXXX XXXX X XXXXX X
X X X X X X
X X X X X X
X X XXXXXX XXXXX XXX

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW

THE DEFINITIONS OF VARIABLES -RIIMP- AND -RIIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE THE DEFINITION OF -ANSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 20 SEP 81 THIS IS THE FORTRAN77 VERSION NEW OPTIONS DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DBS WRITE STAGE FREQUENCY, DBS READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE NEW FINITE DIFFERENCE ALGORITHM

1 HEC-1 INPUT PAGE 1

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	BACIA HIDROGRAFICA DO RESERVATORIO DE SOUSA									
2	ID	MODELO DE ONDA CINEMATICA DA BACIA TR-1000									
3	IT	30	15APR96	1500	300						
4	IO	30									
5	IM										
		*DIAGRAM									
6	KK	BACIA ABANICO COMPONENTE E									
7	KM	ESC SUPERFICIAL SUBBACIA E									
8	KO	3									
9	BA	15 83									
10	PH	0 1	220 0	20 8	45 0	81 8	107 0	120 0	160 0	178 0	209 8
11	LB		74			74					
12	UK	810	05	40	40.6						
13	UK	1170	05	40	59.4						
14	RK	8000	006	045		TRAP	0	40			
15	KK	BACIA ABANICO COMPONENTE H									
16	KM	ESC SUPERFICIAL SUBBACIA H									
17	KO	3									
18	BA	22 1									
19	UK	1130	016	40	46.1						
20	UK	1320	016	40	53.9						
21	RK	9000	008	045		TRAP	0	40			
22	KK	BACIA ABANICO COMPONENTE I									
23	KM	ESC SUPERFICIAL SUBBACIA I									
24	KO	3									
25	BA	19 42									
26	UK	1700	016	40	35.0						
27	UK	3150	016	40	65.0						
28	RK	4000	0040	045		TRAP	0	40			
29	KK	BACIA ABANICO COMPONENTE J									
30	KM	ESC SUPERFICIAL SUBBACIA J									
31	KO	3									
32	BA	13 87									
33	UK	1210	013	40	52.3						
34	UK	1100	013	40	47.7						
35	RK	6000	0030	045		TRAP	0	40			
36	KK	BACIA EM ABANICO COMPLETA									
37	HC	4									
38	KK	BACIA F-G COMPONENTE F									
39	KM	ESC SUPERFICIAL SUBBACIA F									
40	KO	3									
41	BA	3 04									
42	UK	1250	010	40	61.8						
43	UK	780	010	40	30.2						
44	RK	1500	0080	045		TRAP	0	40			

1 HEC-1 INPUT PAGE 2

LINE	ID	1	2	3	4	5	6	7	8	9	10
45	KK	BACIA F - G COMPONENTE G									
46	KM	ESC SUPERFICIAL SUBBACIA G									
47	KO	3									
48	BA	4 92									
49	UK	980	010	40	59.9						
50	UK	660	010	40	40.1						
51	RK	3000	0080	045		TRAP	0	40	YES		
52	KK	BACIA EM ABANICO COMPLETA + BACIA F - G									
53	HC	2									
54	KK	BACIA P									
55	KM	ESC SUPERFICIAL BACIA P									
56	KO	3									
57	BA	8 33									
58	UK	820	013	40	49.4						
59	UK	840	013	40	50.6						
60	RK	5000	0030	045		TRAP	0	40	YES		

000100

61	KK	BACIA K-L COMPONENTE K										
62	KM	ESC SUPERFICIAL SUBBACIA K										
63	KO	3										
64	BA	19 41										
65	UK	1340	003	40	48 8							
66	UK	1400	003	40	51 2							
67	RR	7100	0080	045		TRAP	0	40				
68	KK	BACIA K - L COMPONENTE L										
69	KM	ESC SUPERFICIAL SUBBACIA L										
70	KO	3										
71	BA	15 48										
72	UK	440	020	40	24 3							
73	UK	1380	020	40	75 7							
74	RR	8500	0030	045		TRAP	0	40	YES			
75	KK	BACIA M										
76	KM	ESC SUPERFICIAL BACIA M										
77	KO	3										
78	BA	12 89										
79	UK	750	010	40	31 9							
80	UK	1590	010	40	68 1							
81	RR	5500	0080	045		TRAP	0	40				
82	KK	BACIA M + BACIA K - L										
83	HC	2										
84	KK	BACIA Q										
85	KM	ESC SUPERFICIAL SUBBACIA Q										
86	KO	3										
87	BA	5 28										
88	UK	750	013	40	42.4							
89	UK	1010	013	40	57.6							
90	RR	3000	0030	045		TRAP	0	40	YES			

PAGE 3

LINE	ID	1	2	3	4	5	6	7	8	9	10
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91	KK	BACIA D-C COMPONENTE D										
92	KM	ESC SUPERFICIAL SUBBACIA D										
93	KO	3										
94	BA	14 94										
95	UK	1860	080	40	74 8							
96	UK	630	080	40	25 2							
97	RR	6000	0080	045		TRAP	0	40				
98	KK	BACIA D - C COMPONENTE C										
99	KM	ESC SUPERFICIAL SUBBACIA C										
100	KO	3										
101	BA	7 52										
102	UK	870	030	40	24 3							
103	UK	800	030	40	75 7							
104	RR	4500	0060	045		TRAP	0	40	YES			
105	KK	BACIA Q + BACIA P + BACIA D-C										
106	HC	3										
107	KK	BACIA R										
108	KM	ESC SUPERFICIAL SUBBACIA R										
109	KO	3										
110	BA	3 67										
111	UK	1160	013	40	63.4							
112	UK	670	013	40	36 6							
113	RR	2000	0030	045		TRAP	0	40	YES			
114	KK	BACIA B-A COMPONENTE B										
115	KM	ESC SUPERFICIAL SUBBACIA B										
116	KO	3										
117	BA	6 44										
118	UK	490	050	40	30 5							
119	UK	1120	050	40	69 5							
120	RR	4800	0100	045		TRAP	0	40				
121	KK	BACIA B - A COMPONENTE A										
122	KM	ESC SUPERFICIAL SUBBACIA A										
123	KO	3										
124	BA	8 85										
125	UK	910	030	40	51 5							
126	UK	860	030	40	48 5							
127	RR	5000	0080	045		TRAP	0	40	YES			
128	KK	BACIA N										
129	KM	ESC SUPERFICIAL SUBBACIA N										
130	KO	3										
131	BA	19 60										
132	UK	780	013	40	37 9							
133	UK	1280	013	40	62 1							
134	RR	9500	0030	045		TRAP	0	40				

PAGE 4

LINE	ID	1	2	3	4	5	6	7	8	9	10
------	----	---	---	---	---	---	---	---	---	---	----

135	KK	BACIA R + BACIA N + BACIA A-B										
136	HC	3										
137	KK	BACIA S										
138	KM	ESC SUPERFICIAL SUBBACIA S										
139	KO	1										
140	BA	4 11										
141	UK	450	010	40	32.6							
142	UK	920	010	40	67 3							
143	RR	3000	0030	045		TRAP	0	40	YES			
144	KK	BARRAGEM DE SOUSA										
145	KO	1										
146	RS	1	STOR	30000	-1							
147	SV		18043	22516	28092 3	33668 4	39244 4					
148	SE	97	98	99	100	101	102					
149	SS	100 5	130	1 4	1.5							
150	ZZ											

000101

1

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT
LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW
NO () CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW

```

6
15
22
29
36 BACIA
38
45      V
      V
      ***
52 BACIA
54      V
      V
      ***
61
68      V
      V
      ***
75
82      BACIA
      V
      V
      ***
84
91
98      V
      V
      ***
105 BACIA
107      V
      V
      ***
114
121      V
      V
      ***
128
135 BACIA
137      V
      V
      ***
144 BARR
  
```

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* SEPTEMBER 1990 *
* VERSION 4 0 *
* RUN DATE 05/05/1996 TIME 18 10 55 *
*****
  
```

```

*****
* U S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****
  
```

BACIA HIDROGRAFICA DO RESERVATORIO DE SOUSA
MODELO DE ONDA CINEMATICA DA BACIA TR-1000

```

4 10 OUTPUT CONTROL VARIABLES
      IPRT 30 PRINT CONTROL
      IPLOT 0 PLOT CONTROL
      QSCAL 0 HYDROGRAPH PLOT SCALE

11 HYDROGRAPH TIME DATA
      MMIN 30 MINUTES IN COMPUTATION INTERVAL
      IDATE 15APR96 STARTING DATE
      ITIME 1500 STARTING TIME
      NQ 300 NUMBER OF HYDROGRAPH ORDINATES
      NDDATE 21APR96 ENDING DATE
      NDTIME 2030 ENDING TIME
      ICENT 19 CENTURY MARK

      COMPUTATION INTERVAL 50 HOURS
      TOTAL TIME BASE 149 50 HOURS
  
```

000102

METRIC UNITS
 DRAINAGE AREA SQUARE KILOMETERS
 PRECIPITATION DEPTH MILLIMETERS
 LENGTH, ELEVATION METERS
 FLOW CUBIC METERS PER SECOND
 STORAGE VOLUME CUBIC METERS
 SURFACE AREA SQUARE METERS
 TEMPERATURE DEGREES CELSIUS

8 KO OUTPUT CONTROL VARIABLES
 IPRNT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

9 BA SUBBASIN CHARACTERISTICS
 TAREA 15 83 SUBBASIN AREA

PRECIPITATION DATA

10 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
 HYDRO-35 TP-40 TP-49
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
 20 80 45 00 81 80 107 00 120 00 160 00 178 00 209 80 00 00 00 00

STORM AREA - 220 00

11 LS SCS LOSS RATE
 STRL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STRL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

12 UK KINEMATIC WAVE
 OVERLAND-FLOW ELEMENT NO 1
 L 810 OVERLAND FLOW LENGTH
 S 0500 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 40.6 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

13 UK OVERLAND-FLOW ELEMENT NO 2
 L 1170 OVERLAND FLOW LENGTH
 S 0500 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 59.4 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

14 RK KINEMATIC WAVE
 MAIN CHANNEL
 L 8000 CHANNEL LENGTH
 S 0060 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 15 83 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 DXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ NO ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	56	1 67	17.51	162 05	45 87	790.53	114 21	16
PLANE2	56	1 67	23 15	234 08	55 73	816.27	114.00	17
MAIN	32	1 33	24.02	2667 54	89 31	849 81	113 96	1 85

CONTINUITY SUMMARY (1000 CU-M)--INFLOW- 0000E+00 EXCESS- 1812E+04 OUTFLOW- 1798E+04 BASIN STORAGE- 1149E+01 PERCENT ERROR- 7

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 32 1 33 30 00 87 99 870 00 113 97

HYDROGRAPH AT STATION

TOTAL RAINFALL - 191 61, TOTAL LOSS - 76 81, TOTAL EXCESS - 114 80

PEAK FLOW (CU M/S)	TIME (HR)	6-HR (CU M/S)	24-HR (MM)	72-HR (1000 CU M)	149 50-HR (1000 CU M)
88	14 50	55	75 653	111 100	113 908
			1198	1759	1800

CUMULATIVE AREA - 15 83 SQ KM

17 KO OUTPUT CONTROL VARIABLES
 IPRNT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

18 BA SUBBASIN CHARACTERISTICS
 TAREA 22 10 SUBBASIN AREA

000103

PRECIPITATION DATA

10 PH HYDRO-35 DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM TP-40 TP-49
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
 20 80 45 00 81 80 107 00 120 00 160.00 178 00 209 80 00 00 00 00

STORM AREA - 220 00

11 LS SCS LOSS RATE
 STRIL 17 85 INITIAL ABSTRACTION
 CRVBR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA
 LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STRIL 17 85 INITIAL ABSTRACTION
 CRVBR 74.00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

19 UK KINEMATIC WAVE
 OVERLAND-FLOW ELEMENT NO. 1
 L 1130 OVERLAND FLOW LENGTH
 S 0160 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 46 1 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS
 20 UK OVERLAND-FLOW ELEMENT NO. 2
 L 1320 OVERLAND FLOW LENGTH
 S 0160 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 53.9 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

21 RK KINEMATIC WAVE
 MAIN CHANNEL
 L 9000 CHANNEL LENGTH
 S 0080 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 22 10 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 DXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ NO ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	32	1 67	29.82	188.40	47 90	899.94	113.79	.11
PLANE2	32	1 67	29 48	188.63	50 67	900.14	113.83	.11
MAIN	37	1 33	24 08	3000 98	91.64	934 85	113.75	2 08

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 0000E+00 EXCESS- 2530E+04 OUTFLOW- 2506E+04 BASIN STORAGE- 4987E+01 PERCENT ERROR- 8

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 37 1 33 30 00 90 98 930 00 113.75

HYDROGRAPH AT STATION

TOTAL RAINFALL - 191 61. TOTAL LOSS - 76 81. TOTAL EXCESS - 114 80

PEAK FLOW (CU M/S)	TIME (HR)	6-HR (CU M/S)	24-HR (MM)	72-HR (1000 CU M)	149 50-HR (MM)
91	15 50	67	65 764	1453.	5
		28	107 887	2384	113 686
		10	113 148	2501	2512

CUMULATIVE AREA - 22 10 SQ KM

24 KO OUTPUT CONTROL VARIABLES
 IPRNT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

25 BA SUBBASIN CHARACTERISTICS
 TAREA 19 42 SUBBASIN AREA

PRECIPITATION DATA

10 PH HYDRO-35 DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM TP-40 TP-49
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
 20 80 45 00 81 80 107 00 120.00 160.00 178 00 209 80 00 00 00 00

STORM AREA - 220 00

11 LS SCS LOSS RATE
 STRIL 17 85 INITIAL ABSTRACTION
 CRVBR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA
 LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STRIL 17 85 INITIAL ABSTRACTION
 CRVBR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

000104

26 UK KINEMATIC WAVE
OVERLAND-FLOW ELEMENT NO. 1
L 1700 OVERLAND FLOW LENGTH
S 0160 SLOPE
N 400 ROUGHNESS COEFFICIENT
PA 35 0 PERCENT OF SUBBASIN
DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

27 UK OVERLAND-FLOW ELEMENT NO. 2
L 3150 OVERLAND FLOW LENGTH
S 0160 SLOPE
N 400 ROUGHNESS COEFFICIENT
PA 65 0 PERCENT OF SUBBASIN
DXMIN 4 MINIMUM NUMBER OF DX INTERVALS

28 RK KINEMATIC WAVE
MAIN CHANNEL
L 4000 CHANNEL LENGTH
S 0040 SLOPE
N 045 CHANNEL ROUGHNESS COEFFICIENT
CA 19 42 CONTRIBUTING AREA
SHAPE TRAP CHANNEL SHAPE
WD 00 BOTTOM WIDTH OR DIAMETER
Z 40 00 SIDE SLOPE
NDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
RUPSTQ NO ROUTE UPSTREAM HYDROGRAPH

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	32	1 67	30 00	212 57	24 07	957.71	113.85	11
PLANE2	32	1 67	30 00	262 59	28 95	1169.77	113.50	12
MAIN	26	1.33	16 31	1333.77	47 85	992 78	113 60	1 37

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 0000E+00 EXCESS- 2223E+04 OUTFLOW- 2199E+04 BASIN STORAGE- 1148E+02 PERCENT ERROR- 6

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 26 1 33 30 00 47 77 990 00 113 58

HYDROGRAPH AT STATION

TOTAL RAINFALL - 191 61, TOTAL LOSS - 76.81, TOTAL EXCESS - 114 80

PEAK FLOW (CU M/S)	TIME (HR)	6-HR (CU M/S)	24-HR (MM)	72-HR (1000 CU M)	MAXIMUM AVERAGE FLOW (MM)
48	16 50	43	40 217	936	113 525
		23	101 623	1974	2205
		8.	112 196	2179	
		4	113 525	2205	

CUMULATIVE AREA - 19 42 SQ KM

31 KO OUTPUT CONTROL VARIABLES
IPRNT 3 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

32 BA SUBBASIN CHARACTERISTICS
TAREA 13 87 SUBBASIN AREA

PRECIPITATION DATA

10 PH DEPTH FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35			TP-40				TP-49				
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
20 80	45 00	81 80	107 00	120 00	160 00	178 00	209 80	.00	00	00	00

STORM AREA - 220 00

11 LS SCS LOSS RATE
STRFL 17 85 INITIAL ABSTRACTION
CRVMBR 74 00 CURVE NUMBER
RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
STRFL 17 85 INITIAL ABSTRACTION
CRVMBR 74 00 CURVE NUMBER
RTIMP 00 PERCENT IMPERVIOUS AREA

33 UK KINEMATIC WAVE
OVERLAND-FLOW ELEMENT NO. 1
L 1210 OVERLAND FLOW LENGTH
S .0130 SLOPE
N 400 ROUGHNESS COEFFICIENT
PA 52 3 PERCENT OF SUBBASIN
DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

34 UK OVERLAND-FLOW ELEMENT NO. 2
L 1180 OVERLAND FLOW LENGTH
S 0130 SLOPE
N 400 ROUGHNESS COEFFICIENT
PA 47 7 PERCENT OF SUBBASIN
DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

35 RK KINEMATIC WAVE
MAIN CHANNEL
L 6000 CHANNEL LENGTH
S 0030 SLOPE
N .045 CHANNEL ROUGHNESS COEFFICIENT

000105

CA 13 87 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 NDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ NO ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	29	1 67	29 84	172 91	30.47	903 59	113 83	10
PLANE2	29	1 67	27 41	157 19	29.56	912 30	113 95	10
MAIN	22	1 33	26 27	2000 66	55.23	960 96	113 83	1 27

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 0000E+00 EXCESS- 1580E+04 OUTFLOW- 1574E+04 BASIN STORAGE- 3103E+01 PERCENT ERROR- .7

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 22 1 33 30 00 55 22 960 00 113.84

HYDROGRAPH AT STATION

TOTAL RAINFALL - 191 61, TOTAL LOSS - 76 81, TOTAL EXCESS - 114 80

PEAK FLOW (CU M/S)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	149 50-HR
55	16 00	41 (MM) 64 500 (1000 CU M)	17 107.565 1492	6. 113 211 1570	3 113 777 1570

CUMULATIVE AREA - 13 87 SQ KM

40 RO OUTPUT CONTROL VARIABLES

IPRMT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

41 BA SUBBASIN CHARACTERISTICS

TAREA 3 04 SUBBASIN AREA

PRECIPITATION DATA

10 PH

HYDRO-35 DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM TP-49
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
 20 80 45 00 81 80 107 00 120 00 160 00 178 00 209.80 00 00 .00 00
 STORM AREA - 220 00

11 LS

SCS LOSS RATE

STRIL 17 85 INITIAL ABSTRACTION
 CRVNB 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT

STRIL 17 85 INITIAL ABSTRACTION
 CRVNB 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

KINEMATIC WAVE

42 UR

OVERLAND-FLOW ELEMENT NO 1
 L 1250 OVERLAND FLOW LENGTH
 S 0100 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 61 8 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

43 UR

OVERLAND-FLOW ELEMENT NO 2
 L 780 OVERLAND FLOW LENGTH
 S 0100 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 38 2 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

44 RK

KINEMATIC WAVE

MAIN CHANNEL
 L 1500 CHANNEL LENGTH
 S 0080 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 3 04 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 NDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ NO ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	25	1 67	26 91	138.93	7 03	941 24	113 93	.09
PLANE2	25	1 67	26 47	130.04	5.81	874 39	113.98	08
MAIN	37	1 33	6 66	500 16	12 01	898 86	113 98	1 26

000106

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 37 1 33 30 00 12 01 900 00 114 30

*** **

HYDROGRAPH AT STATION

TOTAL RAINFALL - 191 61, TOTAL LOSS - 76 81, TOTAL EXCESS - 114 80

PEAK FLOW (CU M/S)	TIME (HR)	6-HR (CU M/S)	24-HR (1000 CU M)	72-HR (1000 CU M)	149 50-HR (1000 CU M)
12	15 00	9	4	1	1
		65 610	108 360	113.681	114 241
		199	329	346	347

CUMULATIVE AREA - 3 04 SQ KM

47 RO OUTPUT CONTROL VARIABLES
 IPRMT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

48 BA SUBBASIN CHARACTERISTICS
 TAREA 4 92 SUBBASIN AREA

PRECIPITATION DATA

10 PH	HYDRO-35	DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM						TP-49	10-DAY		
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
20 80	45 00	81 80	107 00	120 00	168 00	178 00	209 80	00	00	00	00

STORM AREA - 220 00

11 LS SCS LOSS RATE
 STRIL 17 85 INITIAL ABSTRACTION
 CRVMBR 74 00 CURVE NUMBER
 RTIMP .00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECONW OVERLAND FLOW ELEMENT
 STRIL 17 85 INITIAL ABSTRACTION
 CRVMBR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

49 UK KINEMATIC WAVE
 OVERLAND-FLOW ELEMENT NO 1
 L 900 OVERLAND FLOW LENGTH
 S 0100 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 59.9 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

50 UK OVERLAND-FLOW ELEMENT NO 2
 L 660 OVERLAND FLOW LENGTH
 S 0100 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 40 1 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

51 RK KINEMATIC WAVE
 MAIN CHANNEL
 L 3000 CHANNEL LENGTH
 S 0080 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 4 92 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 NDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTO YES ROUTE UPSTREAM HYDROGRAPH

COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	N	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	25	1 67	27 80	140 05	13 00	915 72	113 88	08
PLANE2	25	1 67	23 22	110 04	10 63	846 53	114 04	.08
MAIN	37	1 33	10 66	1000 33	33.45	911 44	114 09	1.59

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 37 1 33 30 00 33 30 900 00 113 99

*** **

HYDROGRAPH AT STATION

TOTAL RAINFALL - 191 61, TOTAL LOSS - 76 81, TOTAL EXCESS - 114 80

PEAK FLOW (CU M/S)	TIME (HR)	6-HR	24-HR	72-HR	149 50-HR

090107

33 15 00 (CU M/S) 25 10 3 2
 (MM) 67 493 108 663 113 452 113 932
 (1000 CU M) 537 865 903 907

CUMULATIVE AREA - 7 96 SQ KM

56 KO OUTPUT CONTROL VARIABLES
 IPRNT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

57 BA SUBBASIN CHARACTERISTICS
 TAREA 8 33 SUBBASIN AREA

PRECIPITATION DATA

10 PH DEPIHS FOR 0-PERCENT HYPOTHETICAL STORM
 HYDRO-35 TP-40
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
 20 80 45 00 81 80 107 00 120 00 160.00 178 00 209 80 00 00 00 00
 STORM AREA - 220 00

11 LS SCS LOSS RATE
 STRIL 17 85 INITIAL ABSTRACTION
 CRVBR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STRIL 17 85 INITIAL ABSTRACTION
 CRVBR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

58 UK KINEMATIC WAVE OVERLAND-FLOW ELEMENT NO 1
 L 820 OVERLAND FLOW LENGTH
 S 0130 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 49.4 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

59 UK OVERLAND-FLOW ELEMENT NO 2
 L 840 OVERLAND FLOW LENGTH
 S 0130 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 50.6 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

60 RK KINEMATIC WAVE MAIN CHANNEL
 L 5000 CHANNEL LENGTH
 S 0030 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 8 33 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 HDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ YES ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	.29	1.67	24.84	136.71	21.34	860.36	114.03	.09
PLANE2	.29	1.67	25.31	140.05	21.64	864.52	113.92	.09
MAIN	.22	1.33	15.04	1667.21	324.52	936.19	113.46	1.86

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 8974E+04 EXCESS- 9536E+03 OUTFLOW- 9896E+04 BASIN STORAGE- 2473E+01 PERCENT ERROR- 3

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 22 1 33 30 00 323 56 930 00 113 45

HYDROGRAPH AT STATION

TOTAL RAINFALL - 191 61, TOTAL LOSS - 76 81, TOTAL EXCESS - 114 80

PEAK FLOW TIME MAXIMUM AVERAGE FLOW
 (CU M/S) (HR) 6-HR 24-HR 72-HR 149 50-HR
 324 15 50 (CU M/S) 251 100 38 18
 (MM) 61 927 106.658 112 741 113 987
 (1000 CU M) 5419 9334 9866 9922
 CUMULATIVE AREA - 87 51 SQ KM

63 KO OUTPUT CONTROL VARIABLES
 IPRNT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

64 BA SUBBASIN CHARACTERISTICS
 TAREA 19 41 SUBBASIN AREA

PRECIPITATION DATA

10 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
 HYDRO-35 TP-40 TP-49
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
 20 80 45 00 81 80 107 00 120 00 160 00 178 00 209 80 00 00 00 00

STORM AREA - 220 00

11 IS SCS LOSS RATE
 STRTL 17 85 INITIAL ABSTRACTION
 CRVWBR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STRTL 17 85 INITIAL ABSTRACTION
 CRVWBR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

65 UK KINEMATIC WAVE
 OVERLAND-FLOW ELEMENT NO. 1
 L 1340 OVERLAND FLOW LENGTH
 S 0030 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 48 8 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

66 UK OVERLAND-FLOW ELEMENT NO. 2
 L 1400 OVERLAND FLOW LENGTH
 S 0030 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 51 2 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

67 RK KINEMATIC WAVE
 MAIN CHANNEL
 L 7100 CHANNEL LENGTH
 S 0080 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 19 41 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 80 SIDE SLOPE
 DXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ NO ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	14	1 67	27 47	83.78	22 09	1164 08	113 69	05
PLANE2	14	1 67	26 86	82 38	22 52	1183 77	113.67	05
MAIN	37	1 33	22 77	2367 44	43 64	1199.67	113 64	1 73

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 0000E+00 EXCESS- 2222E+04 OUTFLOW- 2199E+04 BASIN STORAGE- 1484E+02 PERCENT ERROR- 4

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 37 1 33 30 00 43.64 1200 00 113 64

HYDROGRAPH AT STATION

TOTAL RAINFALL - 191 61. TOTAL LOSS - 76 81. TOTAL EXCESS - 114 80

PEAK FLOW (CU M/S)	TIME (HR)	6-HR (CU M/S)	24-HR (MM)	72-HR (1000 CU M)	149 50-HR (MM)
44	20.80	40	44 645	867	111 996
			22	8	4
			99 484	1931	2174
			2205		

CUMULATIVE AREA - 19 41 SQ KM

70 KO OUTPUT CONTROL VARIABLES
 IPRINT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

71 BA SUBBASIN CHARACTERISTICS
 IAREA 15 48 SUBBASIN AREA

PRECIPITATION DATA

10 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
 HYDRO-35 TP-40 TP-49
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
 20 80 45 00 81 80 107 00 120 00 160 00 178 00 209 80 00 00 00 00

STORM AREA - 220.00

11 IS SCS LOSS RATE
 STRTL 17.85 INITIAL ABSTRACTION
 CRVWBR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STRTL 17 85 INITIAL ABSTRACTION
 CRVWBR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

KINEMATIC WAVE

000109

72 UK OVERLAND-FLOW ELEMENT NO 1
 L 440 OVERLAND FLOW LENGTH
 S 0200 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 24 3 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

73 UK OVERLAND-FLOW ELEMENT NO 2
 L 1380 OVERLAND FLOW LENGTH
 S 0200 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 75 7 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

74 RK KINEMATIC WAVE
 MAIN CHANNEL
 L 8500 CHANNEL LENGTH
 S 0030 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 15 48 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 NDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ YES ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	35	1 67	15 36	88 03	28 60	789 97	114 28	10
PLANE2	35	1 67	30 00	230 08	51 13	918 25	113 83	12
MAIN	22	1 33	30 00	2834 26	87 33	966.72	113.20	1 39

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 2197E+04 EXCESS- 1772E+04 OUTFLOW- 3936E+04 BASIN STORAGE- 4308E+01 PERCENT ERROR- 7

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 22 1 33 30 00 86 87 960 00 113 20

HYDROGRAPH AT STATION

TOTAL RAINFALL - 191 61 TOTAL LOSS - 76 81 TOTAL EXCESS - 114 80

PEAK FLOW (CU M/S)	TIME (HR)	6-HR (CU M/S)	24-HR (MM)	72-HR (1000 CU M)	149 50-HR (MM)
87	16 00	75	46 346	1617	113 143
		41	101 343	3536	3948
		15	111 924	3905	3948

CUMULATIVE AREA - 34 89 SQ KM

77 KO OUTPUT CONTROL VARIABLES
 IPRINT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUMOFF DATA

78 BA SUBBASIN CHARACTERISTICS
 TAREA 12 89 SUBBASIN AREA

PRECIPITATION DATA

DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM											
HYDRO-35			TP-40			TP-49					
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
20 80	45 00	81 80	107 00	120 00	160 00	178 00	209 80	00	.00	00	.00

STORM AREA - 220 00

11 LS SCS LOSS RATE
 STRTL 17 85 INITIAL ABSTRACTION
 CRVWR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STRTL 17 85 INITIAL ABSTRACTION
 CRVWR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

79 UK KINEMATIC WAVE
 OVERLAND-FLOW ELEMENT NO 1
 L 750 OVERLAND FLOW LENGTH
 S 0100 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 31 9 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

80 UK OVERLAND-FLOW ELEMENT NO 2
 L 1590 OVERLAND FLOW LENGTH
 S 0100 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 68 1 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

81 RK KINEMATIC WAVE
 MAIN CHANNEL
 L 5500 CHANNEL LENGTH
 S 0080 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 12 89 CONTRIBUTING AREA

080110

SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 NDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ NO ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	25	1 67	25.66	125 04	20.95	867 56	113.91	08
PLANE2	25	1 67	30 00	159 05	27 52	1003.13	113 81	09
MAIN	37	1 33	17 86	1833 93	42 24	925 43	113 81	1 72

CONTINUITY SUMMARY (1000 CU-M)--INFLOW- 0000E+00 EXCESS- 1476E+04 OUTFLOW- 1462E+04 BASIN STORAGE- 4033E+01 PERCENT ERROR- 6

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 37 1 33 30 00 42 20 930.00 113 77

HYDROGRAPH AT STATION

TOTAL RAINFALL - 191 61. TOTAL LOSS - 76 81. TOTAL EXCESS - 114 80

PEAK FLOW (CU M/S)	TIME (HR)	6-HR (CU M/S)	24-HR (MM)	72-HR (1000 CU M)	149 50-HR (MM)
42	15 50	36	59 692	769	113 708
		16	106 128	1368	1456
		6	112 961	1456	1466
		3			

CUMULATIVE AREA - 12 89 SQ KM

86 KO

OUTPUT CONTROL VARIABLES

IPRINT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

87 BA

SUBBASIN CHARACTERISTICS

IAREA 5 28 SUBBASIN AREA

PRECIPITATION DATA

10 PH

HYDRO-35

DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
20 80	45 00	81 80	107 00	120 00	160 00	178 00	209 80	00	.00	00	00

STORM AREA - 220 00

11 LS

SCS LOSS RATE

STRTL 17 85 INITIAL ABSTRACTION
 CRVMBR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT

STRTL 17 85 INITIAL ABSTRACTION
 CRVMBR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

88 UK

KINEMATIC WAVE

OVERLAND-FLOW ELEMENT NO. 1

L 750 OVERLAND FLOW LENGTH
 S 0130 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 42 4 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

89 UK

OVERLAND-FLOW ELEMENT NO. 2

L 1010 OVERLAND FLOW LENGTH
 S 0130 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 57 6 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

90 RK

KINEMATIC WAVE

MAIN CHANNEL

L 3000 CHANNEL LENGTH
 S 0030 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 5.28 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 NDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ YES ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	29	1 67	27 87	150.05	12 03	849 40	113 83	09
PLANE2	29	1 67	25 49	144 33	14 42	895 06	113 96	09
MAIN	22	1 33	10 88	1000.33	146.40	956.65	113 23	1 53

000111

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 22 1 33 30 00 146 17 960 00 113 23

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HYDROGRAPH AT STATION

TOTAL RAINFALL - 191 61, TOTAL LOSS - 76 81, TOTAL EXCESS - 114 80

PEAK FLOW (CU M/S)	TIME (HR)	6-HR (CU M/S)	24-HR (MM)	72-HR (1000 CU M)	149.50-HR (MM)
146	16 00	123	63	23	11
		50 003	102 741	112 121	113 172
		2653	5451	5949	6005

CUMULATIVE AREA - 53 06 SQ KM

93 KO OUTPUT CONTROL VARIABLES
 IPRINT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUMOFF DATA

94 BA SUBBASIN CHARACTERISTICS
 TAREA 14 94 SUBBASIN AREA

PRECIPITATION DATA

10 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35			... TP-40				... TP-49				
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
20 80	45 00	81 80	107 00	120 00	160 00	178 00	209 80	00	00	00	00

STORM AREA - 220 00

11 LS SCS LOSS RATE
 STRIL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STRIL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

95 UK KINEMATIC WAVE
 OVERLAND-FLOW ELEMENT NO 1
 L 1860 OVERLAND FLOW LENGTH
 S 0800 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 74 8 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

96 UK KINEMATIC WAVE
 OVERLAND-FLOW ELEMENT NO 2
 L 630 OVERLAND FLOW LENGTH
 S 0800 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 25 2 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

97 RK KINEMATIC WAVE
 MAIN CHANNEL
 L 6000 CHANNEL LENGTH
 S 0080 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 14 94 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 MDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ NO ROUTE UPSTREAM HYDROGRAPH

COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	H	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	71	1.67	27 66	372 12	60 07	849 36	113 83	.22
PLANE2	71	1 67	11 92	126 04	32 84	768 79	114.43	18
MAIN	37	1 33	16 72	2000 66	76.52	845 60	113 96	1 99

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 37 1 33 30 00 76 10 840 00 114 05

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HYDROGRAPH AT STATION

TOTAL RAINFALL - 191 61, TOTAL LOSS - 76 81, TOTAL EXCESS - 114 80

PEAK FLOW (CU M/S)	TIME (HR)	6-HR (CU M/S)	24-HR (MM)	72-HR (1000 CU M)	149.50-HR (MM)

000112

76 14 00 52 19 7 3
 (MM) 74 841 110 815 113 726 113 986
 (1000 CU M) 1118 1656 1699 1703

CUMULATIVE AREA - 14 94 SQ KM

100 KO OUTPUT CONTROL VARIABLES
 IPRINT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

101 BA SUBBASIN CHARACTERISTICS
 TAREA 7 52 SUBBASIN AREA

PRECIPITATION DATA

10 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
 HYDRO-35 TP-40 TP-49
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
 20 00 45 00 81 00 107 00 120 00 160 00 178 00 209 00 00 00 00 00

STORM AREA - 220 00

11 LS SCS LOSS RATE
 STRIL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STRIL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

102 UK KINEMATIC WAVE
 OVERLAND-FLOW ELEMENT NO 1
 L 870 OVERLAND FLOW LENGTH
 S 0300 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 24 3 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

103 UK OVERLAND-FLOW ELEMENT NO 2
 L 800 OVERLAND FLOW LENGTH
 S 0300 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 75 7 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

104 RK KINEMATIC WAVE
 MAIN CHANNEL
 L 4500 CHANNEL LENGTH
 S 0060 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 7.52 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 NDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ YES ROUTE UPSTREAM HYDROGRAPH

COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	43	1 67	22 46	174 06	11.00	810 76	114 01	13
PLANE2	43	1 67	21 18	160.05	35.60	821.08	114.07	13
MAIN	32	1 33	13 13	1500.49	117 26	854 95	113.86	1 91

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 1697E+04 EXCESS- 8609E+03 OUTFLOW- 2549E+04 BASIN STORAGE- 6979E+00 PERCENT ERROR- 3

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 32 1 33 30 00 116.42 870 00 113 90

HYDROGRAPH AT STATION

TOTAL RAINFALL - 191 61, TOTAL LOSS - 76 81, TOTAL EXCESS - 114 80

PEAK FLOW TIME MAXIMUM AVERAGE FLOW
 (CU M/S) (HR) 6-HR 24-HR 72-HR 149 50-HR
 + 116 14 50 77 29 10 5.
 (MM) 74 355 110 697 113 593 113 844
 (1000 CU M) 1670 2486 2551 2557

CUMULATIVE AREA - 22 46 SQ KM

107 KO OUTPUT CONTROL VARIABLES
 IPRINT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

110 BA SUBBASIN CHARACTERISTICS
 TAREA 3 67 SUBBASIN AREA

PRECIPITATION DATA

10 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
 HYDRO-35 IP-40 IP-49
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
 20 80 45 00 81 80 107 00 120 00 160 00 178 00 209 80 00 00 00 00

STORM AREA - 220 00

11 LS SCS LOSS RATE
 STRTL 17 85 INITIAL ABSTRACTION
 CRVMBR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STRTL 17 85 INITIAL ABSTRACTION
 CRVMBR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

111 UK KINEMATIC WAVE
 OVERLAND-FLOW ELEMENT NO 1
 L 1160 OVERLAND FLOW LENGTH
 S 0130 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 63 4 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

112 UK OVERLAND-FLOW ELEMENT NO 2
 L 670 OVERLAND FLOW LENGTH
 S 0130 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 36 6 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

113 RK KINEMATIC WAVE
 MAIN CHANNEL
 L 2000 CHANNEL LENGTH
 S 0030 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 3 67 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40.00 SIDE SLOPE
 NDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ YES ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	29	1 67	28.74	165 77	10 00	895 16	113 88	10
PLANE2	29	1 67	25 35	134 04	7.57	832 05	113 94	09
MAIN	22	1 33	5.20	666 89	578 99	940 69	113 32	2 14

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 1842E+05 EXCESS- 4201E+03 OUTFLOW- 1883E+05 BASIN STORAGE- 1600E+01 PERCENT ERROR- .1

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 22 1 33 30 00 573 90 930.00 113.30

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HYDROGRAPH AT STATION

TOTAL RAINFALL - 191 61, TOTAL LOSS - 76 81, TOTAL EXCESS - 114 80

PEAK FLOW (CU M/S)	TIME (HR)	6-HR (CU M/S)	24-HR (MM)	72-HR (1000 CU M)	149.50-HR (MM)
574	15 50	453	58 648	17616	18877
			105 676	112.512	113.239

CUMULATIVE AREA - 166 70 SQ KM

116 KO OUTPUT CONTROL VARIABLES
 IPRNT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

117 BA SUBBASIN CHARACTERISTICS
 TAREA 6 44 SUBBASIN AREA

PRECIPITATION DATA

10 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
 HYDRO-35 IP-40 IP-49
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
 20 80 45 00 81 80 107 00 120 00 160 00 178 00 209 80 00 00 00 00

STORM AREA - 220.00

11 LS SCS LOSS RATE
 STRTL 17 85 INITIAL ABSTRACTION
 CRVMBR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STRTL 17 85 INITIAL ABSTRACTION
 CRVMBR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

118 UK KINEMATIC WAVE
 OVERLAND-FLOW ELEMENT NO 1

000114

119 UK OVERLAND-FLOW ELEMENT NO 2
 L 490 OVERLAND FLOW LENGTH
 S 0500 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 30 5 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

120 RK KINEMATIC WAVE MAIN CHANNEL
 L 4000 CHANNEL LENGTH
 S 0100 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 6 44 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 NDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ NO ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	56	1 67	11 79	98.03	17.21	767 23	114.45	14
PLANE2	56	1 67	22 41	224.07	26.98	810.39	114 02	17
MAIN	41	1 33	12 18	1333.77	39 22	822.55	114 16	1 83

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 0000E+00 EXCESS- 7373E+03 OUTFLOW- 7328E+03 BASIN STORAGE- 4450E+00 PERCENT ERROR- .5

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 41 1 33 30 00 38 94 810 00 114 15

HYDROGRAPH AT STATION

TOTAL RAINFALL - 191 61. TOTAL LOSS - 76 81. TOTAL EXCESS - 114 80

PEAK FLOW (CU M/S)	TIME (HR)	6-HR (CU M/S)	24-HR MAXIMUM AVERAGE FLOW	72-HR	149 50-HR
39	13 50	23	8	3	1
		(MM) 78 737	111 822	113 918	114 093
		(1000 CU M) 507	720	734	735

CUMULATIVE AREA - 6 44 SQ KM

123 KO OUTPUT CONTROL VARIABLES
 IPRINT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QBCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

124 BA SUBBASIN CHARACTERISTICS
 TAREA 8 85 SUBBASIN AREA

PRECIPITATION DATA

10 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
 HYDRO-35 TP-40 TP-49
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
 20.80 45 00 81 80 107 00 120 00 160.00 178 00 209.80 00 00 80 .00
 STORM AREA - 220 00

11 LS SCS LOSS RATE
 STRIL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STRIL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

125 UK KINEMATIC WAVE OVERLAND-FLOW ELEMENT NO 1
 L 910 OVERLAND FLOW LENGTH
 S 0300 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 51.5 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

126 UK OVERLAND-FLOW ELEMENT NO 2
 L 860 OVERLAND FLOW LENGTH
 S 0300 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 48 5 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

127 RK KINEMATIC WAVE MAIN CHANNEL
 L 5000 CHANNEL LENGTH
 S 0080 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 8 85 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE

WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 NDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ YES ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	43	1 67	23 22	182 06	26 97	816.82	114 03	.13
PLANE2	43	1 67	22 27	172 06	25 96	809 19	114 04	.13
MAIN	37	1 33	13 76	1667 21	89 51	853.45	114 19	2.02

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 7322E+03 EXCESS- .1013E+04 OUTFLOW- 1740E+04 BASIN STORAGE- 7830E+00 PERCENT ERROR- 2

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 37 1 33 30 00 89.30 840 00 114 34

HYDROGRAPH AT STATION

TOTAL RAINFALL - 191 61, TOTAL LOSS - 76 81, TOTAL EXCESS - 114 80

PEAR FLOW + (CU M/S)	TIME (HR)	6-HR (CU M/S)	MAXIMUM 24-HR	AVERAGE 72-HR	149 50-HR
+ 89	14 00	55	20	7	3
		(MM) 77 226	111 564	114 063	114 276
		(1000 CU M) 1181	1706	1744	1747

CUMULATIVE AREA - 15 29 SQ KM

130 RO OUTPUT CONTROL VARIABLES
 IPRINT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

131 BA SUBBASIN CHARACTERISTICS
 TAREA 19 60 SUBBASIN AREA

PRECIPITATION DATA

10 PH DEPTH FOR 0-PERCENT HYPOTHETICAL STORM
 HYDRO-35 TP-40
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
 20 80 45 00 81 80 107 00 120 00 160.00 178 00 209 80 00 00 00 00 00

STORM AREA - 220 00

11 LS SCS LOSS RATE
 STRL 17.85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STRL 17.85 INITIAL ABSTRACTION
 CRVNR 74.00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

KINEMATIC WAVE

137 UK OVERLAND-FLOW ELEMENT NO 1
 L 780 OVERLAND FLOW LENGTH
 S 0130 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 37 9 PERCENT OF SUBBASIN
 NDXMIN 5 MINIMUM NUMBER OF DX INTERVALS

133 UK OVERLAND-FLOW ELEMENT NO 2
 L 1200 OVERLAND FLOW LENGTH
 S 0130 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 62 1 PERCENT OF SUBBASIN
 NDXMIN 5 MINIMUM NUMBER OF DX INTERVALS

KINEMATIC WAVE

134 RK MAIN CHANNEL
 L 9500 CHANNEL LENGTH
 S .0030 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 19 60 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 NDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ NO ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	29	1 67	28 54	156 05	39 21	855.76	113.89	09
PLANE2	29	1 67	27 57	160.05	49 18	916.17	113 91	10
MAIN	22	1 33	29 38	2375 78	75.95	948.44	113 76	1.35

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 22 1 33 30 00 75 66 960.00 113 69

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HYDROGRAPH AT STATION

TOTAL RAINFALL - 191 61. TOTAL LOSS - 76 81. TOTAL EXCESS - 114 80

PEAK FLOW (CU M/S)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	149 50-HR
76	16 00	58	24	9	4
		63 619	107 344	113.069	113 626
		1247	2104	2216	2227

CUMULATIVE AREA - 19 60 SQ KM

139 KO OUTPUT CONTROL VARIABLES
 IPRMI 1 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

140 BA SUBBASIN CHARACTERISTICS
 TAREA 4 11 SUBBASIN AREA

PRECIPITATION DATA

10 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35		TP-40						TP-49			
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
20 00	45 00	81 80	107 00	120 00	160 00	178 00	209 80	.00	.00	.00	.00

STORM AREA - 220 00

11 LS SCS LOSS RATE
 STRTL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STRTL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

141 UK KINEMATIC WAVE OVERLAND-FLOW ELEMENT NO 1
 L 450 OVERLAND FLOW LENGTH
 S 0100 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 32 6 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

142 UK OVERLAND-FLOW ELEMENT NO 2
 L 920 OVERLAND FLOW LENGTH
 S 0100 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 67 3 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

143 RK KINEMATIC WAVE MAIN CHANNEL
 L 3000 CHANNEL LENGTH
 S 0030 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 4 11 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 WDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ YES ROUTE UPSTREAM HYDROGRAPH

COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAR (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	25	1 67	20 68	90 03	8.50	817 54	114.08	07
PLANE2	25	1 67	26 27	131.47	12.84	902 53	113.99	08
MAIN	22	1 33	7 36	1000 33	726.51	945 94	113 25	2 27

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 22 1 33 30 00 719 64 960.00 113 18

HYDROGRAPH AT STATION

DA	MON	HRMN	ORD	RAIN	LOSS	EXCESS	COMP Q	DA	MON	HRMN	ORD	RAIN	LOSS	EXCESS	COMP Q
15	APR	1500	1	00	00	00	0	18	APR	1800	151	00	00	00	2
15	APR	1530	2	1 16	1 16	00	0	18	APR	1830	152	00	00	00	2.
15	APR	1600	3	1 20	1 20	00	0	18	APR	1900	153	00	00	00	2

15 APR 1630	4	1 74	1 24	00	0	*	18 APR 1930	154	00	00	00	2
15 APR 1700	5	1 28	1 28	00	0	*	18 APR 2000	155	00	00	00	2
15 APR 1730	6	1 33	1 33	00	0	*	18 APR 2030	156	00	00	00	2
15 APR 1800	7	1 38	1 38	00	0	*	18 APR 2100	157	00	00	00	2
15 APR 1830	8	1 43	1 43	00	0	*	18 APR 2130	158	00	00	00	2
15 APR 1900	9	1 49	1 49	00	0	*	18 APR 2200	159	00	00	00	2
15 APR 1930	10	1 56	1 56	00	0	*	18 APR 2230	160	00	00	00	2
15 APR 2000	11	1 63	1 63	00	0	*	18 APR 2300	161	00	00	00	2
15 APR 2030	12	1 72	1 72	00	0	*	18 APR 2330	162	00	00	00	2
15 APR 2100	13	1 81	1 81	00	0	*	19 APR 0000	163	00	00	00	2
15 APR 2130	14	1 52	1 51	01	0	*	19 APR 0030	164	00	00	00	1
15 APR 2200	15	1 63	1 57	06	0	*	19 APR 0100	165	00	00	00	1
15 APR 2230	16	1 75	1 63	13	0	*	19 APR 0130	166	00	00	00	1
15 APR 2300	17	1 91	1 70	20	0	*	19 APR 0200	167	00	00	00	1
15 APR 2330	18	2 09	1 79	30	0	*	19 APR 0230	168	00	00	00	1
16 APR 0000	19	2 33	1 91	42	0	*	19 APR 0300	169	00	00	00	1
16 APR 0030	20	5 70	4 31	1 39	0	*	19 APR 0330	170	00	00	00	1
16 APR 0100	21	6 27	4 24	2 04	0	*	19 APR 0400	171	00	00	00	1
16 APR 0130	22	7 06	4 23	2 83	0	*	19 APR 0430	172	00	00	00	1
16 APR 0200	23	6 81	3 63	3 18	1	*	19 APR 0500	173	00	00	00	1
16 APR 0230	24	11 97	5 51	6 45	2	*	19 APR 0530	174	00	00	00	1
16 APR 0300	25	20 33	7 46	12 88	6	*	19 APR 0600	175	00	00	00	1
16 APR 0330	26	33 89	8 93	24 97	20	*	19 APR 0630	176	00	00	00	1
16 APR 0400	27	13 90	2 81	11 08	75	*	19 APR 0700	177	00	00	00	1
16 APR 0430	28	7 57	1 40	6 27	209	*	19 APR 0730	178	00	00	00	1
16 APR 0500	29	7 59	1 29	6 30	394	*	19 APR 0800	179	00	00	00	1
16 APR 0530	30	6 63	1 06	5 58	558	*	19 APR 0830	180	00	00	00	1
16 APR 0600	31	5 97	90	5 07	667	*	19 APR 0900	181	00	00	00	1
16 APR 0630	32	2 47	36	2 11	716	*	19 APR 0930	182	00	00	00	1
16 APR 0700	33	2 20	31	1 89	720	*	19 APR 1000	183	00	00	00	1
16 APR 0730	34	2 00	28	1 72	692	*	19 APR 1030	184	00	00	00	1
16 APR 0800	35	1 83	25	1 58	646	*	19 APR 1100	185	00	00	00	1
16 APR 0830	36	1 69	23	1 46	591	*	19 APR 1130	186	00	00	00	1
16 APR 0900	37	1 57	21	1 36	535	*	19 APR 1200	187	00	00	00	1
16 APR 0930	38	1 86	25	1 62	482	*	19 APR 1230	188	00	00	00	1
16 APR 1000	39	1 76	23	1 53	435	*	19 APR 1300	189	00	00	00	1
16 APR 1030	40	1 67	21	1 46	395	*	19 APR 1330	190	00	00	00	1
16 APR 1100	41	1 59	20	1 39	362	*	19 APR 1400	191	00	00	00	1
16 APR 1130	42	1 52	19	1 33	335	*	19 APR 1430	192	00	00	00	1
16 APR 1200	43	1 46	18	1 28	313	*	19 APR 1500	193	00	00	00	1
16 APR 1230	44	1 40	17	1 23	294	*	19 APR 1530	194	00	00	00	1
16 APR 1300	45	1 35	16	1 19	277	*	19 APR 1600	195	00	00	00	1
16 APR 1330	46	1 30	16	1 15	261	*	19 APR 1630	196	00	00	00	1
16 APR 1400	47	1 26	15	1 11	247	*	19 APR 1700	197	00	00	00	1
16 APR 1430	48	1 22	14	1 08	233	*	19 APR 1730	198	00	00	00	1
16 APR 1500	49	1 18	14	1 04	220	*	19 APR 1800	199	00	00	00	1
16 APR 1530	50	00	00	00	208	*	19 APR 1830	200	00	00	00	1
16 APR 1600	51	00	00	00	196	*	19 APR 1900	201	00	00	00	1
16 APR 1630	52	00	00	00	184	*	19 APR 1930	202	00	00	00	1
16 APR 1700	53	00	00	00	172	*	19 APR 2000	203	00	00	00	1
16 APR 1730	54	00	00	00	160	*	19 APR 2030	204	00	00	00	1
16 APR 1800	55	00	00	00	148	*	19 APR 2100	205	00	00	00	1
16 APR 1830	56	00	00	00	137	*	19 APR 2130	206	00	00	00	1
16 APR 1900	57	00	00	00	126	*	19 APR 2200	207	00	00	00	1
16 APR 1930	58	00	00	00	116	*	19 APR 2230	208	00	00	00	1
16 APR 2000	59	00	00	00	106	*	19 APR 2300	209	00	00	00	1
16 APR 2030	60	00	00	00	98	*	19 APR 2330	210	00	00	00	1
16 APR 2100	61	00	00	00	90	*	20 APR 0000	211	00	00	00	1
16 APR 2130	62	00	00	00	83	*	20 APR 0030	212	00	00	00	1
16 APR 2200	63	00	00	00	76	*	20 APR 0100	213	00	00	00	1
16 APR 2230	64	00	00	00	71	*	20 APR 0130	214	00	00	00	1
16 APR 2300	65	00	00	00	65	*	20 APR 0200	215	00	00	00	1
16 APR 2330	66	00	00	00	61	*	20 APR 0230	216	00	00	00	1
17 APR 0000	67	00	00	00	56	*	20 APR 0300	217	00	00	00	1
17 APR 0030	68	00	00	00	52	*	20 APR 0330	218	00	00	00	1
17 APR 0100	69	00	00	00	49	*	20 APR 0400	219	00	00	00	1
17 APR 0130	70	00	00	00	45	*	20 APR 0430	220	00	00	00	1
17 APR 0200	71	00	00	00	42	*	20 APR 0500	221	00	00	00	1
17 APR 0230	72	00	00	00	40	*	20 APR 0530	222	00	00	00	1
17 APR 0300	73	00	00	00	37	*	20 APR 0600	223	00	00	00	1
17 APR 0330	74	00	00	00	35	*	20 APR 0630	224	00	00	00	1
17 APR 0400	75	00	00	00	33	*	20 APR 0700	225	00	00	00	1
17 APR 0430	76	00	00	00	31	*	20 APR 0730	226	00	00	00	1
17 APR 0500	77	00	00	00	29	*	20 APR 0800	227	00	00	00	1
17 APR 0530	78	00	00	00	27	*	20 APR 0830	228	00	00	00	1
17 APR 0600	79	00	00	00	26	*	20 APR 0900	229	00	00	00	1
17 APR 0630	80	00	00	00	24	*	20 APR 0930	230	00	00	00	1
17 APR 0700	81	00	00	00	23	*	20 APR 1000	231	00	00	00	0
17 APR 0730	82	00	00	00	22	*	20 APR 1030	232	00	00	00	0
17 APR 0800	83	00	00	00	21	*	20 APR 1100	233	00	00	00	0
17 APR 0830	84	00	00	00	19	*	20 APR 1130	234	00	00	00	0
17 APR 0900	85	00	00	00	19	*	20 APR 1200	235	00	00	00	0
17 APR 0930	86	00	00	00	18	*	20 APR 1230	236	00	00	00	0
17 APR 1000	87	00	00	00	17	*	20 APR 1300	237	00	00	00	0
17 APR 1030	88	00	00	00	16	*	20 APR 1330	238	00	00	00	0
17 APR 1100	89	00	00	00	15	*	20 APR 1400	239	00	00	00	0
17 APR 1130	90	00	00	00	14	*	20 APR 1430	240	00	00	00	0
17 APR 1200	91	00	00	00	14	*	20 APR 1500	241	00	00	00	0
17 APR 1230	92	00	00	00	13	*	20 APR 1530	242	00	00	00	0
17 APR 1300	93	00	00	00	13	*	20 APR 1600	243	00	00	00	0
17 APR 1330	94	00	00	00	12	*	20 APR 1630	244	00	00	00	0
17 APR 1400	95	00	00	00	11	*	20 APR 1700	245	00	00	00	0
17 APR 1430	96	00	00	00	11	*	20 APR 1730	246	00	00	00	0
17 APR 1500	97	00	00	00	11	*	20 APR 1800	247	00	00	00	0
17 APR 1530	98	00	00	00	10	*	20 APR 1830	248	00	00	00	0
17 APR 1600	99	00	00	00	10	*	20 APR 1900	249	00	00	00	0
17 APR 1630	100	00	00	00	9	*	20 APR 1930	250	00	00	00	0
17 APR 1700	101	00	00	00	9	*	20 APR 2000	251	00	00	00	0
17 APR 1730	102	00	00	00	9	*	20 APR 2030	252	00	00	00	0
17 APR 1800	103	00	00	00	8	*	20 APR 2100	253	00	00	00	0
17 APR 1830	104	00	00	00	8	*	20 APR 2130	254	00	00	00	0
17 APR 1900	105	00	00	00	8	*	20 APR 2200	255	00	00	00	0
17 APR 1930	106	00	00	00	7	*	20 APR 2230	256	00	00	00	0
17 APR 2000	107	00	00	00	7	*	20 APR 2300	257	00	00	00	0
17 APR 2030	108	00	00	00	7	*	20 APR 2330	258	00	00	00	0
17 APR 2100	109	00	00	00	7	*	21 APR 0000	259	00	00	00	0
17 APR 2130	110	00	00	00	6	*	21 APR 0030	260	00	00	00	0
17 APR 2200	111	00	00	00	6	*	21 APR 0100	261	00	00	00	0
17 APR 2230	112	00	00	00	6	*	21 APR 0130	262	00	00	00	0
17 APR 2300	113	00	00	00	6	*	21 APR 0200	263	00	00	00	0
17 APR 2330	114	00	00	00	6	*	21 APR 0230	264	00	00	00	0
18 APR 0000	115	00	00	00	5	*	21 APR 0300	265	00	00	00	0
18 APR 0030	116	00	00	00	5	*	21 APR 0330	266	00	00	00	0

18 APR 0100	117	00	00	00	5	*	21 APR 0400	267	00	00	00	0
18 APR 0130	118	00	00	00	5	*	21 APR 0430	268	00	00	00	0
18 APR 0200	119	00	00	00	5	*	21 APR 0500	269	00	00	00	0
18 APR 0230	120	00	00	00	5	*	21 APR 0530	270	00	00	00	0
18 APR 0300	121	00	00	00	4	*	21 APR 0600	271	00	00	00	0
18 APR 0330	122	00	00	00	4	*	21 APR 0630	272	00	00	00	0
18 APR 0400	123	00	00	00	4	*	21 APR 0700	273	00	00	00	0
18 APR 0430	124	00	00	00	4	*	21 APR 0730	274	00	00	00	0
18 APR 0500	125	00	00	00	4	*	21 APR 0800	275	00	00	00	0
18 APR 0530	126	00	00	00	4	*	21 APR 0830	276	00	00	00	0
18 APR 0600	127	00	00	00	4	*	21 APR 0900	277	00	00	00	0
18 APR 0630	128	00	00	00	4	*	21 APR 0930	278	00	00	00	0
18 APR 0700	129	00	00	00	3	*	21 APR 1000	279	00	00	00	0
18 APR 0730	130	00	00	00	3	*	21 APR 1030	280	00	00	00	0
18 APR 0800	131	00	00	00	3	*	21 APR 1100	281	00	00	00	0
18 APR 0830	132	00	00	00	3	*	21 APR 1130	282	00	00	00	0
18 APR 0900	133	00	00	00	3	*	21 APR 1200	283	00	00	00	0
18 APR 0930	134	00	00	00	3	*	21 APR 1230	284	00	00	00	0
18 APR 1000	135	00	00	00	3	*	21 APR 1300	285	00	00	00	0
18 APR 1030	136	00	00	00	3	*	21 APR 1330	286	00	00	00	0
18 APR 1100	137	00	00	00	3	*	21 APR 1400	287	00	00	00	0
18 APR 1130	138	00	00	00	3	*	21 APR 1430	288	00	00	00	0
18 APR 1200	139	00	00	00	3	*	21 APR 1500	289	00	00	00	0
18 APR 1230	140	00	00	00	3	*	21 APR 1530	290	00	00	00	0
18 APR 1300	141	00	00	00	3	*	21 APR 1600	291	00	00	00	0
18 APR 1330	142	00	00	00	2	*	21 APR 1630	292	00	00	00	0
18 APR 1400	143	00	00	00	2	*	21 APR 1700	293	00	00	00	0
18 APR 1430	144	00	00	00	2	*	21 APR 1730	294	00	00	00	0
18 APR 1500	145	00	00	00	2	*	21 APR 1800	295	00	00	00	0
18 APR 1530	146	00	00	00	2	*	21 APR 1830	296	00	00	00	0
18 APR 1600	147	00	00	00	2	*	21 APR 1900	297	00	00	00	0
18 APR 1630	148	00	00	00	2	*	21 APR 1930	298	00	00	00	0
18 APR 1700	149	00	00	00	2	*	21 APR 2000	299	00	00	00	0
18 APR 1730	150	00	00	00	2	*	21 APR 2030	300	00	00	00	0

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TOTAL RAINFALL - 191 61, TOTAL LOSS - 76 93, TOTAL EXCESS - 114 69

PEAK FLOW	TIME		6-HR	24-HR	72-HR	149 50-HR
(CU M/S)	(HR)	(CU M/S)				
720	16 00	(MM)	568	252	89	43
		(1000 CU M)	59 633	105 900	112 443	113 123
			12266	21784	23130	23269

CUMULATIVE AREA - 205 70 SQ KM

145 KO OUTPUT CONTROL VARIABLES

IPRMT 1 PRINT CONTROL

IPLOT 0 PLOT CONTROL

QSCAL 0 HYDROGRAPH PLOT SCALE

HYDROGRAPH ROUTING DATA

146 RS STORAGE ROUTING

NSIPS 1 NUMBER OF SUBREACHES

ITYP 0 TYPE OF INITIAL CONDITION

RSVRC 30000 00 INITIAL CONDITION

X -1 00 WORKING R AND D COEFFICIENT

147 SV STORAGE 0 18043.0 22516.0 28092.3 33668.4 39244.4

148 SE ELEVATION 97 00 98 00 99 00 100.00 101.00 102 00

149 SS SPILLWAY

CREL 100 50 SPILLWAY CREST ELEVATION

SPWID 130 00 SPILLWAY WIDTH

COQW 1 40 WEIR COEFFICIENT

EXPW 1 50 EXPONENT OF HEAD

COMPUTED OUTFLOW-ELEVATION DATA

OUTFLOW	00	00	06	46	1 55	3 67	7 17	12 38	19 66	29 35
ELEVATION	97 00	100 50	100 50	100.52	100 54	100.57	100 62	100 67	100 73	100.80
OUTFLOW	41 79	57 33	76 31	99 07	125 96	157 32	193.49	234 83	281 67	334 36
ELEVATION	100 88	100 96	101 06	101.17	101 28	101 41	101.54	101 69	101 84	102 00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	00	18043 00	22516 00	28092 30	30880 35	30983.60	31112 67	31293 39	31525 71	31809 69
OUTFLOW	00	00	00	.00	00	.46	1.55	3.67	7.17	12.38
ELEVATION	97 00	98 00	99 00	100.00	100 50	100.52	100.54	100 57	100 62	100 67
STORAGE	32145 30	32532 52	32971 39	33461.86	33668 40	34004 01	34597 72	35243 11	35940 11	36688 72
OUTFLOW	19 66	29 35	41 79	57.33	64 35	76 31	99 07	125 96	157 32	193 49
ELEVATION	100 73	100 80	100.88	100 96	101 00	101 06	101 17	101 28	101 41	101 54
STORAGE	37489 01	38340 86	39244 40							
OUTFLOW	234 83	281 67	334 36							
ELEVATION	101 69	101 84	102 00							

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HYDROGRAPH AT STATION BARR

DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE
15 APR	1500	1	0	30000 0	100 3	17 APR	1700	101	59	33506 1	101 0	19 APR	1900	201	9	31603 6	100 6			
15 APR	1530	2	0	30000 0	100 3	17 APR	1730	102	57	33461 7	101 0	19 APR	1930	202	8	31596 6	100 6			
15 APR	1600	3	0	30000 0	100 3	17 APR	1800	103	56	33418 3	101.8	19 APR	2000	203	8	31589 7	100 6			
15 APR	1630	4	0	30000 0	100 3	17 APR	1830	104	55	33375 9	100 9	19 APR	2030	204	8	31582.9	100 6			
15 APR	1700	5	0	30000 0	100 3	17 APR	1900	105	53	33334 4	100 9	19 APR	2100	205	8	31576.2	100 6			
15 APR	1730	6	0	30000 0	100 3	17 APR	1930	106	52	33293.8	100.9	19 APR	2130	206	8	31569 6	100 6			
15 APR	1800	7	0	30000 0	100 3	17 APR	2000	107	51	33254 0	100.9	19 APR	2200	207	8	31563 1	100 6			

15 APR 1830	8	0	30000 0	100 3	* 17 APR 2030	108	50	33215.2	100 9	* 19 APR 2230	208	8.	31556.7	100.6
15 APR 1900	9	0	30000 0	100 3	* 17 APR 2100	109	48	33177.2	100 9	* 19 APR 2300	209	8	31550.4	100 6
15 APR 1930	10	0	30000 0	100 3	* 17 APR 2130	110	47	33140.0	100 9	* 19 APR 2330	210	8	31544.2	100 6
15 APR 2000	11	0	30000 0	100 3	* 17 APR 2200	111	46	33103.7	100 9	* 20 APR 0000	211	7	31538.1	100 6
15 APR 2030	12	0	30000 0	100 3	* 17 APR 2230	112	45	33068.3	100 9	* 20 APR 0030	212	7	31532.1	100.6
15 APR 2100	13	0	30000 0	100 3	* 17 APR 2300	113	44	33033.6	100 9	* 20 APR 0100	213	7	31526.2	100 6
15 APR 2130	14	0	30000 0	100 3	* 17 APR 2330	114	43	32999.8	100 9	* 20 APR 0130	214	7	31520.3	100 6
15 APR 2200	15	0	30000 0	100 3	* 18 APR 0000	115	42	32966.7	100 9	* 20 APR 0200	215	7	31514.5	100 6
15 APR 2230	16	0	30000 0	100 3	* 18 APR 0030	116	41	32934.4	100 9	* 20 APR 0230	216	7	31508.8	100 6
15 APR 2300	17	0	30000 0	100 3	* 18 APR 0100	117	40	32902.7	100 9	* 20 APR 0300	217	7	31503.2	100 6
15 APR 2330	18	0	30000 0	100 3	* 18 APR 0130	118	39	32871.7	100 9	* 20 APR 0330	218	7	31497.6	100 6
16 APR 0000	19	0	30000 1	100 3	* 18 APR 0200	119	38	32841.3	100 9	* 20 APR 0400	219	7	31492.1	100 6
16 APR 0030	20	0	30000 1	100 3	* 18 APR 0230	120	37	32811.5	100 8	* 20 APR 0430	220	7	31486.7	100 6
16 APR 0100	21	0	30000 2	100 3	* 18 APR 0300	121	36	32782.4	100 8	* 20 APR 0500	221	6	31481.3	100.6
16 APR 0130	22	0	30000 4	100 3	* 18 APR 0330	122	36	32753.9	100 8	* 20 APR 0530	222	6	31476.0	100 6
16 APR 0200	23	0	30000 9	100 3	* 18 APR 0400	123	35	32726.0	100.8	* 20 APR 0600	223	6.	31470.8	100.6
16 APR 0230	24	0	30002 1	100 3	* 18 APR 0430	124	34	32698.7	100 8	* 20 APR 0630	224	6	31465.6	100 6
16 APR 0300	25	0	30005 6	100 3	* 18 APR 0500	125	33	32671.9	100 8	* 20 APR 0700	225	6	31460.5	100 6
16 APR 0330	26	0	30017 1	100 3	* 18 APR 0530	126	33	32645.8	100 8	* 20 APR 0730	226	6	31455.4	100 6
16 APR 0400	27	0	30059 7	100 4	* 18 APR 0600	127	32	32620.1	100 8	* 20 APR 0800	227	6	31450.4	100.6
16 APR 0430	28	0	30187 1	100 4	* 18 APR 0630	128	31	32595.1	100 8	* 20 APR 0830	228	6	31445.5	100 6
16 APR 0500	29	0	30458 4	100 4	* 18 APR 0700	129	30	32570.5	100.8	* 20 APR 0900	229	6	31440.6	100.6
16 APR 0530	30	0	30886 8	100 5	* 18 APR 0730	130	30	32546.5	100 8	* 20 APR 0930	230	6.	31435.8	100 6
16 APR 0600	31	6	31435 3	100 6	* 18 APR 0800	131	29	32523.1	100 8	* 20 APR 1000	231	6	31431.0	100 6
16 APR 0630	32	18	32047 1	100 7	* 18 APR 0830	132	29	32500.0	100.8	* 20 APR 1030	232	6	31426.3	100 6
16 APR 0700	33	33	32670 6	100 8	* 18 APR 0900	133	28	32477.4	100.8	* 20 APR 1100	233	6	31421.7	100.6
16 APR 0730	34	51	33268 2	100 9	* 18 APR 0930	134	27	32455.2	100 8	* 20 APR 1130	234	6.	31417.1	100 6
16 APR 0800	35	70	33816 0	101 0	* 18 APR 1000	135	27	32433.5	100.8	* 20 APR 1200	235	5.	31412.6	100.6
16 APR 0830	36	80	34301 6	101 1	* 18 APR 1030	136	26	32412.2	100 8	* 20 APR 1230	236	5	31408.1	100.6
16 APR 0900	37	104	34721 6	101 2	* 18 APR 1100	137	26	32391.2	100 8	* 20 APR 1300	237	5	31403.7	100 6
16 APR 0930	38	119	35078 6	101 3	* 18 APR 1130	138	25	32370.7	100 8	* 20 APR 1330	238	5.	31399.3	100 6
16 APR 1000	39	132	35378 1	101 3	* 18 APR 1200	139	25	32350.6	100 8	* 20 APR 1400	239	5	31395.0	100 6
16 APR 1030	40	143	35627 8	101 4	* 18 APR 1230	140	24	32330.9	100 8	* 20 APR 1430	240	5.	31390.8	100 6
16 APR 1100	41	153	35835 5	101 4	* 18 APR 1300	141	24	32311.5	100.8	* 20 APR 1500	241	5	31386.6	100.6
16 APR 1130	42	161	36000 5	101 4	* 18 APR 1330	142	23	32292.5	100.8	* 20 APR 1530	242	5	31382.4	100 6
16 APR 1200	43	168	36152 5	101 4	* 18 APR 1400	143	23	32273.9	100 7	* 20 APR 1600	243	5	31378.3	100 6
16 APR 1230	44	173	36272 0	101 5	* 18 APR 1430	144	22	32255.6	100 7	* 20 APR 1630	244	5.	31374.3	100 6
16 APR 1300	45	178	36370 5	101 5	* 18 APR 1500	145	22	32237.7	100 7	* 20 APR 1700	245	5	31370.3	100 6
16 APR 1330	46	182	36450 4	101 5	* 18 APR 1530	146	22	32220.2	100 7	* 20 APR 1730	246	5.	31366.4	100 6
16 APR 1400	47	185	36513 8	101 5	* 18 APR 1600	147	21	32203.0	100 7	* 20 APR 1800	247	5	31362.5	100.6
16 APR 1430	48	187	36562 0	101 5	* 18 APR 1630	148	21	32186.1	100 7	* 20 APR 1830	248	5	31358.6	100 6
16 APR 1500	49	189	36596 6	101 5	* 18 APR 1700	149	20	32169.6	100 7	* 20 APR 1900	249	5	31354.8	100 6
16 APR 1530	50	190	36618 8	101 5	* 18 APR 1730	150	20	32153.3	100 7	* 20 APR 1930	250	5.	31351.0	100.6
16 APR 1600	51	191	36629 4	101 5	* 18 APR 1800	151	19	32137.4	100 7	* 20 APR 2000	251	4	31347.3	100 6
16 APR 1630	52	191	36629 1	101 5	* 18 APR 1830	152	19	32121.8	100 7	* 20 APR 2030	252	4	31343.7	100 6
16 APR 1700	53	190	36618 3	101 5	* 18 APR 1900	153	19	32106.4	100 7	* 20 APR 2100	253	4	31340.0	100.6
16 APR 1730	54	189	36597 4	101 5	* 18 APR 1930	154	18	32091.3	100 7	* 20 APR 2130	254	4	31336.5	100.6
16 APR 1800	55	188	36566 9	101 5	* 18 APR 2000	155	18	32076.4	100 7	* 20 APR 2200	255	4	31332.9	100 6
16 APR 1830	56	186	36527 2	101 5	* 18 APR 2030	156	18	32061.8	100 7	* 20 APR 2230	256	4	31329.4	100 6
16 APR 1900	57	183	36479 3	101 5	* 18 APR 2100	157	18	32047.5	100 7	* 20 APR 2300	257	4	31326.0	100 6
16 APR 1930	58	181	36424 1	101 5	* 18 APR 2130	158	17	32033.4	100 7	* 20 APR 2330	258	4	31322.6	100 6
16 APR 2000	59	178	36362 6	101 5	* 18 APR 2200	159	17	32019.5	100 7	* 21 APR 0000	259	4.	31319.2	100 6
16 APR 2030	60	175	36295 8	101 5	* 18 APR 2230	160	17	32005.9	100 7	* 21 APR 0030	260	4.	31315.9	100.6
16 APR 2100	61	171	36224 6	101 5	* 18 APR 2300	161	16	31992.5	100 7	* 21 APR 0100	261	4	31312.6	100 6
16 APR 2130	62	167	36149 9	101 4	* 18 APR 2330	162	16	31979.3	100 7	* 21 APR 0130	262	4	31309.4	100 6
16 APR 2200	63	164	36072 5	101 4	* 19 APR 0000	163	16	31966.3	100 7	* 21 APR 0200	263	4	31306.2	100 6
16 APR 2230	64	160	35993 0	101 4	* 19 APR 0030	164	16.	31953.6	100 7	* 21 APR 0230	264	4.	31303.0	100 6
16 APR 2300	65	156	35912 1	101 4	* 19 APR 0100	165	15	31941.1	100 7	* 21 APR 0300	265	4	31299.9	100 6
16 APR 2330	66	152	35830 0	101 4	* 19 APR 0130	166	15	31928.8	100 7	* 21 APR 0330	266	4	31296.8	100 6
17 APR 0000	67	149	35747 2	101 4	* 19 APR 0200	167	15	31916.7	100 7	* 21 APR 0400	267	4	31293.8	100 6
17 APR 0030	68	145	35664 0	101 4	* 19 APR 0230	168	14	31904.9	100 7	* 21 APR 0430	268	4	31290.8	100 6
17 APR 0100	69	141	35580 7	101 3	* 19 APR 0300	169	14	31893.2	100 7	* 21 APR 0500	269	4	31287.8	100 6
17 APR 0130	70	137	35497 7	101 3	* 19 APR 0330	170	14	31881.8	100 7	* 21 APR 0530	270	4.	31284.8	100 6
17 APR 0200	71	134	35415 1	101 3	* 19 APR 0400	171	14	31870.5	100 7	* 21 APR 0600	271	4.	31281.9	100 6
17 APR 0230	72	130	35333 3	101 3	* 19 APR 0430	172	13	31859.4	100 7	* 21 APR 0630	272	4	31279.0	100 6
17 APR 0300	73	126	35252 4	101 3	* 19 APR 0500	173	13	31848.6	100 7	* 21 APR 0700	273	3	31276.2	100 6
17 APR 0330	74	123	35172 5	101 3	* 19 APR 0530	174	13.	31837.9	100 7	* 21 APR 0730	274	3	31273.3	100 6
17 APR 0400	75	120	35093 6	101 3	* 19 APR 0600	175	13	31827.4	100 7	* 21 APR 0800	275	3	31270.5	100 6
17 APR 0430	76	116	35015 8	101 2	* 19 APR 0630	176	13	31817.1	100 7	* 21 APR 0830	276	3	31267.7	100 6
17 APR 0500	77	113	34939 2	101 2	* 19 APR 0700	177	12	31806.9	100 7	* 21 APR 0900	277	3.	31265.0	100 6
17 APR 0530	78	110	34863 8	101 2	* 19 APR 0730	178	12	31796.9	100 7	* 21 APR 0930	278	3	31262.2	100 6
17 APR 0600	79	107	34789 8	101 2	* 19 APR 0800	179	12	31787.1	100 7	* 21 APR 1000	279	3	31259.5	100 6
17 APR 0630	80	104	34717 2	101 2	* 19 APR 0830	180	12	31777.4	100 7	* 21 APR 1030	280	3	31256.8	100 6
17 APR 0700	81	101	34646 0	101 2	* 19 APR 0900	181	12	31767.8	100 7	* 21 APR 1100	281	3	31254.2	100 6
17 APR 0730	82	98	34576 4	101 2	* 19 APR 0930	182	11	31758.4	100 7	* 21 APR 1130	282	3	31251.5	100 6
17 APR 0800	83	96	34508 1	101 2	* 19 APR 1000	183	11	31749.1	100 7	* 21 APR 1200	283	3	31248.9	100 6
17 APR 0830	84	93	34441 2	101 1	* 19 APR 1030	184	11	31740.0	100 7	* 21 APR 1230	284	3	31246.4	100 6
17 APR 0900	85	91	34375 7	101 1	* 19 APR 1100	185	11	31731.0	100 7	* 21 APR 1300	285	3.	31243.8	100 6
17 APR 0930	86	88	34311 5	101 1	* 19 APR 1130	186	11	31722.1	100 7	* 21 APR 1330	286	3	31241.3	100 6
17 APR 1000	87	86	34248 8	101 1	* 19 APR 1200	187	11							

CUMULATIVE AREA - 205 70 SQ KM

1

RUNOFF SUMMARY, AVERAGE FLOW IN CUBIC METERS PER SECOND
AREA IN SQUARE KILOMETERS

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT		88 04	14 50	55 44	20 36	6 94	15 83		
HYDROGRAPH AT		91 04	15 50	67 29	27 60	9 65	22 10		
HYDROGRAPH AT		47 80	16 50	43 35	22 84	8 41	19 42		
HYDROGRAPH AT		55 25	16 00	41 42	17 27	6 06	13 87		
4 COMBINED AT	BACIA	258 94	15 50	202 83	88 01	31 05	71 22		
HYDROGRAPH AT		12 01	15 00	9 23	3 81	1 33	3 04		
HYDROGRAPH AT		33 32	15 00	24 87	10 01	3 48	7 96		
2 COMBINED AT	BACIA	291 67	15 50	227 15	98 02	34.54	79 18		
HYDROGRAPH AT		323 76	15 50	250 89	108 03	38 06	87 51		
HYDROGRAPH AT		43 66	20 00	40 12	22 35	8 39	19 41		
HYDROGRAPH AT		86 92	16 00	74 86	40 92	15 07	34 89		
HYDROGRAPH AT		42 22	15 50	35 62	15 83	5 62	12.89		
2 COMBINED AT	BACIA	128 29	16 00	109 27	56 69	20.68	47 78		
HYDROGRAPH AT		146 26	16 00	122.83	63 10	22.95	53.06		
HYDROGRAPH AT		76 14	14 00	51 76	19 16	6 56	14 94		
HYDROGRAPH AT		116 49	14 50	77 32	28 78	9 84	22 46		
3 COMBINED AT	BACIA	565 63	15 50	444 57	199 66	70 85	163.03		
HYDROGRAPH AT		574 24	15 50	452 62	203 89	72.36	166.70		
HYDROGRAPH AT		38 96	13 50	23 48	8 33	2 83	6 44		
HYDROGRAPH AT		89 36	14 00	54.67	19 74	6 73	15 29		
HYDROGRAPH AT		75 71	16 00	57 73	24 35	8 55	19 60		
3 COMBINED AT	BACIA	712 75	15 50	558 37	247 67	87 63	201 59		
HYDROGRAPH AT		720 07	16 00	567 89	252 13	89 23	205.70		
ROUTED TO	BARR	190 62	25 00	187 50	150 55	75 20	205 70	101 53	25 00

1

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

ISTAQ	ELEMENT	DT	PEAK	TIME TO PEAK	VOLUME	DT	INTERPOLATED TO COMPUTATION INTERVAL		VOLUME		
							PEAK	TIME TO PEAK			
		(MIN)	(CMS)	(MIN)	(MM)	(MIN)	(CMS)	(MIN)	(MM)		
	MANE	24 02	89 31	849 81	113 96	30 00	87 99	870 08	113 97		
CONTINUITY SUMMARY (1000 CU-M)-INFLOW-			0000E+00	EXCESS-	1812E+04	OUTFLOW-	1798E+04	BASIN STORAGE-	1149E+01	PERCENT ERROR-	7
	MANE	24 08	91 64	934 85	113 75	30 00	90 98	930 00	113 75		
CONTINUITY SUMMARY (1000 CU-M)-INFLOW-			0000E+00	EXCESS-	2530E+04	OUTFLOW-	2506E+04	BASIN STORAGE-	4987E+01	PERCENT ERROR-	8
	MANE	16 31	47 85	992 78	113 60	30 00	47 77	990 08	113 58		
CONTINUITY SUMMARY (1000 CU-M)-INFLOW-			0000E+00	EXCESS-	2223E+04	OUTFLOW-	2199E+04	BASIN STORAGE-	1148E+02	PERCENT ERROR-	6

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	MANE	26 27	55 23	960 96	113 83	30 00	55 22	960 00	113 84	
CONTINUITY SUMMARY	(1000 CU-M)-INFLOW-	0000E+00	EXCESS-	1588E+04	OUTFLOW-	1574E+04	BASIN STORAGE-	3103E+01	PERCENT ERROR-	7
	MANE	6 66	12 01	898 86	113 98	30 00	12.01	900 00	114 30	
CONTINUITY SUMMARY	(1000 CU-M)-INFLOW-	0000E+00	EXCESS-	3480E+03	OUTFLOW-	3453E+03	BASIN STORAGE-	6814E+00	PERCENT ERROR-	6
	MANE	10 66	33 45	911 44	114 09	30 00	33.30	900 00	113.99	
CONTINUITY SUMMARY	(1000 CU-M)-INFLOW-	3461E+03	EXCESS-	.5633E+03	OUTFLOW-	9051E+03	BASIN STORAGE-	8841E+00	PERCENT ERROR-	4
	MANE	15 04	324 52	936 19	113 46	30 00	323 56	930 00	113 45	
CONTINUITY SUMMARY	(1000 CU-M)-INFLOW-	8974E+04	EXCESS-	9536E+03	OUTFLOW-	8896E+04	BASIN STORAGE-	2473E+01	PERCENT ERROR-	3
	MANE	27 77	43 64	1199 67	113.64	30 00	43 64	1200 00	113 64	
CONTINUITY SUMMARY	(1000 CU-M)-INFLOW-	0000E+00	EXCESS-	2222E+04	OUTFLOW-	2199E+04	BASIN STORAGE-	1484E+02	PERCENT ERROR-	4
	MANE	30 00	87 33	966 72	113 20	30 00	86 87	960.00	113 20	
CONTINUITY SUMMARY	(1000 CU-M)-INFLOW-	2197E+04	EXCESS-	1772E+04	OUTFLOW-	3936E+04	BASIN STORAGE-	4308E+01	PERCENT ERROR-	7
	MANE	17 86	42 24	925 43	113 81	30 00	42 20	930 00	113 77	
CONTINUITY SUMMARY	(1000 CU-M)-INFLOW-	0000E+00	EXCESS-	1476E+04	OUTFLOW-	1462E+04	BASIN STORAGE-	4033E+01	PERCENT ERROR-	6
	MANE	10 88	146 40	956 65	113 23	30 00	146 17	960 00	113 23	
CONTINUITY SUMMARY	(1000 CU-M)-INFLOW-	5394E+04	EXCESS-	6045E+03	OUTFLOW-	5988E+04	BASIN STORAGE-	1608E+01	PERCENT ERROR-	2
	MANE	16 72	76 52	845 68	113 96	30 00	76.10	840 00	114 05	
CONTINUITY SUMMARY	(1000 CU-M)-INFLOW-	0000E+00	EXCESS-	1710E+04	OUTFLOW-	1697E+04	BASIN STORAGE-	1490E+01	PERCENT ERROR-	7
	MANE	13 13	117 26	854 95	113 86	30 00	116 42	870 00	113 90	
CONTINUITY SUMMARY	(1000 CU-M)-INFLOW-	1697E+04	EXCESS-	8609E+03	OUTFLOW-	2549E+04	BASIN STORAGE-	6979E+00	PERCENT ERROR-	3
	MANE	5 20	578 99	940 69	113 32	30 00	573.90	930.00	113 30	
CONTINUITY SUMMARY	(1000 CU-M)-INFLOW-	1842E+05	EXCESS-	4201E+03	OUTFLOW-	1883E+05	BASIN STORAGE-	1600E+01	PERCENT ERROR-	1
	MANE	12 18	39 22	822 55	114 16	30 00	38 94	810 00	114 15	
CONTINUITY SUMMARY	(1000 CU-M)-INFLOW-	0000E+00	EXCESS-	7373E+03	OUTFLOW-	7328E+03	BASIN STORAGE-	4450E+00	PERCENT ERROR-	5
	MANE	13 76	89 51	853 45	114 19	30 00	89 30	840 00	114 34	
CONTINUITY SUMMARY	(1000 CU-M) INFLOW-	7322E+03	EXCESS-	1013E+04	OUTFLOW-	1740E+04	BASIN STORAGE-	.7830E+00	PERCENT ERROR-	.2
	MANE	29 38	75 95	948 44	113.76	30 00	75 66	960.00	113 69	
CONTINUITY SUMMARY	(1000 CU-M)-INFLOW-	0000E+00	EXCESS-	2244E+04	OUTFLOW-	2222E+04	BASIN STORAGE-	4408E+01	PERCENT ERROR-	8
	MANE	7 36	726 51	945.94	113 25	30 00	719.64	960 00	113.18	
CONTINUITY SUMMARY	(1000 CU-M)-INFLOW-	2277E+05	EXCESS-	4701E+03	OUTFLOW-	2322E+05	BASIN STORAGE-	.2301E+01	PERCENT ERROR-	.1

*** NORMAL END OF HEC-1 ***

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+			486 24	16 00	389 70	180 09	64 31	166 70		
+	HYDROGRAPH AT		32 65	13 50	20 51	7 40	2 52	6 44		
+	HYDROGRAPH AT		74 78	14 50	47 56	17 48	5 97	15 29		
+	HYDROGRAPH AT		63 66	16 00	49 80	21 56	7.60	19.60		
+	3 COMBINED AT	BACIA	599 99	16 00	480 19	218 92	77 87	201 59		
+	HYDROGRAPH AT		612 54	16 00	488 24	222 69	79.29	205 70		
+	ROUTED TO	BARR	159 77	26 00	156 98	127 82	65 49	205 70	101 42	26 00

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUMBE ROUTING
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

ISTAQ	ELEMENT	DT (MIN)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	DT (MIN)	INTERPOLATED TO COMPUTATION INTERVAL		VOLUME (MM)		
							PEAK (CMS)	TIME TO PEAK (MIN)			
	MANE	25 16	76 39	864 79	101 35	30 00	75 84	870 00	101 41		
CONTINUITY SUMMARY (1000 CU-M)-INFLOW-			0000E+00	EXCESS-	1612E+04	OUTFLOW-	1599E+04	BASIN STORAGE-	1187E+01	PERCENT ERROR-	7
	MANE	25.15	76.77	953.46	101.19	30.00	76.30	960.00	101.17		
CONTINUITY SUMMARY (1000 CU-M)-INFLOW-			0000E+00	EXCESS-	2251E+04	OUTFLOW-	2229E+04	BASIN STORAGE-	4756E+01	PERCENT ERROR-	8
	MANE	17 12	39 95	1011 92	101 00	30.00	39 95	1020.00	101.00		
CONTINUITY SUMMARY (1000 CU-M)-INFLOW-			0000E+00	EXCESS-	1978E+04	OUTFLOW-	1955E+04	BASIN STORAGE-	1173E+02	PERCENT ERROR-	6
	MANE	27 45	46 95	956 84	101 23	30 00	46 86	960.00	101 28		
CONTINUITY SUMMARY (1000 CU-M)-INFLOW-			0000E+00	EXCESS-	1413E+04	OUTFLOW-	1399E+04	BASIN STORAGE-	3254E+01	PERCENT ERROR-	7
	MANE	7 10	9 98	933 22	101 39	30.00	9 98	930.00	101.30		
CONTINUITY SUMMARY (1000 CU-M)-INFLOW-			0000E+00	EXCESS-	3096E+03	OUTFLOW-	3072E+03	BASIN STORAGE-	6957E+00	PERCENT ERROR-	6
	MANE	10 99	28 38	909 94	101 34	30 00	28 16	930.00	101 25		
CONTINUITY SUMMARY (1000 CU-M)-INFLOW-			3067E+03	EXCESS-	5011E+03	OUTFLOW-	5040E+03	BASIN STORAGE-	6424E+00	PERCENT ERROR-	4
	MANE	15.58	274 43	957 18	100 89	30.00	273 77	960.00	100 88		
CONTINUITY SUMMARY (1000 CU-M)-INFLOW-			7981E+04	EXCESS-	8484E+03	OUTFLOW-	8799E+04	BASIN STORAGE-	2433E+01	PERCENT ERROR-	.3
	MANE	23 87	36 64	1231 43	101 03	30 00	36 62	1230 00	101 03		
CONTINUITY SUMMARY (1000 CU-M)-INFLOW-			0000E+00	EXCESS-	1977E+04	OUTFLOW-	1955E+04	BASIN STORAGE-	1416E+02	PERCENT ERROR-	.4
	MANE	27 03	72 56	973 51	100 81	30.00	72 40	990.00	100 78		
CONTINUITY SUMMARY (1000 CU-M)-INFLOW-			1953E+04	EXCESS-	1577E+04	OUTFLOW-	3505E+04	BASIN STORAGE-	4498E+01	PERCENT ERROR-	.6
	MANE	18 57	35 43	944.72	101 21	30 00	35.35	930 00	101 24		
CONTINUITY SUMMARY (1000 CU-M)-INFLOW-			0000E+00	EXCESS-	1313E+04	OUTFLOW-	1300E+04	BASIN STORAGE-	4098E+01	PERCENT ERROR-	6
	MANE	11 41	121 71	978 97	100 78	30 00	121.39	990.00	100.77		
CONTINUITY SUMMARY (1000 CU-M)-INFLOW-			4802E+04	EXCESS-	5377E+03	OUTFLOW-	5329E+04	BASIN STORAGE-	1575E+01	PERCENT ERROR-	2
	MANE	17 56	64 45	861 76	101 32	30 00	64.44	870 00	101.22		
CONTINUITY SUMMARY (1000 CU-M)-INFLOW-			0000E+00	EXCESS-	1522E+04	OUTFLOW-	1509E+04	BASIN STORAGE-	1364E+01	PERCENT ERROR-	8
	MANE	13 73	99 25	871 39	101.13	30.00	99.14	870.00	101 19		
CONTINUITY SUMMARY (1000 CU-M)-INFLOW-			1506E+04	EXCESS-	7659E+03	OUTFLOW-	2264E+04	BASIN STORAGE-	7091E+00	PERCENT ERROR-	3
	MANE	5 52	488 01	943 22	100 77	30 00	485 95	960 00	100 76		
CONTINUITY SUMMARY (1000 CU-M)-INFLOW-			1638E+05	EXCESS-	3738E+03	OUTFLOW-	1674E+05	BASIN STORAGE-	1622E+01	PERCENT ERROR-	1

MANE	12 73	33 02	824 74	101 48	30.00	32 63	810 00	101 47	
CONTINUITY SUMMARY (1000 CU-M)-INFLOW-	0000E+00	EXCESS-	6559E+03	OUTFLOW-	6514E+03	BASIN STORAGE-	4270E+00	PERCENT ERROR-	6
MANE	14 41	75 64	854 81	101 43	30 00	74 74	870 00	101 46	
CONTINUITY SUMMARY (1000 CU-M)-INFLOW-	6508E+03	EXCESS-	9013E+03	OUTFLOW-	1546E+04	BASIN STORAGE-	8552E+00	PERCENT ERROR-	4
MANE	30 00	64 32	969 24	101 22	30 00	63 62	960 00	101 16	
CONTINUITY SUMMARY (1000 CU-M)-INFLOW-	0000E+00	EXCESS-	1996E+04	OUTFLOW-	1977E+04	BASIN STORAGE-	4511E+01	PERCENT ERROR-	.7
MANE	7 82	612 75	950 26	100 69	30 00	612.18	960 00	100 63	
CONTINUITY SUMMARY (1000 CU-M)-INFLOW-	2025E+05	EXCESS-	4182E+03	OUTFLOW-	2064E+05	BASIN STORAGE-	2263E+01	PERCENT ERROR-	1

*** NORMAL END OF HEC-1 ***

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000124

Simulação Modelo TR 10.000

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*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* SEPTEMBER 1990 *
* VERSION 4 0 *
* RUN DATE 05/05/1996 TIME 18 11 28 *
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*****
* U S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1G, HEC1DB, AND HEC1KW. THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMBRK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 29 SEP 81 THIS IS THE FORTRAN77 VERSION. NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS-WRITE STAGE FREQUENCY, DSS-READ TIME SERIES AT DESIRED CALCULATION INTERVAL, LOSS RATE GREEN AND AMPT INFILTRATION, KINEMATIC WAVE NEW FINITE DIFFERENCE ALGORITHM.

1 HEC-1 INPUT PAGE 1

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	BACIA HIDROGRAFICA DO RESERVATORIO DE SOUSA									
2	ID	MODELO DE ONDA CINEMATICA DA BACIA TR-10000									
3	IT	30	15APR96	1500	300						
4	IO	30									
5	IN										
		*DIAGRAM									
6	KK	BACIA ABANICO COMPONENTE E									
7	KM	ESC SUPERFICIAL SUBBACIA E									
8	KO	3									
9	BA	15 83									
10	PH	0 1	220 0	26.0	49.0	98 7	120 0	138 0	190 0	215 0	260 5
11	LS		74			74					
12	UK	810	05	40	40.6						
13	UK	1170	05	40	59.4						
14	RK	8000	006	045		TRAP	0	40			
15	KK	BACIA ABANICO COMPONENTE H									
16	KM	ESC SUPERFICIAL SUBBACIA H									
17	KO	3									
18	BA	22 1									
19	UK	1130	016	40	46 1						
20	UK	1320	016	40	53.9						
21	RK	9000	008	045		TRAP	0	40			
22	KK	BACIA ABANICO COMPONENTE I									
23	KM	ESC SUPERFICIAL SUBBACIA I									
24	KO	3									
25	BA	19 42									
26	UK	1700	016	40	35 0						
27	UK	3150	016	40	65 0						
28	RK	4000	0040	045		TRAP	0	40			
29	KK	BACIA ABANICO COMPONENTE J									
30	KM	ESC SUPERFICIAL SUBBACIA J									
31	KO	3									
32	BA	13 87									
33	UK	1210	013	40	52.3						
34	UK	1100	013	40	47.7						
35	RK	6000	0030	045		TRAP	0	40			
36	KK	BACIA EM ABANICO COMPLETA									
37	HC	4									
38	KK	BACIA F-G COMPONENTE F									
39	KM	ESC SUPERFICIAL SUBBACIA F									
40	KO	3									
41	BA	3 04									
42	UK	1250	010	40	61.0						
43	UK	780	010	40	38 2						
44	RK	1500	0080	045		TRAP	0	40			

1 HEC-1 INPUT PAGE 2

LINE	ID	1	2	3	4	5	6	7	8	9	10
45	KK	BACIA F - G COMPONENTE G									
46	KM	ESC SUPERFICIAL SUBBACIA G									
47	KO	3									
48	BA	4 92									
49	UK	980	010	40	59 9						
50	UK	660	010	40	40 1						
51	RK	3000	0080	045		TRAP	0	40	YES		
52	KK	BACIA EM ABANICO COMPLETA + BACIA F - G									
53	HC	2									
54	KK	BACIA P									
55	KM	ESC SUPERFICIAL BACIA P									
56	KO	3									
57	BA	8 33									
58	UK	820	013	40	49.4						
59	UK	840	013	40	50.6						
60	RK	5000	0030	045		TRAP	0	40	YES		

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61	KK	BACIA K-L COMPONENTE K						
62	KM	ESC SUPERFICIAL SUBBACIA K						
63	KO	3						
64	BA	19 41						
65	UK	1340	003	40	48.8			
66	UK	1400	003	40	51.2			
67	RK	7100	0080	045		TRAP	0 40	
68	KK	BACIA K - L COMPONENTE L						
69	KM	ESC SUPERFICIAL SUBBACIA L						
70	KO	3						
71	BA	15 48						
72	UK	440	020	40	24.3			
73	UK	1380	020	40	75.7			
74	RK	8500	0030	.045		TRAP	0 40 YES	
75	KK	BACIA M						
76	KM	ESC SUPERFICIAL BACIA M						
77	KO	3						
78	BA	12 89						
79	UK	750	010	40	31.9			
80	UK	1590	010	40	68.1			
81	RK	5500	0080	045		TRAP	0 40	
82	KK	BACIA M + BACIA K - L						
83	HC	2						
84	KK	BACIA Q						
85	KM	ESC SUPERFICIAL SUBBACIA Q						
86	KO	3						
87	BA	5 28						
88	UK	750	013	40	42.4			
89	UK	1010	.013	40	57.6			
90	RK	3000	0030	.045		TRAP	0 40 YES	

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1

LINE ID 1 2 3 ...4 5 .. 6. 7. . 8. .9 ...10

91	KK	BACIA D-C COMPONENTE D						
92	KM	ESC SUPERFICIAL SUBBACIA D						
93	KO	3						
94	BA	14.94						
95	UK	1860	080	40	74.8			
96	UK	630	080	40	25.2			
97	RK	6000	0080	045		TRAP	0 40	
98	KK	BACIA D - C COMPONENTE C						
99	KM	ESC. SUPERFICIAL SUBBACIA C						
100	KO	3						
101	BA	7 52						
102	UK	870	030	40	24.3			
103	UK	800	030	40	75.7			
104	RK	4500	0060	.045		TRAP	0 40 YES	
105	KK	BACIA Q + BACIA P + BACIA D-C						
106	HC	3						
107	KK	BACIA R						
108	KM	ESC SUPERFICIAL SUBBACIA R						
109	KO	3						
110	BA	3 67						
111	UK	1160	013	40	63.4			
112	UK	670	013	40	36.6			
113	RK	2000	0030	045		TRAP	0 40 YES	
114	KK	BACIA B-A COMPONENTE B						
115	KM	ESC SUPERFICIAL SUBBACIA B						
116	KO	3						
117	BA	6.44						
118	UK	490	050	40	30.5			
119	UK	1120	050	40	69.5			
120	RK	4000	0100	045		TRAP	0 40	
121	KK	BACIA B - A COMPONENTE A						
122	KM	ESC SUPERFICIAL SUBBACIA A						
123	KO	3						
124	BA	8 85						
125	UK	910	030	40	51.5			
126	UK	860	030	40	48.5			
127	RK	5000	0080	045		TRAP	0 40 YES	
128	KK	BACIA N						
129	KM	ESC SUPERFICIAL SUBBACIA N						
130	KO	3						
131	BA	19 60						
132	UK	780	013	40	37.9			
133	UK	1200	013	.40	62.1			
134	RK	9500	0030	045		TRAP	0 40	

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LINE ID 1 2 3 4 . 5 . 6.. 7 8. 9 ..10

135	KK	BACIA R + BACIA N + BACIA A-B						
136	HC	3						
137	KK	BACIA S						
138	KM	ESC SUPERFICIAL SUBBACIA S						
139	KO	1						
140	BA	4 11						
141	UK	450	.010	.40	32.6			
142	UK	920	010	40	67.3			
143	RK	3000	0030	045		TRAP	0 40 YES	
144	KK	BARRAGEM DE SOUSA						
145	KO	1						
146	RS	1	STOR	30000	-1			
147	SV	18043	22516.	28092.3	33668.4	39244.4		
148	SE	97	98	99	100	101	102	
149	SS	100 5	130.	1.4	1.5			
150	ZZ							

000127

61	KK	BACIA K-L COMPONENTE K			
62	KM	ESC SUPERFICIAL SUBBACIA K			
63	KO	3			
64	BA	19 41			
65	UK	1340	.003	40	48 8
66	UK	1400	003	40	51.2
67	RK	7100	0080	045	TRAP 0 40
68	KK	BACIA K - L COMPONENTE L			
69	KM	ESC SUPERFICIAL SUBBACIA L			
70	KO	3			
71	BA	15 48			
72	UK	440	020	40	24 3
73	UK	1380	020	40	75 7
74	RK	8500	0030	045	TRAP 0 40 YES
75	KK	BACIA M			
76	KM	ESC SUPERFICIAL BACIA M			
77	KO	3			
78	BA	12 89			
79	UK	750	010	40	31 9
80	UK	1590	010	40	68 1
81	RK	5500	0080	045	TRAP 0 40
82	KK	BACIA M + BACIA K - L			
83	HC	2			
84	KK	BACIA Q			
85	KM	ESC SUPERFICIAL SUBBACIA Q			
86	KO	3			
87	BA	5 28			
88	UK	750	013	40	42 4
89	UK	1010	013	40	57.6
90	RK	3000	0030	045	TRAP 0 40 YES

HEC-1 INPUT

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LINE ID 1 2 3 4 ... 5 6 7 . 8. 9 . 10

91	KK	BACIA D-C COMPONENTE D			
92	KM	ESC SUPERFICIAL SUBBACIA D			
93	KO	3			
94	BA	14 94			
95	UK	1860	080	40	74 8
96	UK	638	080	40	25.2
97	RK	6000	.0080	045	TRAP 0 40
98	KK	BACIA D - C COMPONENTE C			
99	KM	ESC SUPERFICIAL SUBBACIA C			
100	KO	3			
101	BA	7 52			
102	UK	870	.030	40	24 3
103	UK	800	030	40	75 7
104	RK	4500	.0060	045	TRAP 0 40 YES
105	KK	BACIA Q + BACIA P + BACIA D-C			
106	HC	3			
107	KK	BACIA R			
108	KM	ESC SUPERFICIAL SUBBACIA R			
109	KO	3			
110	BA	3 67			
111	UK	1160	013	40	63.4
112	UK	670	013	40	36 6
113	RK	2000	0030	045	TRAP 0 40 YES
114	KK	BACIA B-A COMPONENTE B			
115	KM	ESC SUPERFICIAL SUBBACIA B			
116	KO	3			
117	BA	6 44			
118	UK	490	050	40	30 5
119	UK	1120	050	40	69.5
120	RK	4800	0100	045	TRAP 0 40
121	KK	BACIA B - A COMPONENTE A			
122	KM	ESC SUPERFICIAL SUBBACIA A			
123	KO	3			
124	BA	8 85			
125	UK	910	030	40	51.5
126	UK	860	030	40	48 5
127	RK	5000	0080	045	TRAP 0 40 YES
128	KK	BACIA N			
129	KM	ESC SUPERFICIAL SUBBACIA N			
130	KO	3			
131	BA	19 60			
132	UK	780	013	40	37 9
133	UK	1280	013	40	62 1
134	RK	9500	0030	045	TRAP 0 40

HEC-1 INPUT

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LINE ID 1 2 3 4 . 5 6 . 7 . 8 9 10

135	KK	BACIA R + BACIA N + BACIA A-B			
136	HC	3			
137	KK	BACIA S			
138	KM	ESC SUPERFICIAL SUBBACIA S			
139	KO	1			
140	BA	4 11			
141	UK	450	010	40	32 6
142	UK	920	010	.40	67 3
143	RK	3000	0030	045	TRAP 0 40 YES
144	KK	BARRAGEM DE SOUSA			
145	KO	1			
146	RS	1	STOR	30000	-1
147	SV	18043.	22516.	28092.3	33668.4 39244.4
148	SE	97	98	99	100 101 102.
149	SS	100 5	130.	1 4	1 5
150	ZZ				

000128

METRIC UNITS

DRAINAGE AREA SQUARE KILOMETERS
 PRECIPITATION DEPTH MILLIMETERS
 LENGTH, ELEVATION METERS
 FLOW CUBIC METERS PER SECOND
 STORAGE VOLUME CUBIC METERS
 SURFACE AREA SQUARE METERS
 TEMPERATURE DEGREES CELSIUS

8 RO OUTPUT CONTROL VARIABLES
 IPRNT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

9 BA SUBBASIN CHARACTERISTICS
 TAREA 15 83 SUBBASIN AREA

PRECIPITATION DATA

10 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
 HYDRO-35 TP-49
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
 26 00 49 00 98 70 120 00 138 00 190.00 215.00 268.50 .00 .00 .00 .00

STORM AREA = 220 00

11 LS SCS LOSS RATE
 STRIL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA
 LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STRIL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

12 UR KINEMATIC WAVE
 OVERLAND-FLOW ELEMENT NO 1
 L 810 OVERLAND FLOW LENGTH
 S 0500 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 40 6 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

13 UR OVERLAND-FLOW ELEMENT NO 2
 L 1170 OVERLAND FLOW LENGTH
 S 0500 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 59.4 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

14 RK KINEMATIC WAVE
 MAIN CHANNEL
 L 8000 CHANNEL LENGTH
 S 0060 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 15.83 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 NDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ NO ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	56	1 67	15 34	162 05	62.29	776 19	156 06	18
PLANE2	56	1 67	20 45	234 08	77 19	809 64	155 77	19
MAIN	32	1 33	22 13	2667.54	123 91	842 73	155.79	2 01

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 0000E+00 EXCESS- .2472E+04 OUTFLOW- 2450E+04 BASIN STORAGE- 1173E+01 PERCENT ERROR- 5

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 32 1 33 30 00 123 68 840 80 155.62

HYDROGRAPH AT STATION

TOTAL RAINFALL = 237 92, TOTAL LOSS = 81 34, TOTAL EXCESS = 156 58

PEAK FLOW (CU M/S)	TIME (HR)	6-HR (CU M/S)	24-HR (MM)	72-HR (1000 CU M)	149 50-HR
+	124	14 00	76	28	9
+			103 411	152 195	155.542
			1637	2409	2462

CUMULATIVE AREA = 15 83 SQ KM

17 RO OUTPUT CONTROL VARIABLES
 IPRNT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

18 BA SUBBASIN CHARACTERISTICS
 TAREA 22 10 SUBBASIN AREA

000129

26 UK KINEMATIC WAVE
 OVERLAND-FLOW ELEMENT NO 1
 I 1700 OVERLAND FLOW LENGTH
 S 0160 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 35 0 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

27 UK OVERLAND-FLOW ELEMENT NO 2
 I 3150 OVERLAND FLOW LENGTH
 S 0160 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 65 0 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

28 RK KINEMATIC WAVE
 MAIN CHANNEL
 L 4000 CHANNEL LENGTH
 S 0040 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 19 42 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 WDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RIPSTQ NO ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	N	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	32	1 67	30 00	242 94	36 58	924 23	155 32	13
PLANE2	32	1 67	30 00	286 46	44 18	1071 60	155 12	14
MAIN	26	1 33	14 68	1333 77	73 68	951 82	155 20	1 52

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 0000E+00 EXCESS- .3032E+04 OUTFLOW- 3004E+04 BASIN STORAGE- 1223E+02 PERCENT ERROR- 5

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 26 1 33 30 00 73 53 960 00 155.17

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HYDROGRAPH AT STATION

TOTAL RAINFALL - 237 92, TOTAL LOSS - 81 34, TOTAL EXCESS - 156 58

PEAK FLOW + (CU M/S)	TIME (HR)	6-HR (CU M/S)	MAXIMUM 24-HR	AVERAGE FLOW 72-HR	149 50-HR
+ 74	16 00	65	32	12	6
		(MM) 71 803	141 486	153 659	155 091
		(1000 CU M) 1394	2748	2984	3012

CUMULATIVE AREA - 19 42 SQ KM

31 KO OUTPUT CONTROL VARIABLES
 IPRINT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

32 BA SUBBASIN CHARACTERISTICS
 IAREA 13 07 SUBBASIN AREA

PRECIPITATION DATA

10 PH DEPIHS FOR 0-PERCENT HYPOTHETICAL STORM
 HYDRO-35 TP-40
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
 26 00 49 00 98 70 120 00 138 00 190 00 215.00 260.50 00 .00 00 00
 STORM AREA - 220 00

11 LS SCS LOSS RATE
 STRIL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STRIL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

33 UK KINEMATIC WAVE
 OVERLAND-FLOW ELEMENT NO 1
 L 1210 OVERLAND FLOW LENGTH
 S 0130 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 52 3 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

34 UK OVERLAND-FLOW ELEMENT NO. 2
 L 1180 OVERLAND FLOW LENGTH
 S 0130 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 47 7 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

35 RK KINEMATIC WAVE
 MAIN CHANNEL
 L 6000 CHANNEL LENGTH
 S 0030 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT

000131

CA 13 87 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 NDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ NO ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWS IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	29	1 67	25.19	172.91	44 70	898.58	155 64	11
PLANE2	29	1 67	27 25	183 39	41 94	858 13	155 49	11
MAIN	22	1 33	23 98	2000 66	81 31	927 58	155.49	1 40

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 0000E+00 EXCESS- 2166E+04 OUTFLOW- 2150E+04 BASIN STORAGE- 3193E+01 PERCENT ERROR- .6

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 22 1 33 30 00 80.93 930 00 155 47

HYDROGRAPH AT STATION

TOTAL RAINFALL - 237 92. TOTAL LOSS - 81 34. TOTAL EXCESS - 156 58

PEAK FLOW (CU M/S)	TIME (HR)	6-HR (CU M/S)	24-HR (MM)	72-HR (1000 CU M)	149.50-HR (MM)
81	15 50	58	91 057	1263	149.50
			24	8	4
			148 302	2057	155.394
			154.785	2147	2155

CUMULATIVE AREA - 13 87 SQ KM

40 KO OUTPUT CONTROL VARIABLES
 IPRINT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

41 BA SUBBASIN CHARACTERISTICS
 TAREA 3 04 SUBBASIN AREA

PRECIPITATION DATA

DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM										TP-49	
HYDRO-35		TP-40						TP-49		10-DAY	
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
26 00	49 00	98 70	120 00	138 00	190.00	215 00	260 50	00	00	00	00

STORM AREA - 720 00

11 LS SCS LOSS RATE
 STRIL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STRIL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

42 UR KINEMATIC WAVE OVERLAND-FLOW ELEMENT NO 1
 L 1250 OVERLAND FLOW LENGTH
 S .0100 SLOPE
 N .400 ROUGHNESS COEFFICIENT
 PA 61 8 PERCENT OF SUBBASIN
 NDXMIN 5 MINIMUM NUMBER OF DX INTERVALS

43 UK OVERLAND-FLOW ELEMENT NO 2
 L 700 OVERLAND FLOW LENGTH
 S 0100 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 38 2 PERCENT OF SUBBASIN
 NDXMIN 5 MINIMUM NUMBER OF DX INTERVALS

44 RK KINEMATIC WAVE MAIN CHANNEL
 L 1500 CHANNEL LENGTH
 S 0800 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 3 04 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 NDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ NO ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWS IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	25	1 67	25 55	156 30	10.74	904.77	155 55	10
PLANE2	25	1 67	23.10	130.04	8 13	844 11	155.67	.09
MAIN	37	1 33	6 15	500 16	17 55	889.14	155 64	1 38

000132

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 37 1 33 30 00 17 40 870.00 155 34

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HYDROGRAPH AT STATION

TOTAL RAINFALL - 237 92, TOTAL LOSS - 81 34, TOTAL EXCESS - 156 58

PEAK FLOW (CU M/S)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	149 50-HR
17	14 50	13	5	2	1
		91 949	148 392	154.656	155 264
		(1000 CU M)	280	451	470

CUMULATIVE AREA - 3 04 SQ KM

47 KO OUTPUT CONTROL VARIABLES
 IPRNT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

48 BA SUBBASIN CHARACTERISTICS
 TAREA 4 92 SUBBASIN AREA

PRECIPITATION DATA

HYDRO-35	DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM										
	5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY
26 00	49 00	98 70	120 00	138 00	190 00	215 00	260.50	00	.00	00	00

STORM AREA - 220.00

11 LS SCS LOSS RATE
 STRIL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RIIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STRIL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RIIMP 00 PERCENT IMPERVIOUS AREA

49 UK KINEMATIC WAVE OVERLAND-FLOW ELEMENT NO. 1
 L 980. OVERLAND FLOW LENGTH
 S 0100 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 59.9 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

50 UK OVERLAND-FLOW ELEMENT NO. 2
 L 660 OVERLAND FLOW LENGTH
 S 0100 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 40 1 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

51 RK KINEMATIC WAVE MAIN CHANNEL
 L 3000 CHANNEL LENGTH
 S .0080 SLOPE
 N .045 CHANNEL ROUGHNESS COEFFICIENT
 CA 4.92 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 NDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ YES ROUTE UPSTREAM HYDROGRAPH

COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	25	1 67	23.76	140.05	18 80	881.27	155 63	10
PLANE2	25	1 67	24 26	132.04	14 72	823.64	155 61	09
MAIN	37	1 33	9.61	1000 33	48.32	873 32	155 52	1 74

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 37 1 33 30 00 48 21 870 00 155 45

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HYDROGRAPH AT STATION

TOTAL RAINFALL - 237 92, TOTAL LOSS - 81 34, TOTAL EXCESS - 156.58

PEAK FLOW (CU M/S)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	149 50-HR

000133

48 14 50 (CU M/S) 35 14 5 2
 (MM) 94 351 149 207 154 844 155 365
 (1000 CU M) 751 1188 1233 1237

CUMULATIVE AREA - 7 96 SQ KM

56 RO OUTPUT CONTROL VARIABLES
 IPRINT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

57 BA SUBBASIN CHARACTERISTICS
 TAREA 8 33 SUBBASIN AREA

PRECIPITATION DATA

10 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
 HYDRO-35 TP-40 TP-49
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
 26 00 49 00 98 70 120 00 138.00 190.00 215 00 260 50 .00 .00 .00 .00

STORM AREA - 220.00

11 LS SCS LOSS RATE
 STRL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STRL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

58 UK KINEMATIC WAVE
 OVERLAND-FLOW ELEMENT NO 1
 L 820. OVERLAND FLOW LENGTH
 S 0130 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 49.4 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

59 UK OVERLAND-FLOW ELEMENT NO 2
 L 840 OVERLAND FLOW LENGTH
 S 0130 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 50.6 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

60 RK KINEMATIC WAVE
 MAIN CHANNEL
 L 5000. CHANNEL LENGTH
 S 0030 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 8 33 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD .00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 DXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ YES ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	.29	1 67	25.91	164.05	29.75	838 49	155 54	.11
PLANE2	.29	1 67	26 53	168.06	30 20	843 05	155 46	.11
MAIN	.22	1 33	13 68	1667 21	471 57	918 58	154 98	2.05

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 1225E+05 EXCESS- 1301E+04 OUTFLOW- 1352E+05 BASIN STORAGE- 2516E+01 PERCENT ERROR- .3

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 22 1 33 30.00 469 10 900 00 155 00

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HYDROGRAPH AT STATION

TOTAL RAINFALL - 237 92, TOTAL LOSS - 81.34, TOTAL EXCESS - 156 58

PEAK FLOW TIME MAXIMUM AVERAGE FLOW
 (CU M/S) (HR) 6-HR 24-HR 72-HR 149 50-HR
 469 15 00 (CU M/S) 356 149 52 25.
 (MM) 87 774 147.122 154.220 154.916
 (1000 CU M) 7681. 12875. 13496. 13557.
 CUMULATIVE AREA - 87 51 SQ KM

63 RO OUTPUT CONTROL VARIABLES
 IPRINT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

64 BA SUBBASIN CHARACTERISTICS
 TAREA 19 41 SUBBASIN AREA

PRECIPITATION DATA

000134

10 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
 HYDRO-35 TP-40 IP-49
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
 26 00 49 00 98 70 120 00 138 00 190 00 215.00 260 50 00 .00 00 00

STORM AREA - 220.00

11 LS SCS LOSS RATE
 STRTL 17 85 INITIAL ABSTRACTION
 CRVWBR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STRTL 17 85 INITIAL ABSTRACTION
 CRVWBR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

65 UK KINEMATIC WAVE
 OVERLAND-FLOW ELEMENT NO. 1
 L 1340 OVERLAND FLOW LENGTH
 S .0030 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 48 8 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

66 UK OVERLAND-FLOW ELEMENT NO. 2
 L 1400 OVERLAND FLOW LENGTH
 S .0030 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 51 2 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

67 RK KINEMATIC WAVE
 MAIN CHANNEL
 L 7100. CHANNEL LENGTH
 S 0080 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 19.41 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD .80 BOTTOM WIDTH OR DIAMETER
 Z 40.00 SIDE SLOPE
 NDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ NO ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	14	1 67	24 73	89 36	33.71	1067 40	155 41	.06
PLANE2	14	1 67	25 67	93.36	34.30	1083.55	155 32	.06
MAIN	37	1 33	20 54	2367 44	67 00	1117 13	155 33	1.93

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 0000E+00 EXCESS- 3031E+04 OUTFLOW- 3005E+04 BASIN STORAGE- 1471E+02 PERCENT ERROR- .4

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 37 1 33 30.00 66 92 1110 00 155 34

HYDROGRAPH AT STATION

TOTAL RAINFALL - 237 92, TOTAL LOSS - 81.34, TOTAL EXCESS - 156 58

PEAK FLOW (CU M/S)	TIME (HR)	6-HR (CU M/S)	24-HR (1000 CU M)	72-HR (1000 CU M)	149 50-HR (1000 CU M)
67	18 50	60	1383	2701.	3014.
		(MM) 67 132	139 177	153 575	155 255

CUMULATIVE AREA - 19 41 SQ KM

70 RO OUTPUT CONTROL VARIABLES
 IPRINT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

71 BA SUBBASIN CHARACTERISTICS
 TAREA 15 48 SUBBASIN AREA

PRECIPITATION DATA

10 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
 HYDRO-35 TP-40 IP-49
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
 26 00 49 00 98 70 120 00 138.00 190 00 215 00 260.50 00 .00 00 00

STORM AREA - 220 00

11 LS SCS LOSS RATE
 STRTL 17 85 INITIAL ABSTRACTION
 CRVWBR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STRTL 17 85 INITIAL ABSTRACTION
 CRVWBR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

KINEMATIC WAVE

000135

72 UK OVERLAND-FLOW ELEMENT NO 1
 L 440 OVERLAND FLOW LENGTH
 S 0200 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 24 3 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

73 UK OVERLAND-FLOW ELEMENT NO 2
 L 1300 OVERLAND FLOW LENGTH
 S 0200 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 75 7 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

74 RK KINEMATIC WAVE
 MAIN CHANNEL
 L 0500 CHANNEL LENGTH
 S .0030 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 15.48 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 DXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ YES ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	N	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	35	1 67	13 60	88 03	38 55	774.81	156.13	.11
PLANE2	35	1 67	27 48	230 08	73 98	860 22	155.54	14
MAIN	22	1 33	30 00	2834 26	128.88	939 09	154 84	1 53

CONTINUITY SUMMARY (1000 CU-M)-INFLOW= 3003E+04 EXCESS= 2417E+04 OUTFLOW= .5384E+04 BASIN STORAGE= 4333E+01 PERCENT ERROR= 6

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 22 1 33 30 00 128 04 930.00 154 82

HYDROGRAPH AT STATION

TOTAL RAINFALL - 237 92, TOTAL LOSS - 81.34, TOTAL EXCESS - 156 58

PEAK FLOW (CU M/S)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	149.50-HR
+ 128	15 50	111. (MM) (1000 CU M)	57 68 704 2397	21 141 258 4929	10 153 441 5354
					154.742 5399

CUMULATIVE AREA - 34 89 SQ KM

77 KO OUTPUT CONTROL VARIABLES
 IPRT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

78 BA SUBBASIN CHARACTERISTICS
 TAREA 12 89 SUBBASIN AREA

PRECIPITATION DATA

10 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
 HYDRO-35 TP-49
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
 26 00 49 00 98 70 120 00 138 00 190 00 215.00 260.50 00 .00 00 00
 STORM AREA = 220 00

11 LS SCS LOSS RATE
 STRTL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STRTL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

79 UK KINEMATIC WAVE
 OVERLAND-FLOW ELEMENT NO 1
 L 750 OVERLAND FLOW LENGTH
 S 0100 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 31 9 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

80 UK OVERLAND-FLOW ELEMENT NO. 2
 L 1590 OVERLAND FLOW LENGTH
 S 0100 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 68 1 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

81 RK KINEMATIC WAVE
 MAIN CHANNEL
 L 5500 CHANNEL LENGTH
 S 0080 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 12 89 CONTRIBUTING AREA

000136

SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 MDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ NO ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	25	1.67	26 08	150.05	29.21	846 31	155.45	09
PLANE2	.25	1.67	28 10	176.72	42 18	933 00	155 47	10
MAIN	37	1.33	16 15	1033.93	62 00	930.02	155 47	1 90

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 0000E+00 EXCESS- 2013E+04 OUTFLOW- 1997E+04 BASIN STORAGE- 42B9E+01 PERCENT ERROR- 5

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 37 1 33 30.00 62 00 930 00 155 55

HYDROGRAPH AT STATION

TOTAL RAINFALL - 237 92. TOTAL LOSS - 81 34. TOTAL EXCESS - 156 58

PEAK FLOW (CU M/S)	TIME (HR)	6-HR (CU M/S)	24-HR (MM)	72-HR (MM)	149 50-HR (MM)
63	15 50	51	85 498	22	8
		1102	146.652	154 652	155.469
			1890.	1993	2004

CUMULATIVE AREA - 12 89 SQ KM

86 KO

OUTPUT CONTROL VARIABLES

IPRINT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

87 BA

SUBBASIN CHARACTERISTICS

TAREA 5 28 SUBBASIN AREA

PRECIPITATION DATA

10 PH

HYDRO-35		DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM						TP-49			
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
26 00	49 00	98 70	120 00	138 00	190 00	215 00	260 50	00	.00	00	00

STORM AREA - 220 00

11 LS

SCE LOSS RATE

STRIAL 17.85 INITIAL ABSTRACTION
 CRVWBR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT

STRIAL 17 85 INITIAL ABSTRACTION
 CRVWBR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

88 UK

KINEMATIC WAVE

OVERLAND-FLOW ELEMENT NO. 1
 L 750 OVERLAND FLOW LENGTH
 S 0130 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 42 4 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

89 UK

OVERLAND-FLOW ELEMENT NO. 2

L 1010 OVERLAND FLOW LENGTH
 S 0130 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 57 6 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

90 RK

KINEMATIC WAVE

MAIN CHANNEL
 L 3000 CHANNEL LENGTH
 S 0030 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 5 28 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40.00 SIDE SLOPE
 MDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ YES ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	29	1.67	24 20	150.05	16.72	823.09	155 61	10
PLANE2	29	1.67	25 54	160 39	20 22	868.55	155 54	.11
MAIN	22	1.33	9.94	1000.33	215.29	947 21	154 82	1.68

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 22 1 33 30 00 214 68 930 00 154 74

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HYDROGRAPH AT STATION

TOTAL RAINFALL - 237.92, TOTAL LOSS - 81.34, TOTAL EXCESS - 156.58

PEAK FLOW (CU M/S)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	149.50-HR
215	15 50	179	88	31	15
	(CU M/S)	72.792	142.649	153.526	154.656
	(MM)	3862	7569.	8146	8206
	(1000 CU M)				

CUMULATIVE AREA - 53 06 SQ KM

93 KO OUTPUT CONTROL VARIABLES
 IPRNT 3 PRINT CONTROL
 IPLOT 8 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

94 BA SUBBASIN CHARACTERISTICS
 TAREA 14 94 SUBBASIN AREA

PRECIPITATION DATA

10 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
 HYDRO-35 TP-40 TP-49
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
 26.00 49 00 98 70 120 00 138 00 190 00 215 00 260 50 00 00 00 00

STORM AREA - 220.00

11 LS SCS LOSS RATE
 STRL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STRL 17.85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

95 UK KINEMATIC WAVE OVERLAND-FLOW ELEMENT NO 1
 L 1060. OVERLAND FLOW LENGTH
 S 0800 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 74 8 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

96 UK OVERLAND-FLOW ELEMENT NO 2
 L 630 OVERLAND FLOW LENGTH
 S 0800 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 25 2 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

97 RK KINEMATIC WAVE MAIN CHANNEL
 L 6000 CHANNEL LENGTH
 S 0800 SLOPE
 N .045 CHANNEL ROUGHNESS COEFFICIENT
 CA 14 94 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD .00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 DXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ NO ROUTE UPSTREAM HYDROGRAPH

COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	N	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
FLAME1	71	1 67	24 19	372 12	83.47	823.04	155.61	26
FLAME2	71	1 67	10 70	126.04	44.61	756.86	156.22	.20
MAIN	37	1 33	15 44	2000 66	110 03	824 51	155 77	2 18

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 37 1 33 30 00 108 76 810 00 155 79

*** **

HYDROGRAPH AT STATION

TOTAL RAINFALL - 237.92 TOTAL LOSS - 81.34, TOTAL EXCESS - 156.58

PEAK FLOW (CU M/S)	TIME (HR)	MAXIMUM AVERAGE FLOW		
		6-HR	24-HR	72-HR
				149.50-HR
	(CU M/S)			

000138

+ 109 13 50 71 26. 9 4
 (MM) 102 462 151 955 155 431 155 708
 (1000 CU M) 1531 2270 2322 2326
 CUMULATIVE AREA - 14 94 SQ KM

100 KO OUTPUT CONTROL VARIABLES
 IPRNT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

101 BA SUBBASIN CHARACTERISTICS
 TAREA 7 52 SUBBASIN AREA

PRECIPITATION DATA

10 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM
 HYDRO-35 TP-40
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
 26 00 49 00 98 70 120 00 138 00 190.00 215 00 260 50 00 00 00 00 .00
 STORM AREA - 220 00

11 LS SCS LOSS RATE
 STRTL 17 85 INITIAL ABSTRACTION
 CRVWR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA
 LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STRTL 17 85 INITIAL ABSTRACTION
 CRVWR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

102 UK KINEMATIC WAVE OVERLAND-FLOW ELEMENT NO 1
 L 870 OVERLAND FLOW LENGTH
 S .0300 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 24 3 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

103 UK OVERLAND-FLOW ELEMENT NO 2
 L 800 OVERLAND FLOW LENGTH
 S .0300 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 75 7 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

104 RK KINEMATIC WAVE MAIN CHANNEL
 L 4500 CHANNEL LENGTH
 S .0060 SLOPE
 N .045 CHANNEL ROUGHNESS COEFFICIENT
 CA 7 52 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 NDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ YES ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	N	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	43	1 67	19 85	174 06	15 34	803.72	155 80	.15
PLANE2	43	1 67	18 57	160 85	49 48	791 02	155.86	.14
MAIN	32	1.33	12.13	1500 49	166 63	839 64	155.70	2 08

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 2318E+04 EXCESS- 1174E+04 OUTFLOW- 3485E+04 BASIN STORAGE- .7301E+00 PERCENT ERROR- 2

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 32 1 33 30 00 166.55 840 00 155 70

HYDROGRAPH AT STATION

TOTAL RAINFALL - 237 92, TOTAL LOSS - 81 34, TOTAL EXCESS - 156.58

PEAK FLOW TIME MAXIMUM AVERAGE FLOW
 (CU M/S) (HR) 6-HR 24-HR 72-HR 149.50-HR
 + 167 14 00 (CU M/S) 106 39 13 6
 (MM) 102 371 151 883 155 346 155 614
 (1000 CU M) 2299. 3411 3489 3495.
 CUMULATIVE AREA - 22 46 SQ KM

109 KO OUTPUT CONTROL VARIABLES
 IPRNT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

110 BA SUBBASIN CHARACTERISTICS
 TAREA 3 67 SUBBASIN AREA

PRECIPITATION DATA

000139

10 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35		TP-40						TP-49			
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
26 00	49 00	98 70	120 00	138 00	190 00	215 00	260 50	00	00	00	.00

STORM AREA - 220 00

11 LS SCS LOSS RATE

STRTL	17 85	INITIAL ABSTRACTION
CRVNR	74 00	CURVE NUMBER
RTIMP	00	PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT

STRTL	17 85	INITIAL ABSTRACTION
CRVNR	74 00	CURVE NUMBER
RTIMP	00	PERCENT IMPERVIOUS AREA

111 UK KINEMATIC WAVE OVERLAND-FLOW ELEMENT NO. 1

L	1160.	OVERLAND FLOW LENGTH
S	0130	SLOPE
N	400	ROUGHNESS COEFFICIENT
PA	63 4	PERCENT OF SUBBASIN
DXMIN	5	MINIMUM NUMBER OF DX INTERVALS

112 UK OVERLAND-FLOW ELEMENT NO. 2

L	670	OVERLAND FLOW LENGTH
S	0130	SLOPE
N	.400	ROUGHNESS COEFFICIENT
PA	36.6	PERCENT OF SUBBASIN
DXMIN	5	MINIMUM NUMBER OF DX INTERVALS

113 RK KINEMATIC WAVE MAIN CHANNEL

L	2800	CHANNEL LENGTH
S	0030	SLOPE
N	045	CHANNEL ROUGHNESS COEFFICIENT
CA	3 67	CONTRIBUTING AREA
SHAPE	TRAP	CHANNEL SHAPE
WD	00	BOTTOM WIDTH OR DIAMETER
Z	40 00	SIDE SLOPE
NDXMIN	2	MINIMUM NUMBER OF DX INTERVALS
RUPSTQ	YES	ROUTE UPSTREAM HYDROGRAPH

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	29	1 67	20 36	193 40	14 47	868.08	155 47	.11
PLANE2	29	1 67	22 22	134 04	10 49	805.37	155.75	10
MAIN	22	1 33	4 73	666 89	835 05	908.48	154 86	2.35

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 2517E+05 EXCESS- .5730E+03 OUTFLOW- 2573E+05 BASIN STORAGE- 1713E+01 PERCENT ERROR- 0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	22	1 33	30 00	828 06	900.00	154 89
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HYDROGRAPH AT STATION

TOTAL RAINFALL - 297.92, TOTAL LOSS - 81 34, TOTAL EXCESS - 156 58

PEAK FLOW (CU M/S)	TIME (HR)	MAXIMUM AVERAGE FLOW		
		6-HR	24-HR	72-HR
829	15 00	644	282	99
		(MM) 83.461	146 051	154 028
		(1000 CU M) 13913	24347	25676

CUMULATIVE AREA - 166 70 SQ KM

116 KO OUTPUT CONTROL VARIABLES

IPRNT	3	PRINT CONTROL
IFLOT	0	FLOT CONTROL
QSCAL	0	HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

117 BA SUBBASIN CHARACTERISTICS

TAREA	6 44	SUBBASIN AREA
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PRECIPITATION DATA

10 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35		TP-40						TP-49			
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
26 00	49.00	98 70	120 00	138 00	190.00	215.00	260 50	00	00	.00	.00

STORM AREA - 220 00

11 LS SCS LOSS RATE

STRTL	17 85	INITIAL ABSTRACTION
CRVNR	74 00	CURVE NUMBER
RTIMP	00	PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT

STRTL	17 85	INITIAL ABSTRACTION
CRVNR	74 00	CURVE NUMBER
RTIMP	00	PERCENT IMPERVIOUS AREA

118 UK KINEMATIC WAVE OVERLAND-FLOW ELEMENT NO. 1

000140

119 UK OVERLAND-FLOW ELEMENT NO 2

L	490	OVERLAND FLOW LENGTH
S	0500	SLOPE
N	400	ROUGHNESS COEFFICIENT
PA	30 5	PERCENT OF SUBBASIN
DXMIN	5	MINIMUM NUMBER OF DX INTERVALS

119 UK OVERLAND-FLOW ELEMENT NO 2

L	1120	OVERLAND FLOW LENGTH
S	0500	SLOPE
N	400	ROUGHNESS COEFFICIENT
PA	69 5	PERCENT OF SUBBASIN
DXMIN	5	MINIMUM NUMBER OF DX INTERVALS

120 RK KINEMATIC WAVE MAIN CHANNEL

L	4000	CHANNEL LENGTH
S	0100	SLOPE
N	045	CHANNEL ROUGHNESS COEFFICIENT
CA	6 44	CONTRIBUTING AREA
SHAPE	TRAP	CHANNEL SHAPE
WD	00	BOTTOM WIDTH OR DIAMETER
Z	40 00	SIDE SLOPE
MDXMIN	2	MINIMUM NUMBER OF DX INTERVALS
RUPSTQ	NO	ROUTE UPSTREAM HYDROGRAPH

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	56	1.67	10 43	98.03	23 52	754 98	156 22	16
PLANE2	56	1 67	19 66	224 07	37.62	803 29	155 80	19
MAIN	41	1 33	11 18	1333 77	55 62	803 01	155 96	2 00

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 0000E+00 EXCESS- 1006E+04 OUTFLOW- 1001E+04 BASIN STORAGE- 3996E+00 PERCENT ERROR- 4

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	41	1 33	30 00	54 84	810 00	156 03
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HYDROGRAPH AT STATION

TOTAL RAINFALL - 237 92, TOTAL LOSS - 81 34, TOTAL EXCESS - 156 58

PEAK FLOW (CU M/S)	TIME (HR)	6-HR (CU M/S)	24-HR (1000 CU M)	72-HR (1000 CU M)	149 50-HR (1000 CU M)
55	13 50	32	11	4	2
		106 683	153 203	155 759	155 944
		687	987	1003	1004

CUMULATIVE AREA - 6 44 SQ KM

123 KO OUTPUT CONTROL VARIABLES

IPRNT	3	PRINT CONTROL
IPLOT	0	PLOT CONTROL
QSCAL	0	HYDROGRAPH PLOT SCALE

SUBBASIN RUMOFF DATA

124 BA SUBBASIN CHARACTERISTICS

TAREA	8 85	SUBBASIN AREA
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PRECIPITATION DATA

10 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

	HYDRO-35								IP-40				TP-49
	5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY	
	26.00	49 00	98 70	120.00	138 00	190 00	215 00	260 50	00	00	00	00	00

STORM AREA - 220.00

11 LS SCS LOSS RATE

STRTL	17 85	INITIAL ABSTRACTION
CRVNR	74 00	CURVE NUMBER
RTIMP	00	PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT

STRTL	17 85	INITIAL ABSTRACTION
CRVNR	74 00	CURVE NUMBER
RTIMP	00	PERCENT IMPERVIOUS AREA

125 UR KINEMATIC WAVE OVERLAND-FLOW ELEMENT NO 1

L	910	OVERLAND FLOW LENGTH
S	0300	SLOPE
N	400	ROUGHNESS COEFFICIENT
PA	51 5	PERCENT OF SUBBASIN
DXMIN	5	MINIMUM NUMBER OF DX INTERVALS

126 UK OVERLAND-FLOW ELEMENT NO. 2

L	840	OVERLAND FLOW LENGTH
S	0300	SLOPE
N	400	ROUGHNESS COEFFICIENT
PA	48 5	PERCENT OF SUBBASIN
DXMIN	5	MINIMUM NUMBER OF DX INTERVALS

127 RK KINEMATIC WAVE MAIN CHANNEL

L	5000	CHANNEL LENGTH
S	0000	SLOPE
N	045	CHANNEL ROUGHNESS COEFFICIENT
CA	8.85	CONTRIBUTING AREA
SHAPE	TRAP	CHANNEL SHAPE

000141

WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 NDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ YES ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	43	1 67	20.51	182 06	37 32	810 21	155 77	15
PLANE2	43	1 67	19 59	172.06	36.17	800 81	155.77	15
MAIN	37	1 33	12 82	1667 21	124 70	827 82	156 84	2 19

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 1001E+04 EXCESS- 1382E+04 OUTFLOW- 2378E+04 BASIN STORAGE- .7360E+00 PERCENT ERROR- 2

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 37 1 33 30 00 122 59 840 00 155 97

HYDROGRAPH AT STATION

TOTAL RAINFALL - 237 92, TOTAL LOSS - 81 34, TOTAL EXCESS - 156.58

PEAK FLOW (CU M/S)	TIME (HR)	6-HR (CU M/S)	24-HR MAXIMUM AVERAGE FLOW	72-HR	149 50-HR
123	14 00	74	27	9	4
		(MM) 104 708	152 648	155 664	155 890
		(1000 CU M) 1601	2334.	2380	2384

CUMULATIVE AREA - 15 29 SQ KM

130 RO OUTPUT CONTROL VARIABLES
 IPRINT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

131 BA SUBBASIN CHARACTERISTICS
 IAREA 19 60 SUBBASIN AREA

PRECIPITATION DATA

10 PH DEPTHS FOR 0-PERCENT HYPOTHETICAL STORM

HYDRO-35	TP-40	TP-49									
5-MIN	15-MIN	60-MIN	2-HR	3-HR	6-HR	12-HR	24-HR	2-DAY	4-DAY	7-DAY	10-DAY
26 00	49 00	98 70	120 00	138 00	190.00	215.00	260 50	.00	00	00	00

STORM AREA - 220 00

11 LS SCS LOSS RATE
 STIPL 17 85 INITIAL ABSTRACTION
 CRVBR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STIPL 17.05 INITIAL ABSTRACTION
 CRVBR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

KINEMATIC WAVE

132 UK OVERLAND-FLOW ELEMENT NO. 1
 L 780 OVERLAND FLOW LENGTH
 S 0130 SLOPE
 N .400 ROUGHNESS COEFFICIENT
 PA 37 9 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

133 UK OVERLAND-FLOW ELEMENT NO. 2
 L 1280 OVERLAND FLOW LENGTH
 S 0130 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 62.1 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

KINEMATIC WAVE

134 RK MAIN CHANNEL
 L 9500 CHANNEL LENGTH
 S 0030 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 19 60 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD .00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 NDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ NO ROUTE UPSTREAM HYDROGRAPH

 COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	29	1 67	24.95	156.05	54.65	829.62	155 59	10
PLANE2	29	1 67	26.48	182 92	72 92	884 45	155.48	12
MAIN	22	1 33	26.81	2375 78	110 57	916.79	155 40	1.48

000142

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 22 1 33 30 00 109 94 930 00 155 33

HYDROGRAPH AT STATION

TOTAL RAINFALL - 237 97, TOTAL LOSS - 81 34, TOTAL EXCESS - 156 50

PEAK FLOW (CU M/S)	TIME (HR)	6-HR (CU M/S)	24-HR (1000 CU M)	72-HR (1000 CU M)	149.50-HR (1000 CU M)
110	15 50	81	34	12	6
		89 545	147 987	154 652	155 253
		1755	2901	3031	3043

CUMULATIVE AREA - 19 60 SQ KM

139 KO OUTPUT CONTROL VARIABLES
 IPRMT 1 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0 HYDROGRAPH PLOT SCALE

SUBBASIN RUNOFF DATA

140 BA SUBBASIN CHARACTERISTICS
 TAREA 4 11 SUBBASIN AREA

PRECIPITATION DATA

10 PH DEPTH FOR 0-PERCENT HYPOTHETICAL STORM
 HYDRO-35 TP-40 TP-49
 5-MIN 15-MIN 60-MIN 2-HR 3-HR 6-HR 12-HR 24-HR 2-DAY 4-DAY 7-DAY 10-DAY
 26 00 49 00 98 70 120 00 138 00 190 00 215 00 260 50 00 00 00 00
 STORM AREA - 220 00

11 LS SCS LOSS RATE
 STRIL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

LOSS RATE VARIABLES FOR SECOND OVERLAND FLOW ELEMENT
 STRIL 17 85 INITIAL ABSTRACTION
 CRVNR 74 00 CURVE NUMBER
 RTIMP 00 PERCENT IMPERVIOUS AREA

141 UK KINEMATIC WAVE OVERLAND-FLOW ELEMENT NO. 1
 L 450 OVERLAND FLOW LENGTH
 S 0100 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 32 6 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

142 UK KINEMATIC WAVE OVERLAND-FLOW ELEMENT NO. 2
 L 920 OVERLAND FLOW LENGTH
 S 0100 SLOPE
 N 400 ROUGHNESS COEFFICIENT
 PA 67 3 PERCENT OF SUBBASIN
 DXMIN 5 MINIMUM NUMBER OF DX INTERVALS

143 RK KINEMATIC WAVE MAIN CHANNEL
 L 3000 CHANNEL LENGTH
 S .0030 SLOPE
 N 045 CHANNEL ROUGHNESS COEFFICIENT
 CA 4.11 CONTRIBUTING AREA
 SHAPE TRAP CHANNEL SHAPE
 WD 00 BOTTOM WIDTH OR DIAMETER
 Z 40 00 SIDE SLOPE
 NDXMIN 2 MINIMUM NUMBER OF DX INTERVALS
 RUPSTQ YES ROUTE UPSTREAM HYDROGRAPH

COMPUTED KINEMATIC PARAMETERS
 VARIABLE TIME STEP
 (DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (M)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	MAXIMUM CELERITY (MPS)
PLANE1	.25	1.67	18 22	90 03	11.78	787 35	155 88	08
PLANE2	.25	1 67	26 28	153.38	10.02	875 94	155 59	.10
MAIN	.22	1 33	6 82	1000.33	1040 74	912.92	154 81	2 49

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN 22 1 33 30 00 1041 42 930.00 154 74

HYDROGRAPH AT STATION

DA	MON	HR:MM	ORD	RAIN	LOSS	EXCESS	COMP Q	DA	MON	HR:MM	ORD	RAIN	LOSS	EXCESS	COMP Q
15	APR	1500	1	00	00	00	0.	18	APR	1800	151	00	.00	00	2.
15	APR	1530	2	1 64	1 64	00	0.	18	APR	1830	152	00	.00	00	2
15	APR	1600	3	1 69	1 69	00	0	18	APR	1900	153	00	00	00	2

15 APR 1630	4	1 74	1 74	00	0	*	18 APR 1930	154	00	00	00	2
15 APR 1700	5	1 80	1 80	00	0	*	18 APR 2000	155	00	00	00	2
15 APR 1730	6	1 86	1 86	00	0	*	18 APR 2030	156	00	00	00	2
15 APR 1800	7	1 93	1 93	00	0	*	18 APR 2100	157	00	00	00	2
15 APR 1830	8	2 00	2 00	00	0	*	18 APR 2130	158	00	00	00	2
15 APR 1900	9	2 08	2 08	00	0	*	18 APR 2200	159	00	00	00	2
15 APR 1930	10	2 16	2 16	00	0	*	18 APR 2230	160	00	00	00	2
15 APR 2000	11	2 26	2 24	02	0.	*	18 APR 2300	161	00	00	00	2.
15 APR 2030	12	2 37	2 24	13	0	*	18 APR 2330	162	00	00	00	2.
15 APR 2100	13	2 49	2 24	25	0	*	19 APR 0000	163	00	00	00	2.
15 APR 2130	14	2 03	1 74	29	0	*	19 APR 0030	164	00	00	00	2.
15 APR 2200	15	2 17	1 78	39	0	*	19 APR 0100	165	00	00	00	2
15 APR 2230	16	2 33	1 83	50	0	*	19 APR 0130	166	00	00	00	1
15 APR 2300	17	2 53	1 89	64	0	*	19 APR 0200	167	00	00	00	1
15 APR 2330	18	2 77	1 97	80	0	*	19 APR 0230	168	00	00	00	1
16 APR 0000	19	3 07	2 07	1 00	0	*	19 APR 0300	169	00	00	00	1
16 APR 0030	20	7 30	4 53	2 85	0	*	19 APR 0330	170	00	00	00	1
16 APR 0100	21	8 05	4 34	3 72	1	*	19 APR 0400	171	00	00	00	1
16 APR 0130	22	8 96	4 22	4 75	2	*	19 APR 0430	172	00	00	00	1
16 APR 0200	23	8 97	3 69	5 27	5	*	19 APR 0500	173	00	00	00	1
16 APR 0230	24	11 07	3 97	7 10	12	*	19 APR 0530	174	00	00	00	1
16 APR 0300	25	25 99	7 42	18.57	33	*	19 APR 0600	175	00	00	00	1.
16 APR 0330	26	39 44	7 92	31 52	91	*	19 APR 0630	176	00	00	00	1
16 APR 0400	27	13 32	2 00	11 25	245.	*	19 APR 0700	177	00	00	00	1
16 APR 0430	28	9 97	1 41	8 57	501	*	19 APR 0730	178	00	00	00	1
16 APR 0500	29	9 57	1 24	8 32	763	*	19 APR 0800	179	00	00	00	1
16 APR 0530	30	8 47	1 03	7 44	945	*	19 APR 0830	180	00	00	00	1
16 APR 0600	31	7 69	88	6 82	1031	*	19 APR 0900	181	00	00	00	1
16 APR 0630	32	3 25	36	2 90	1042	*	19 APR 0930	182	00	00	00	1
16 APR 0700	33	2 91	31	2 60	1003	*	19 APR 1000	183	00	00	00	1
16 APR 0730	34	2 64	28	2 37	933	*	19 APR 1030	184	00	00	00	1
16 APR 0800	35	2 43	25	2 18	847	*	19 APR 1100	185	00	00	00	1
16 APR 0830	36	2 25	23	2 02	759	*	19 APR 1130	186	00	00	00	1
16 APR 0900	37	2 10	21	1 89	677	*	19 APR 1200	187	00	00	00	1.
16 APR 0930	38	2 56	25	2 31	606	*	19 APR 1230	188	00	00	00	1.
16 APR 1000	39	2 43	23	2 20	540	*	19 APR 1300	189	00	00	00	1
16 APR 1030	40	2 31	22	2 09	501	*	19 APR 1330	190	00	00	00	1
16 APR 1100	41	2 21	21	2 00	462	*	19 APR 1400	191	00	00	00	1
16 APR 1130	42	2 12	20	1 92	429	*	19 APR 1430	192	00	00	00	1
16 APR 1200	43	2 04	19	1 85	401	*	19 APR 1500	193	00	00	00	1
16 APR 1230	44	1 96	18	1 78	375	*	19 APR 1530	194	00	00	00	1
16 APR 1300	45	1 89	17	1 72	351	*	19 APR 1600	195	00	00	00	1
16 APR 1330	46	1 83	16	1 67	330	*	19 APR 1630	196	00	00	00	1
16 APR 1400	47	1 77	15	1 62	312	*	19 APR 1700	197	00	00	00	1
16 APR 1430	48	1 72	15	1 57	295	*	19 APR 1730	198	00	00	00	1
16 APR 1500	49	1.67	14	1 53	280	*	19 APR 1800	199	00	00	00	1
16 APR 1530	50	00	00	00	266.	*	19 APR 1830	200	00	00	00	1
16 APR 1600	51	00	00	00	252	*	19 APR 1900	201	00	00	00	1
16 APR 1630	52	00	00	00	237	*	19 APR 1930	202	00	00	00	1
16 APR 1700	53	00	00	00	221	*	19 APR 2000	203	00	00	00	1
16 APR 1730	54	00	00	00	205.	*	19 APR 2030	204	00	00	00	1
16 APR 1800	55	00	00	00	188	*	19 APR 2100	205	00	00	00	1
16 APR 1830	56	00	00	00	172	*	19 APR 2130	206	00	00	00	1
16 APR 1900	57	00	00	00	157	*	19 APR 2200	207	00	00	00	1
16 APR 1930	58	00	00	00	143.	*	19 APR 2230	208	00	00	00	1
16 APR 2000	59	00	00	00	131	*	19 APR 2300	209	00	00	00	1
16 APR 2030	60	00	00	00	119	*	19 APR 2330	210	00	00	00	1
16 APR 2100	61	00	00	00	109.	*	20 APR 0000	211	00	00	00	1
16 APR 2130	62	00	00	00	100	*	20 APR 0030	212	00	00	00	1
16 APR 2200	63	00	00	00	91	*	20 APR 0100	213	00	00	00	1
16 APR 2230	64	00	00	00	84	*	20 APR 0130	214	00	00	00	1
16 APR 2300	65	00	00	00	77	*	20 APR 0200	215	00	00	00	1.
16 APR 2330	66	00	00	00	71	*	20 APR 0230	216	00	00	00	1.
17 APR 0000	67	00	00	00	66	*	20 APR 0300	217	00	00	00	1
17 APR 0030	68	00	00	00	61	*	20 APR 0330	218	00	00	00	1
17 APR 0100	69	00	00	00	56	*	20 APR 0400	219	00	00	00	1
17 APR 0130	70	00	00	00	52.	*	20 APR 0430	220	00	00	00	1
17 APR 0200	71	00	00	00	49	*	20 APR 0500	221	00	00	00	1
17 APR 0230	72	00	00	00	46	*	20 APR 0530	222	00	00	00	1
17 APR 0300	73	00	00	00	42	*	20 APR 0600	223	00	00	00	1.
17 APR 0330	74	00	00	00	40.	*	20 APR 0630	224	00	00	00	1
17 APR 0400	75	00	00	00	37	*	20 APR 0700	225	00	00	00	1
17 APR 0430	76	00	00	00	35	*	20 APR 0730	226	00	00	00	1
17 APR 0500	77	00	00	00	33	*	20 APR 0800	227	00	00	00	1
17 APR 0530	78	00	00	00	31	*	20 APR 0830	228	00	00	00	1
17 APR 0600	79	00	00	00	29	*	20 APR 0900	229	00	00	00	1
17 APR 0630	80	00	00	00	27	*	20 APR 0930	230	00	00	00	1.
17 APR 0700	81	00	00	00	26	*	20 APR 1000	231	00	00	00	1
17 APR 0730	82	00	00	00	24.	*	20 APR 1030	232	00	00	00	1
17 APR 0800	83	00	00	00	23	*	20 APR 1100	233	00	00	00	0
17 APR 0830	84	00	00	00	22	*	20 APR 1130	234	00	00	00	0
17 APR 0900	85	00	00	00	20.	*	20 APR 1200	235	00	00	00	0
17 APR 0930	86	00	00	00	19.	*	20 APR 1230	236	00	00	00	0
17 APR 1000	87	00	00	00	18	*	20 APR 1300	237	00	00	00	0
17 APR 1030	88	00	00	00	17	*	20 APR 1330	238	00	00	00	0.
17 APR 1100	89	00	00	00	17	*	20 APR 1400	239	00	00	00	0.
17 APR 1130	90	00	00	00	16	*	20 APR 1430	240	00	00	00	0
17 APR 1200	91	00	00	00	15	*	20 APR 1500	241	00	00	00	0
17 APR 1230	92	00	00	00	14	*	20 APR 1530	242	00	00	00	0.
17 APR 1300	93	00	00	00	14.	*	20 APR 1600	243	00	00	00	0
17 APR 1330	94	00	00	00	13.	*	20 APR 1630	244	00	00	00	0
17 APR 1400	95	00	00	00	12	*	20 APR 1700	245	00	00	00	0
17 APR 1430	96	00	00	00	12	*	20 APR 1730	246	00	00	00	0
17 APR 1500	97	00	00	00	11	*	20 APR 1800	247	00	00	00	0.
17 APR 1530	98	00	00	00	11	*	20 APR 1830	248	00	00	00	0
17 APR 1600	99	00	00	00	10	*	20 APR 1900	249	00	00	00	0.
17 APR 1630	100	00	00	00	10.	*	20 APR 1930	250	00	00	00	0
17 APR 1700	101	00	00	00	10	*	20 APR 2000	251	00	00	00	0
17 APR 1730	102	00	00	00	9	*	20 APR 2030	252	00	00	00	0.
17 APR 1800	103	00	00	00	9	*	20 APR 2100	253	00	00	00	0
17 APR 1830	104	00	00	00	8	*	20 APR 2130	254	00	00	00	0
17 APR 1900	105	00	00	00	8	*	20 APR 2200	255	00	00	00	0
17 APR 1930	106	00	00	00	8	*	20 APR 2230	256	00	00	00	0
17 APR 2000	107	00	00	00	8	*	20 APR 2300	257	00	00	00	0
17 APR 2030	108	00	00	00	7	*	20 APR 2330	258	00	00	00	0
17 APR 2100	109	00	00	00	7	*	21 APR 0000	259	00	00	00	0.
17 APR 2130	110	00	00	00	7	*	21 APR 0030	260	00	00	00	0
17 APR 2200	111	00	00	00	7	*	21 APR 0100	261	00	00	00	0
17 APR 2230	112	00	00	00	6	*	21 APR 0130	262	00	00	00	0.
17 APR 2300	113	00	00	00	6.	*	21 APR 0200	263	00	00	00	0.
17 APR 2330	114	00	00	00	6	*	21 APR 0230	264	00	00	00	0
18 APR 0000	115	00	00	00	6	*	21 APR 0300	265	00	00	00	0.
18 APR 0030	116	00	00	00	5	*	21 APR 0330	266	00	00	00	0

18 APR 0100	117	00	00	00	5	*	21 APR 0400	267	00	.00	00	0
18 APR 0130	118	00	00	00	5	*	21 APR 0430	268	00	.00	00	0
18 APR 0200	119	00	00	00	5	*	21 APR 0500	269	00	.00	00	0
18 APR 0230	120	00	00	00	5	*	21 APR 0530	270	00	.00	00	0
18 APR 0300	121	00	00	00	5	*	21 APR 0600	271	00	.00	00	0
18 APR 0330	122	00	00	00	5	*	21 APR 0630	272	00	.00	00	0
18 APR 0400	123	00	00	00	4	*	21 APR 0700	273	00	.00	00	0
18 APR 0430	124	00	00	00	4	*	21 APR 0730	274	00	.00	00	0
18 APR 0500	125	00	00	00	4	*	21 APR 0800	275	00	.00	00	0
18 APR 0530	126	00	00	00	4	*	21 APR 0830	276	00	.00	00	0
18 APR 0600	127	00	00	00	4	*	21 APR 0900	277	00	.00	00	0
18 APR 0630	128	00	00	00	4	*	21 APR 0930	278	00	.00	00	0
18 APR 0700	129	00	00	00	4	*	21 APR 1000	279	00	.00	00	0
18 APR 0730	130	00	00	00	4	*	21 APR 1030	280	00	.00	00	0
18 APR 0800	131	00	00	00	3	*	21 APR 1100	281	00	.00	00	0
18 APR 0830	132	00	00	00	3	*	21 APR 1130	282	00	.00	00	0
18 APR 0900	133	00	00	00	3	*	21 APR 1200	283	00	.00	00	0
18 APR 0930	134	00	00	00	3	*	21 APR 1230	284	00	.00	00	0
18 APR 1000	135	00	00	00	3	*	21 APR 1300	285	00	.00	00	0
18 APR 1030	136	00	00	00	3	*	21 APR 1330	286	00	.00	00	0
18 APR 1100	137	00	00	00	3	*	21 APR 1400	287	00	.00	00	0
18 APR 1130	138	00	00	00	3	*	21 APR 1430	288	00	.00	00	0
18 APR 1200	139	00	00	00	3	*	21 APR 1500	289	00	.00	00	0
18 APR 1230	140	00	00	00	3	*	21 APR 1530	290	00	.00	00	0
18 APR 1300	141	00	00	00	3	*	21 APR 1600	291	00	.00	00	0
18 APR 1330	142	00	00	00	3	*	21 APR 1630	292	00	.00	00	0
18 APR 1400	143	00	00	00	3	*	21 APR 1700	293	00	.00	00	0
18 APR 1430	144	00	00	00	2	*	21 APR 1730	294	00	.00	00	0
18 APR 1500	145	00	00	00	2	*	21 APR 1800	295	00	.00	00	0
18 APR 1530	146	00	00	00	2	*	21 APR 1830	296	00	.00	00	0
18 APR 1600	147	00	00	00	2	*	21 APR 1900	297	00	.00	00	0
18 APR 1630	148	00	00	00	2	*	21 APR 1930	298	00	.00	00	0
18 APR 1700	149	00	00	00	2	*	21 APR 2000	299	00	.00	00	0
18 APR 1730	150	00	00	00	2	*	21 APR 2030	300	00	.00	00	0

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TOTAL RAINFALL - 237 92. TOTAL LOSS - 81 50. TOTAL EXCESS - 156 42

PEAK FLOW	TIME	6-HR	MAXIMUM AVERAGE FLOW	72-HR	149 50-HR
(CU M/S)	(HR)		24-HR		
1842	15 50	805	348	122	59
		(MM) 84 493	146 320	153 936	154 664
		(1000 CU M) 17380.	30098	31665	31814

CUMULATIVE AREA - 205 70 SQ KM

145 RO OUTPUT CONTROL VARIABLES

IPRMT 1 PRINT CONTROL

IPLST 0 PLOT CONTROL

QSCAL 0 HYDROGRAPH PLOT SCALE

HYDROGRAPH ROUTING DATA

146 RS STORAGE ROUTING

MSSTPS 1 NUMBER OF SUBREACHES

ITYP STOR TYPE OF INITIAL CONDITION

RSVVIC 30000.00 INITIAL CONDITION

X -1 00 WORKING R AND D COEFFICIENT

147 SV STORAGE 0 18043 0 22516 0 28092 3 33668.4 39244 4

148 SF ELEVATION 97 00 98 00 99 00 100 00 101 00 102 00

149 SS SPILLWAY

CREL 100 50 SPILLWAY CREST ELEVATION

SPWID 130.00 SPILLWAY WIDTH

COEW 1 40 WEIR COEFFICIENT

EXPW 1 50 EXPONENT OF HEAD

COMPUTED OUTFLOW-ELEVATION DATA

OUTFLOW	00	00	06	46	1 55	3.67	7 17	12 38	19 66	29 35
ELEVATION	97.00	100 50	100.50	100.52	100 54	100.57	100 62	100 67	100 73	100 80
OUTFLOW	41 79	57 33	76 31	99 07	125 96	157.32	193.49	234.83	281 67	334.36
ELEVATION	100 88	100 96	101 06	101 17	101.28	101 41	101.54	101 69	101 84	102 00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	00	18043.00	22516 00	28092 30	30880.35	30983 60	31112.67	31293 39	31525 71	31809 69
OUTFLOW	00	.00	00	00	00	.46	1.55	3 67	7 17	12 38
ELEVATION	97.00	98 00	99.00	100.00	100 50	100.52	100 54	100 57	100 62	100 67
STORAGE	32145.30	32532 52	32971 39	33461.06	33668 40	34004.01	34597 72	35243.11	35940.11	36688.72
OUTFLOW	19 66	29 35	41.79	57 33	64 35	76.31	99 07	125 96	157 32	193 49
ELEVATION	100 73	100 80	100.88	100 96	101 00	101 06	101.17	101 28	101.41	101 54
STORAGE	37489 01	38340 86	39244 40							
OUTFLOW	234 83	281 67	334 36							
ELEVATION	101 69	101 84	102 00							

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HYDROGRAPH AT STATION BARR

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DA	MON	HR	MIN	ORD	OUTFLOW	STORAGE	STAGE	DA	MON	HR	MIN	ORD	OUTFLOW	STORAGE	STAGE
15 APR	1500	1	0	30000.0	100.3	17 APR	1700	101	72	33879 3	101.0	19 APR	1900	201	9.
15 APR	1530	2	0	30000.0	100.3	17 APR	1730	102	70.	33824.1	101.0	19 APR	1930	202	9
15 APR	1600	3	0	30000 0	100.3	17 APR	1800	103	68.	33770.2	101 0	19 APR	2000	203	9
15 APR	1630	4	0	30000 0	100.3	17 APR	1830	104	66.	33717.6	101.0	19 APR	2030	204	9.
15 APR	1700	5	0	30000 0	100.3	17 APR	1900	105	64.	33666 4	101 0	19 APR	2100	205	9
15 APR	1730	6	0	30000 0	100 3	17 APR	1930	106	63	33616.5	101.0	19 APR	2130	206	9
15 APR	1800	7	0	30000 0	100 3	17 APR	2000	107	61	33567 9	101 0	19 APR	2200	207	9

15 APR 1830	8	0	30000 0	100 3	17 APR 2030	108	59	33520.5	101 0	19 APR 2230	208	8	31596.4	100 6
15 APR 1900	9	0	30000 0	100 3	17 APR 2100	109	58	33474.2	101 0	19 APR 2300	209	8	31589.5	100 6
15 APR 1930	10	0	30000 0	100 3	17 APR 2130	110	56	33429.1	101 0	19 APR 2330	210	8	31582.6	100 6
15 APR 2000	11	0	30000 0	100 3	17 APR 2200	111	55	33385.0	100 9	20 APR 0000	211	8	31575.9	100 6
15 APR 2030	12	0	30000 0	100 3	17 APR 2230	112	54	33342.0	100 9	20 APR 0030	212	8	31569.3	100 6
15 APR 2100	13	0	30000 0	100 3	17 APR 2300	113	52	33300.0	100 9	20 APR 0100	213	8	31562.8	100 6
15 APR 2130	14	0	30000 0	100 3	17 APR 2330	114	51	33259.0	100 9	20 APR 0130	214	8	31556.3	100 6
15 APR 2200	15	0	30000 0	100 3	18 APR 0000	115	50	33218.9	100 9	20 APR 0200	215	8	31550.0	100 6
15 APR 2230	16	0	30000 0	100 3	18 APR 0030	116	48	33179.8	100 9	20 APR 0230	216	7	31543.7	100 6
15 APR 2300	17	0	30000 1	100 3	18 APR 0100	117	47	33141.6	100 9	20 APR 0300	217	7	31537.0	100 6
15 APR 2330	18	0	30000 1	100 3	18 APR 0130	118	46	33104.4	100 9	20 APR 0330	218	7	31531.6	100 6
16 APR 0000	19	0	30000 3	100 3	18 APR 0200	119	45	33068.1	100 9	20 APR 0400	219	7	31525.6	100 6
16 APR 0030	20	0	30000 5	100 3	18 APR 0230	120	44	33032.6	100 9	20 APR 0430	220	7	31519.8	100 6
16 APR 0100	21	0	30001 0	100 3	18 APR 0300	121	43	32998.0	100 9	20 APR 0500	221	7	31514.0	100 6
16 APR 0130	22	0	30002 2	100 3	18 APR 0330	122	42	32964.2	100 9	20 APR 0530	222	7	31508.2	100 6
16 APR 0200	23	0	30005 2	100 3	18 APR 0400	123	41	32931.2	100 9	20 APR 0600	223	7	31502.6	100 6
16 APR 0230	24	0	30012 8	100 3	18 APR 0430	124	40	32898.9	100 9	20 APR 0630	224	7	31497.0	100 6
16 APR 0300	25	0	30033 0	100 3	18 APR 0500	125	39	32867.3	100 9	20 APR 0700	225	7	31491.4	100 6
16 APR 0330	26	0	30088 8	100 4	18 APR 0530	126	38	32836.4	100 9	20 APR 0730	226	7	31486.0	100 6
16 APR 0400	27	0	30239 9	100 4	18 APR 0600	127	37	32806.2	100 8	20 APR 0800	227	6	31480.6	100 6
16 APR 0430	28	0	30575 4	100 4	18 APR 0630	128	36	32776.6	100 8	20 APR 0830	228	6	31475.3	100 6
16 APR 0500	29	2	31143.2	100 5	18 APR 0700	129	35	32747.6	100 8	20 APR 0900	229	6	31470.0	100 6
16 APR 0530	30	14	31904.3	100 7	18 APR 0730	130	35	32719.3	100 8	20 APR 0930	230	6	31464.8	100 6
16 APR 0600	31	36	32770.6	100 8	18 APR 0800	131	34	32691.7	100 8	20 APR 1000	231	6	31459.7	100 6
16 APR 0630	32	64	33658.4	101 0	18 APR 0830	132	33	32664.6	100 8	20 APR 1030	232	6	31454.6	100 6
16 APR 0700	33	96	34507.0	101 2	18 APR 0900	133	32	32638.1	100 8	20 APR 1100	233	6	31449.6	100 6
16 APR 0730	34	128	35278.1	101 3	18 APR 0930	134	32	32612.2	100 8	20 APR 1130	234	6	31444.7	100 6
16 APR 0800	35	158	35950.9	101 4	18 APR 1000	135	31	32586.9	100 8	20 APR 1200	235	6	31439.8	100 6
16 APR 0830	36	185	36519.2	101 5	18 APR 1030	136	30	32562.2	100 8	20 APR 1230	236	6	31435.0	100 6
16 APR 0900	37	209	36987.9	101 6	18 APR 1100	137	30	32538.0	100 8	20 APR 1300	237	6	31430.2	100 6
16 APR 0930	38	229	37368.4	101 7	18 APR 1130	138	29	32514.3	100 8	20 APR 1330	238	6	31425.5	100 6
16 APR 1000	39	245	37674.9	101 7	18 APR 1200	139	28	32491.1	100 8	20 APR 1400	239	6	31420.9	100 6
16 APR 1030	40	259	37920.3	101 8	18 APR 1230	140	28	32468.3	100 8	20 APR 1430	240	6	31416.3	100 6
16 APR 1100	41	269	38116.0	101 8	18 APR 1300	141	27	32446.0	100 8	20 APR 1500	241	5	31411.7	100 6
16 APR 1130	42	278	38270.7	101 8	18 APR 1330	142	27	32424.1	100 8	20 APR 1530	242	5	31407.3	100 6
16 APR 1200	43	285	38390.9	101 8	18 APR 1400	143	26	32402.7	100 8	20 APR 1600	243	5	31403.0	100 6
16 APR 1230	44	290	38481.3	101 9	18 APR 1430	144	26	32381.6	100 8	20 APR 1630	244	5	31398.5	100 6
16 APR 1300	45	294	38545.3	101 9	18 APR 1500	145	25	32361.0	100 8	20 APR 1700	245	5	31394.2	100 6
16 APR 1330	46	296	38586.7	101 9	18 APR 1530	146	25	32340.8	100 8	20 APR 1730	246	5	31389.9	100 6
16 APR 1400	47	297	38608.7	101 9	18 APR 1600	147	24	32321.0	100 8	20 APR 1800	247	5	31385.7	100 6
16 APR 1430	48	298	38614.3	101 9	18 APR 1630	148	24	32301.6	100 8	20 APR 1830	248	5	31381.5	100 6
16 APR 1500	49	297	38605.8	101 9	18 APR 1700	149	23	32282.6	100 8	20 APR 1900	249	5	31377.4	100 6
16 APR 1530	50	296	38584.9	101 9	18 APR 1730	150	23	32263.9	100 7	20 APR 1930	250	5	31373.4	100 6
16 APR 1600	51	294	38552.7	101 9	18 APR 1800	151	22	32245.7	100 7	20 APR 2000	251	5	31369.4	100 6
16 APR 1630	52	291	38509.3	101 9	18 APR 1830	152	22	32227.7	100 7	20 APR 2030	252	5	31365.4	100 6
16 APR 1700	53	288	38454.8	101 9	18 APR 1900	153	21	32210.2	100 7	20 APR 2100	253	5	31361.5	100 6
16 APR 1730	54	284	38388.0	101 8	18 APR 1930	154	21	32193.0	100 7	20 APR 2130	254	5	31357.7	100 6
16 APR 1800	55	280	38311.8	101 8	18 APR 2000	155	20	32176.1	100 7	20 APR 2200	255	5	31353.8	100 6
16 APR 1830	56	275	38224.1	101 8	18 APR 2030	156	20	32159.5	100 7	20 APR 2230	256	5	31350.1	100 6
16 APR 1900	57	270	38127.3	101 8	18 APR 2100	157	20	32143.3	100 7	20 APR 2300	257	4	31346.4	100 6
16 APR 1930	58	264	38022.3	101 8	18 APR 2130	158	19	32127.4	100 7	20 APR 2330	258	4	31342.7	100 6
16 APR 2000	59	258	37918.8	101 8	18 APR 2200	159	19	32111.8	100 7	21 APR 0000	259	4	31339.1	100 6
16 APR 2030	60	252	37794.0	101 7	18 APR 2230	160	19	32096.5	100 7	21 APR 0030	260	4	31335.5	100 6
16 APR 2100	61	245	37673.4	101 7	18 APR 2300	161	18	32081.4	100 7	21 APR 0100	261	4	31332.0	100 6
16 APR 2130	62	238	37555.0	101 7	18 APR 2330	162	18	32066.5	100 7	21 APR 0130	262	4	31328.5	100 6
16 APR 2200	63	232	37424.8	101 7	19 APR 0000	163	18	32052.0	100 7	21 APR 0200	263	4	31325.0	100 6
16 APR 2230	64	225	37298.3	101 7	19 APR 0030	164	17	32037.6	100 7	21 APR 0230	264	4	31321.6	100 6
16 APR 2300	65	218	37171.3	101 6	19 APR 0100	165	17	32023.9	100 7	21 APR 0300	265	4	31318.3	100 6
16 APR 2330	66	212	37044.4	101 6	19 APR 0130	166	17	32009.7	100 7	21 APR 0330	266	4	31314.9	100 6
17 APR 0000	67	205	36918.2	101 6	19 APR 0200	167	16	31996.1	100 7	21 APR 0400	267	4	31311.7	100 6
17 APR 0030	68	199	36793.1	101 6	19 APR 0230	168	16	31982.8	100 7	21 APR 0430	268	4	31308.4	100 6
17 APR 0100	69	193	36669.7	101 5	19 APR 0300	169	16	31969.7	100 7	21 APR 0500	269	4	31305.2	100 6
17 APR 0130	70	187	36548.0	101 5	19 APR 0330	170	16	31956.8	100 7	21 APR 0530	270	4	31302.1	100 6
17 APR 0200	71	181	36428.2	101 5	19 APR 0400	171	15	31944.1	100 7	21 APR 0600	271	4	31299.0	100 6
17 APR 0230	72	175	36310.4	101 5	19 APR 0430	172	15	31931.7	100 7	21 APR 0630	272	4	31295.9	100 6
17 APR 0300	73	170	36194.8	101 5	19 APR 0500	173	15	31919.4	100 7	21 APR 0700	273	4	31292.8	100 6
17 APR 0330	74	164	36081.6	101 4	19 APR 0530	174	15	31907.4	100 7	21 APR 0730	274	4	31289.8	100 6
17 APR 0400	75	159	35970.7	101 4	19 APR 0600	175	14	31895.6	100 7	21 APR 0800	275	4	31286.8	100 6
17 APR 0430	76	154	35862.4	101 4	19 APR 0630	176	14	31884.0	100 7	21 APR 0830	276	4	31283.9	100 6
17 APR 0500	77	149	35756.4	101 4	19 APR 0700	177	14	31872.6	100 7	21 APR 0900	277	4	31281.0	100 6
17 APR 0530	78	144	35652.8	101 4	19 APR 0730	178	14	31861.4	100 7	21 APR 0930	278	3	31278.1	100 6
17 APR 0600	79	140	35551.6	101 3	19 APR 0800	179	13	31850.4	100 7	21 APR 1000	279	3	31275.2	100 6
17 APR 0630	80	135	35452.8	101 3	19 APR 0830	180	13	31839.6	100 7	21 APR 1030	280	3	31272.4	100 6
17 APR 0700	81	131	35356.5	101 3	19 APR 0900	181	13	31829.0	100 7	21 APR 1100	281	3	31269.5	100 6
17 APR 0730	82	127	35262.8	101 3	19 APR 0930	182	13	31818.6	100 7	21 APR 1130	282	3	31266.8	100 6
17 APR 0800	83	123	35171.5	101 3	19 APR 1000	183	12	31808.3	100 7	21 APR 1200	283	3	31264.0	100 6
17 APR 0830	84	119	35082.5	101 3	19 APR 1030	184	12	31798.2	100 7	21 APR 1230	284	3	31261.3	100 6
17 APR 0900	85	116	34995.7	101 2	19 APR 1100	185	12	31788.3	100 7	21 APR 1300	285	3	31258.6	100 6
17 APR 0930	86	112	34911.2	101 2	19 APR 1130	186	12	31778.5	100 7	21 APR 1330	286	3	31255.9	100 6
17 APR 1000	87	109	34828.8	101 2	19 APR 1200	187	12	31768.8	100 7	21 APR 1400	287	3	31253.2	100 6
17 APR 1030	88	105	34748.6	101 2	19 APR 1230	188	1							

CUMULATIVE AREA - 205 70 SQ KM

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RUNOFF SUMMARY, AVERAGE FLOW IN CUBIC METERS PER SECOND
AREA IN SQUARE KILOMETERS

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT		123 75	14 00	75 79	27 88	9 49	15 83		
HYDROGRAPH AT		132 52	15 00	94 23	37 97	13 18	22 10		
HYDROGRAPH AT		73 57	16 00	64 56	31 80	11 51	19 42		
HYDROGRAPH AT		80 98	15 50	58 47	23.81	8 28	13 87		
4 COMBINED AT	BACIA	379 44	15 00	288 21	121 36	42 46	71 22		
HYDROGRAPH AT		17 41	14 50	12 94	5 22	1 81	3 04		
HYDROGRAPH AT		48 24	14 50	34 77	13.75	4 76	7 96		
2 COMBINED AT	BACIA	426 72	15 00	322 53	135 11	47 22	79 18		
HYDROGRAPH AT		469 38	15 00	355 61	149 01	52 07	87 51		
HYDROGRAPH AT		66 96	18 50	60 33	31 27	11 50	19 41		
HYDROGRAPH AT		128 12	15 50	110 98	57 04	20 65	34 89		
HYDROGRAPH AT		62 84	15 50	51 02	21 88	7 69	12 89		
2 COMBINED AT	BACIA	190 96	15 50	159 63	78 83	28 34	47 78		
HYDROGRAPH AT		214 81	15 50	178 81	87 60	31.43	53 06		
HYDROGRAPH AT		108 83	13 50	70 87	26 28	8.96	14.94		
HYDROGRAPH AT		166 65	14 00	106.45	39 48	13 46	22 46		
3 COMBINED AT	BACIA	817 15	15 00	630 14	275 81	96 95	163 03		
HYDROGRAPH AT		828 55	15 00	644 11	281.79	99 06	166.70		
HYDROGRAPH AT		54 88	13 50	31.81	11 42	3 87	6 44		
HYDROGRAPH AT		122 66	14 00	74 12	27.01	9 18	15 29		
HYDROGRAPH AT		110 08	15 50	81 25	33 57	11 69	19 60		
3 COMBINED AT	BACIA	1028 86	15.00	789 46	342 04	119 93	201 59		
HYDROGRAPH AT		1042 04	15 50	804 64	348 36	122 16	205 70		
ROUTED TO	BARR	297 61	23 50	292 33	227 57	107 48	205 70	101 89 23 50	

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

ISTAQ	ELEMENT	DT (MIN)	PEAK (CMS)	TIME TO PEAK (MIN)	VOLUME (MM)	DT (MIN)	INTERPOLATED TO COMPUTATION INTERVAL		VOLUME (MM)		
							PEAK (CMS)	TIME TO PEAK (MIN)			
	MANE	22 13	123 91	842 73	155 79	30.00	123 68	840 00	155 62		
CONTINUITY SUMMARY (1000 CU-M)-INFLOW-			0000E+00	EXCESS-	2472E+04	OUTFLOW-	2458E+04	BASIN STORAGE-	1173E+01	PERCENT ERROR-	5
	MANE	21 98	132 48	898 69	155 41	30 00	132 44	900 00	155 29		
CONTINUITY SUMMARY (1000 CU-M)-INFLOW-			0000E+00	EXCESS-	3451E+04	OUTFLOW-	3423E+04	BASIN STORAGE-	.5233E+01	PERCENT ERROR-	6
	MANE	14 68	73 68	951 82	155 20	30 00	73.53	960 00	155 17		
CONTINUITY SUMMARY (1000 CU-M)-INFLOW-			0000E+00	EXCESS-	3032E+04	OUTFLOW-	3004E+04	BASIN STORAGE-	1223E+02	PERCENT ERROR-	5

000147

MANE 23 98 81 31 927 58 155 49 30.00 80 93 930.00 155 47

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 0000E+00 EXCESS- 2166E+04 OUTFLOW- 2150E+04 BASIN STORAGE- 3193E+01 PERCENT ERROR- 6

MANE 6 15 17 55 889.14 155 64 30 00 17 40 870 00 155 34

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 0000E+00 EXCESS- 4747E+03 OUTFLOW- 4716E+03 BASIN STORAGE- 6942E+00 PERCENT ERROR- 5

MANE 9 61 48 32 873 32 155 52 30.00 48 21 870 00 155.45

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 4703E+03 EXCESS- 7682E+03 OUTFLOW- 1234E+04 BASIN STORAGE- .8666E+00 PERCENT ERROR- 3

MANE 13 68 471 57 918 58 154 98 30 00 469.10 900 00 155 00

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 1225E+05 EXCESS- 1301E+04 OUTFLOW- 1352E+05 BASIN STORAGE- 2516E+01 PERCENT ERROR- 3

MANE 20 54 67 00 1117 13 155 33 30 00 66.92 1110.00 155 34

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 0000E+00 EXCESS- 3031E+04 OUTFLOW- 3005E+04 BASIN STORAGE- .1471E+02 PERCENT ERROR- 4

MANE 30 00 128 88 939 09 154 84 30 00 128 04 930.00 154 82

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 3003E+04 EXCESS- .2417E+04 OUTFLOW- 5384E+04 BASIN STORAGE- 4333E+01 PERCENT ERROR- 6

MANE 16 15 62.80 930 02 155 47 30 00 62 80 930 00 155 55

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 0000E+00 EXCESS- 2013E+04 OUTFLOW- 1997E+04 BASIN STORAGE- 4289E+01 PERCENT ERROR- 5

MANE 9 94 215 29 947 21 154 82 30 00 214 68 930 00 154 74

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 7377E+04 EXCESS- 8244E+03 OUTFLOW- 8188E+04 BASIN STORAGE- 1629E+01 PERCENT ERROR- 1

MANE 15 44 110 83 824 51 155 77 30 00 108 76 810 00 155 79

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 0000E+00 EXCESS- 2333E+04 OUTFLOW- 2320E+04 BASIN STORAGE- 1613E+01 PERCENT ERROR- 5

MANE 12 13 166 63 839 64 155 70 30 00 166 55 840 00 155 70

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 2318E+04 EXCESS- 1174E+04 OUTFLOW- 3485E+04 BASIN STORAGE- 7301E+00 PERCENT ERROR- 2

MANE 4 73 835 05 908 48 154 86 30 00 828 06 900 00 154 89

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 2517E+05 EXCESS- 5730E+03 OUTFLOW- 2573E+05 BASIN STORAGE- 1713E+01 PERCENT ERROR- 0

MANE 11 18 55 62 803 01 155 96 30 00 54 84 810 00 156.03

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 0000E+00 EXCESS- 1006E+04 OUTFLOW- 1001E+04 BASIN STORAGE- 3996E+00 PERCENT ERROR- 4

MANE 12 82 124 70 827 82 156 04 30 00 122 59 840 00 155.97

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 1001E+04 EXCESS- 1382E+04 OUTFLOW- 2378E+04 BASIN STORAGE- 7360E+00 PERCENT ERROR- 2

MANE 26 81 110 57 916 79 155 40 30 00 109 94 930 00 155 33

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 0000E+00 EXCESS- 3060E+04 OUTFLOW- 3036E+04 BASIN STORAGE- 4801E+01 PERCENT ERROR- 6

MANE 6 82 1048 74 912 92 154 81 30 00 1041 42 930 00 154 74

CONTINUITY SUMMARY (1000 CU-M)-INFLOW- 3112E+05 EXCESS- .6411E+03 OUTFLOW- 3174E+05 BASIN STORAGE- 2357E+01 PERCENT ERROR- .1

*** NORMAL END OF HEC-1 ***

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